1. GENERAL

1.1 ALL SFS DRAWINGS ARE TO BE READ IN CONJUNCTION WITH THE RELEVANT SFS SPECIFICATION AND ALL RELEVANT ARCHITECTS AND SERVICE ENGINEERS DRAWINGS AND SPECIFICATIONS.

1.2 ALL DIMENSIONS ARE IN MILLIMETRES UNLESS STATED OTHERWISE. ALL LEVELS ARE IN METRES AND RELATE TO ORDNANCE DATUM.

1.3 DO NOT SCALE FROM ANY DRAWING . WORK TO FIGURED DIMENSIONS ONLY. ANY DISCREPANCIES IN DIMENSION ARE TO BE REFERRED TO THE DESIGNER BEFORE WORK IS PUT TO HAND.

1.4 ALL DIMENSIONS AND LEVELS ARE TO BE CHECKED ON SITE BY THE CONTRACTOR PRIOR TO PREPARING ANY WORKING DRAWINGS OR COMMENCING ON SITE.

1.5 FOR SETTING OUT INFORMATION REFER TO ARCHITECTS DRAWINGS.

1.6 THE CONTRACTOR IS TOTALLY RESPONSIBLE FOR STABILITY OF THE BUILDING STRUCTURE AND EXCAVATIONS DURING THE CONSTRUCTION PERIOD, i.e. ITS TEMPORARY CONDITION, INCLUDING THE EFFECTS FROM CRANES AND HOISTS WHERE THESE ARE SUPPORTED AND/OR RESTRAINED BY THE STRUCTURE. THE CONTRACTOR SHALL PROVIDE ALL NECESSARY CALCULATIONS FOR THE TEMPORARY STRUCTURE AND WHERE NECESSARY DETAILED MEMBER CHECKS FOR THE PERMANENT STRUCTURE. ALL ADDITIONAL WORK REQUIRED FOR THE TEMPORARY STABILITY OF THE STRUCTURE SHALL DEEMED TO BE INCLUDED BY THE CONTRACTOR AND SHALL BE REMOVED FROM THE SITE ON COMPLETION OF THE PERMANENT STRUCTURE. IT IS THE CONTRACTORS RESPONSIBILITY TO CHECK THE PERMANENT WORKS FOR ANY LOADS APPLIED BY TEMPORARY WORKS.

1.7 ALL WORKS BY THE CONTRACTOR MUST BE CARRIED OUT IN SUCH A WAY THAT ALL REQUIREMENTS UNDER THE HEALTH AND SAFETY AT WORK ACT ARE SATISFIED.

1.8 ALL WORK IS TO BE CARRIED OUT IN COMPLIANCE WITH THE REQUIREMENTS OF THE STATUTORY AUTHORITIES AND CONSTRUCTION DESIGN AND MANAGEMENT REGULATIONS.

2. SITE PREPARATION/TEMPORARY WORKS

2.1 IT IS THE CONTRACTORS RESPONSIBILITY TO ESTABLISH FULL DETAILS AND EXTENT OF ALL PREVIOUS BUILDING FOUNDATIONS AND OBSTRUCTIONS ON THE SITE BY WHATEVER MEANS NECESSARY, PRIOR TO COMMENCING ANY TEMPORARY OR PERMANENT WORK ON SITE.

2.2 THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE DIVERSION OF THE BELOW GROUND SERVICES WITHIN THE WORKS ZONE AND THE PROTECTION OF ALL BELOW GROUND SERVICES WITHIN THE ZONE OF INFLUENCE OF THE WORKS. IT IS THE CONTRACTORS RESPONSIBILITY TO IDENTIFY ALL SERVICES WITH THE VARIOUS UNDERTAKERS AND CONFIRM THEIR LOCATIONS TO THE DESIGN TEAM PRIOR TO UNDERTAKING ANY WORK.

2.3 WATER IN THE EXCAVATIONS SHALL BE DEALT WITH IN SUCH A MANNER SO AS TO PREVENT THE DETERIORATION OF THE SURFACE ON WHICH FOUNDATION OR OTHER WORK WILL BE CONSTRUCTED. IT IS THE CONTRACTORS RESPONSIBILITY TO VERIFY THE LEVEL OF THE WATER TABLE.

2.4 THE CONTRACTOR WILL BE RESPONSIBLE FOR ANY DAMAGE TO THE HIGHWAY AND FOOTPATH AND ADJACENT PROPERTIES OUTSIDE OF THE TEMPORARY WORKS ZONE RESULTING FROM THE INSTALLATION, RENTAL OR PERFORMANCE OF THE TEMPORARY WORKS PRIOR TO THE COMPLETION OF THE PERMANENT CONSTRUCTION. THIS WILL INCLUDE THE REINSTATEMENT OF ANY DAMAGE TO BELOW GROUND SERVICES, AND FULL DEPTH RECONSTRUCTION OF ANY DAMAGED SECTIONS OF THE PUBLIC HIGHWAY.

2.5 THE CONTRACTOR MUST ENSURE THAT THE TEMPORARY WORKS ON SITE ARE TO HIS COMPLETE SATISFACTION.

2.6 NO EXCAVATED MATERIAL MAY BE USED IN THE PERMANENT WORKS WITHOUT PRIOR APPROVAL FROM THE ENGINEER.

3. FOUNDATIONS

3.1 ALL BELOW GROUND WORKS TO BE IN ACCORDANCE WITH BS 8004 : 1986 CODE OF PRACTICE FOR FOUNDATIONS.

3.2 FOUNDATIONS ARE TO BE PLACED CENTRALLY BENEATH COLUMNS AND WALLS UNLESS NOTED OTHERWISE

3.3 ALL FORMATION LEVELS ARE PROVISIONAL AND FORMATIONS ARE SUBJECT TO INSPECTION AND APPROVAL BY THE ENGINEER.

3.4 FORMATION LEVELS FOR FOUNDATIONS SHALL BE PROTECTED IMMEDIATELY AFTER APPROVAL WITH 50mm OF GRADE C16/20 BLINDING CONCRETE. UNLESS FOUNDATIONS ARE APPROVED AND POURED IMMEDIATELY FOLLOWING EXCAVATION.

3.5 NO EXCAVATIONS SHOULD ENCROACH WITHIN A 1 IN 2 SLOPING LINE EXTENDING DOWN FROM THE BOTTOM EDGE OF ANY FOUNDATION. WHERE IT IS NECESSARY FOR EXCAVATION TO BE EXTENDED BENEATH THIS LINE, THAT PART OF THE EXCAVATION BELOW THE SLOPING LINE IS TO BE BACKFILLED WITH MASS CONCRETE.

3.6 ALL EXCAVATIONS SHALL BE CLEAN, ACCURATELY DUG AND ADEQUATELY SUPPORTED.

3.7 NO ADDITIONAL EXCAVATION IS TO TAKE PLACE WITHOUT THE AUTHORITY OF THE ENGINEER WHERE IT IS NECESSARY TO EXCAVATE FURTHER TO REACH ACCEPTABLE BEARING STRATA, THE RESULTANT VOIDS ARE TO BE FILLED WITH MASS CONCRETE GRADE GEN1.

3.8 ALL EXCAVATIONS SHALL BE CLEANED OUT PRIOR TO INSPECTION AND THE CONTRACTOR SHALL PROVIDE THE STRUCTURAL ENGINEER AND THE LOCAL AUTHORITY INSPECTOR EVERY FACILITY TO CARRY OUT SUCH INSPECTION IN A SAFE MANNER.

3.9 THE GROUND BELOW NEW FORMATION LEVEL MUST NOT BE DISTURBED AND EXCAVATION ARE TO REMAIN OPEN FOR AS SHORT A PERIOD AS POSSIBLE AND MUST BE LINED WITHIN THE SAME DAY.

3.10 ALL STRUCTURAL CONCRETE IN CONTACT WITH THE GROUND IS TO BE IN ACCORDANCE WITH THE RELEVANT DS CLASS AND AC CLASS OF ACEC OF BRE SPECIAL DIGEST 1, 2001 (THIS SPECIAL DIGEST REPLACED DIGEST 363 IN AUGUST 2001)

3.11 CARE MUST BE TAKEN TO ENSURE THAT THE GROUND BENEATH ANY GROUND FLOOR SLAB IS NOT DISTURBED DURING EXCAVATIONS

3.12 WATER FROM EXCAVATIONS SHALL BE DEALT WITH IN AN MANNER APPROVED BY THE SCOTTISH ENVIRONMENT PROTECTION AGENCY WITH PARTICULAR ATTENTION BEING GIVEN TO THE FOLLOWING
-DEPOSITION OF FINES OR OTHER CONTAMINATES
-POLLUTION
-FLOODING
-SCOUR

3.13 WATER FROM THE EXCAVATIONS SHALL NOT DISCHARGE TO THE PUBLIC SEWER, UNLESS APPROVED BY SCOTTISH WATER

4. CONCRETE

4.1 ALL STRUCTURAL CONCRETE TO BE GRADE RC32/40 IN ACCORDANCE WITH BS EN206-1 : 2000 AND BS 8500 : PARTS 1 & 2 : 2002 UNLESS NOTED OTHERWISE.

4.2 ALL REINFORCEMENT BARS SHALL BE IN ACCORDANCE WITH BS 8666 WITH YIELD STRENGTH OF 460N/mm^2 OR 250N/mm^2 FOR BARS DENOTED T OR R RESPECTIVELY. ALL WIRE MESH SHALL BE IN ACCORDANCE WITH BS 4482 : 1985 AND BS 4483 : 1998 WITH A MINIMUM YIELD STRENGTH OF 460N/mm^2. ALL HIGH YIELD BARS TO BE DEFORMED TYPE 2.

4.3 SURFACES OF ALL CONCRETE FLOOR SLABS NOT RECEIVING DIRECT FINISHES TO BE SEALED WITH A PROPRIETARY SURFACE HARDENER/ANTI DUST AGENT COMPATIBLE WITH THE TYPE OF CONCRETE PLACED. THE CONTRACTOR SHALL OBTAIN THE APPROVAL OF THE ARCHITECT AND THE ENGINEER PRIOR TO PLACEMENT.

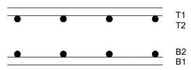
4.4 FLOOR TO HAVE POWER FLOAT FINISH WITH A TOLERANCE OF ±3mm OVER A 3m STRAIGHT EDGE.

5. REINFORCEMENT DETAILING

5.1 CONCRETE TO BE GRADE RC32/40 AND COVER TO REINFORCEMENT IS TO BE: 40mm TO TOP AND SIDES, 40mm TO BTM. U.N.O. NOMINAL COVER WHERE CONCRETE IS CAST DIRECTLY AGAINST EARTH SHALL NOT BE LESS THAN 75mm. WHERE CONCRETE IS CAST AGAINST ADEQUATE BLINDING A NOMINAL COVER OF 40mm SHALL BE USED.

5.2 ABBREVIATIONS.
R = MILD STEEL
B = HIGH YIELD, HIGH BOND
T = TOP
B = BOTTOM
NF = NEAR FACE
FF = FAR FACE
AR = ALTERNATE BARS REVERSED
AP = ALTERNATELY PLACED
STG = STAGGERED

5.3 REINFORCEMENT LAYERS AS INDICATED:



5.4 MINIMUM REINFORCEMENT LAPS (u.n.o)

BAR DIA.	10	12	16	20	25	32	40
MIN LAP	400	480	640	800	1000	1280	1600

6. STEELWORK

6.1 ALL STEELWORK DETAILING, FABRICATION AND ERECTION TO BE IN ACCORDANCE WITH THE LATEST ISSUE OF BS 5950, INCLUDING ALL THE LATEST AMENDMENTS

6.2 STEELWORK GRADES TO BE IN ACCORDANCE WITH BS EN 10025 : 1993, BS EN 10113 : 1993 AND BS EN 10210 : 1994 AS FOLLOWS. UNLESS NOTED OTHERWISE ON THE DRAWINGS.

6.3 STEELWORK GRADES WILL GENERALLY BE; GRADE S355JR QUALITY FOR INTERNAL STEELWORK WHERE MAX ELEMENT THICKNESS <30mm. GRADE S355J0 QUALITY FOR EXTERNAL STEELWORK. GRADE S355JR QUALITY FOR INTERNAL STEELWORK COLUMNS WHERE MAX ELEMENT THICKNESS <21mm. GRADE S355J0 QUALITY FOR EXTERNAL STEELWORK COLUMNS.

6.4 CONNECTIONS SHALL BE MADE USING MINIMUM 2 No M16 GRADE 8.8 BLACK BOLTS PER CONNECTION OR EQUIVALENT IN 6mm FILLET WELD.

6.5 ALL STEEL COLUMNS TO BE CENTRAL ON GRIDS OR EQUALLY SPACED BETWEEN GRIDS UNLESS NOTED OTHERWISE.

6.6 ALL STEEL BEAMS TO BE CENTRAL ON GRIDS OR EQUALLY SPACED BETWEEN COLUMNS UNLESS NOTED OTHERWISE.

6.7 ALL STEELWORK DIMENSIONS ARE TO CENTRELINE OF SECTION UNLESS NOTED OTHERWISE.

6.8 ALL BRACING IS TO BE SET OUT ON THE CENTROIDS OF BRACING MEMBERS AND ON THE CENTRE LINE OF BEAMS AND COLUMNS UNLESS NOTED OTHERWISE.

6.9 WHERE BRACING IS SHOWN TO BE OFFSET FROM CENTRE OF MEMBERS THE DOMESTIC SUB-CONTRACTOR SHALL DESIGN AND PROVIDE ALL NECESSARY STIFFENERS.

6.10 THE DOMESTIC SUB-CONTRACTOR IS TO BE RESPONSIBLE FOR THE DESIGN OF ALL STEEL TO STEEL CONNECTIONS. INCLUDING ALL NECESSARY STIFFENERS TO SATISFY THE APPLIED CONNECTION LOADINGS.

6.11 ALL STEELWORK TO BE BLAST CLEANED TO Sa2.5 AND PAINTED WITH 75 MICRON METRES OF ZINC RICH PRIMER.

6.12 ALL EXTERNAL STEELWORK TO BE GALVANISED

6.13 TOP FLANGES OF BEAMS TO RECEIVE STUD WELDING TO BE LEFT UNPAINTED.

6.14 NO SITE WELDING OR SITE CUTTING OF STEELWORK SHALL BE ALLOWED WITHOUT THE EXPRESS APPROVAL OF THE STRUCTURAL ENGINEER.

6.15 WHERE CONNECTION LOADS ARE NOT APPARENT FROM DRAWINGS, THE CONTRACTOR MUST CONFIRM LOADING CONDITIONS WITH THE STRUCTURAL ENGINEER PRIOR TO FABRICATION, OR DESIGN CONNECTIONS TO RESIST FULL CAPACITY OF MEMBER.

6.16 STEELWORK CONTRACTOR TO PROVIDE ALL CLEADER RAILING AS REQUIRED TO SUPPORT FREE EDGES NOT TRIMMED WITH COLD FORMED OR MILD STEELWORK. TO BE PROVIDED IN ACCORDANCE WITH ARCHITECTS DRAWINGS.

6.17 STEELWORK CONTRACTOR TO ALLOW FOR ALL ANTI-SAG BRACING REQUIREMENTS FOR ROOF PURLIN RAILS, SIDE SHEETING RAILS AND EAVES BEAMS IN ACCORDANCE WITH MANUFACTURERS RECOMMENDATIONS. A MINIMUM OF TWO VERTICAL STRUT RAILS PER BAY SHOULD BE TAKEN TOGETHER WITH THE NECESSARY DIAGONAL TIE BRACING.

6.18 THE STEELWORK CONTRACTOR IS RESPONSIBLE FOR THE DESIGN AND SUPPLY OF ALL STEELWORK TO CONCRETE CAST IN CONNECTION UNLESS NOTED OTHERWISE

7. LOADINGS

7.1 TYPICAL APPLIED LOADS
DEAD:
RAFTER 1.15 kN/m2
CEILING FINISHES 0.25 kN/m2

7.2 IMPOSED FLOOR LOADS
RAFTER 0.60 kN/m2
FLOOR(DOMESTIC) 1.50 kN/m2

7.3 BARRIER LOADS
WHERE GLAZING OR BALLUSTRADES ACT AS A BARRIER THE ELEMENT SHOULD BE DESIGNED IN ACCORDANCE WITH BS6399 PART 1: TAB 4 USING THE BARRIER LOADS APPLICABLE TO THE BUILDING USE. IF IN DOUBT PLEASE CONSULT SFSTRUCTURES PRIOR TO PROGRESSING DESIGN

7.3 WIND LOADS TO BS6399 : PART 2 : 1997 WITH SITE WIND SPEED Vs: SITE SPECIFIC - 23.7 M/S

7.4 SNOW LOADS TO BS6399 : PART 3 : 1988 MINIMUM 0.60 kN/m2

8. TIMBER

8.1 ALL TIMBER TO BE GRADE C24 u..n.o

8.2 SEE PLAN DRAWINGS FOR TIMBER LINTEL REQUIREMENTS

8.3 SEE PLAN DRAWINGS FOR CRIPPLE STUD REQUIREMENTS

8.4 ALL TIMBER TO BE DESIGNED IN ACCORDANCE WITH BS5268 : PARTS 1-3 AND IN ACCORDANCE WITH THE DESIGN LOADS AS DETAILED IN SECTION 7 OF THESE NOTES.

8.5 ALL TIMBER TO BE PRESSURE IMPRENATED VACUUM TREATED WITH PRESERVATIVE IN COMPLIANCE WITH BS5268 PART 5.

8.6 THE TIMBER MOISTURE CONTENT AT ANY TIME SHALL NOT EXCEED 18%

8.7 TIMBER SHALL BE SOUND AND FREE FROM FROM ATTACK BY INSECTS OR FUNGI

8.8 ALL ASPECTS OF TIMBER MATERIALS, FABRICATION AND WORKMANSHIP TO COMPLY WITH BS 5268:PARTS 2 AND 3

8.9 REFER TO GENERAL NOTES FOR NAILING SCHEDULE

8.10 HOLES IN RECTANGULAR JOISTS TO BE DRILLED AT THE NEUTRAL AXIS WITH DIAMETER NOT EXCEEDING 0.25 OF THE DEPTH OF JOISTS AND NOT LESS THAN 3 DIAMETERS (CENTRE TO CENTRE) APART. THEY ARE TO BE LOCATED BETWEEN 0.25 AND 0.40 OF THE SPAN FROM THE SUPPORT.

8.11 FOR HOLING IN STRUCTURALLY ENGINEERED JOISTS REFER TO MANUFACTURERS RECOMMENDATIONS

8.12 NOTCHES IN SIMPLY SUPPORTED RECTANGULAR FLOOR AND ROOF JOISTS SHALL NOT EXCEED 0.125 OF THE DEPTH OF THE JOIST AND SHALL BE LOCATED BETWEEN 0.07 AND 0.25 OF THE SPAN FROM THE SUPPORT

8.13 FLOOR JOISTS TO HAVE MID SPAN FULL DEPTH DWANGS OR HERRING BONES STRUTS FOR SPANS EXCEEDING 2500mm. WHERE SPAN EXCEEDS 4500mm PROVIDE 2 ROWS OF DWANGS OR STRUTS

9. MASONRY

9.1 ALL BLOCKWORK TO BE SOLID WITHOUT VOIDS TO BS EN 771-1 AND TO HAVE A MINIMUM CRUSHING STRENGTH OF 7.5 N/mm^2 UNLESS NOTED OTHERWISE. ALL PERPENDS TO BE FULLY FILLED.

9.2 ALL 140mm THICK BLOCKWORK TO HAVE A MAXIMUM DENSITY OF 1400Kg/m^3 AND A BLOCK WEIGHT NOT EXCEEDING 20Kg.

9.3 ALL BRICKWORK TO BE CONSTRUCTED WITH CLAY BRICKS TO BS 3921: 1985, WITH A MINIMUM CRUSHING STRENGTH OF 20N/mm^2 AND MAXIMUM WATER ABSORPTION OF <12%.

9.4 MORTAR TO BE DESIGNATION M6 (TYPE (iii)) i.e. 1:3 MASONRY SAND CEMENT IN ACCORDANCE WITH BS 5628 PART 3:2005

9.5 LINTELS SHALL BE ROBESLEE HI-SPEC SYSTEM OR EQUAL AND APPROVED UNLESS NOTED OTHERWISE AND INSTALLED IN ACCORDANCE WITH MANUFACTURERS REQUIREMENTS.

EXTERNAL LEAF	CLEAR SPAN	TYPE
UPTO 140mm WIDE	UPTO 1800mm	ROBESLEE TYPE C
UPTO 140mm WIDE	UPTO 2400mm	ROBESLEE TYPE K9

NOTE: WHERE THE EXTERNAL LEAF WIDTH IS GREATER THAN 140mm THE STRUCTURAL ENGINEER SHOULD BE CONSULTED TO CONFIRM THE SUPPORT REQUIREMENTS.

9.6 WHERE CLEAR SPAN EXCEED 3m SECONDARY STEELWORK SHALL BE PROVIDED

9.7 MASONRY TO BE REINFORCED IN FIRST AND THIRD COURSES IN BOTH LEAFS OF EXTERNAL WALLS ABOVE AND BELOW OPENING TO RESTRAIN MOVEMENT USING BRC BRICKFORCE SBF35W60 EXPANDED STEEL MESH TO BS 5628:PART 3. MINIMUM 600mm PROJECTION BEYOND EACH SIDE OF OPENING OR EQUAL AND APPROVED.

9.8 IF DISTANCE BETWEEN MOVEMENT JOINTS EXCEED 6m (MAX 9m) IN BLOCKWORK AND/OR 9m (MAX 12m) IN BRICKWORK, REINFORCE USING BRC BRICKFORCE SBF35W60 (100mm WIDTH) OR SBF35W100 (140mm WIDTH) AT 450 CENTRES OR EQUAL AND APPROVED.

9.9 ALL INTERNAL WALLS TO BE TIED AT INTERSECTIONS WITH EXTERNAL CAVITY WALLS USING ANCON 36/8 WALL EXTENSION SYSTEMS OR EQUAL AND APPROVED, FITTED IN ACCORDANCE WITH MANUFACTURES INSTRUCTIONS.

9.10 BEDJOINT REINFORCEMENT TO BE DESIGNED AND MANUFACTURED IN ACCORDANCE WITH BS 5628 : PART 3 : 1985 AND PROVIDED BY BRC BUILDING PRODUCTS OR EQUAL AND APPROVED.

9.11 ALL PERIMETER EXTERNAL OUTER LEAF WALLS SHOULD BE SUPPORTED AT INTERVALS OF NOT MORE THAN 9m OR THREE STOREYS, WHICHEVER IS LESS, IN ACCORDANCE WITH BS 5628 : PART 1 : 1992.

9.12 TO ALLOW FOR VERTICAL MOVEMENT, MOVEMENT JOINTS SHOULD BE GENERALLY POSITIONED AT EVERY STOREY OR EVERY SECOND STOREY.

10. FORMING OPENINGS IN EXISTING WALLS

10.1 BRICKWORK FOR INFILLING NEEDLE HOLES, SCRUNTIONS AND GENERAL REBUILDING SHOULD BE COMMON CLASS 3 BRICKWORK USING CLASS (iii) MORTAR.

10.2 DRY PACKING MORTAR FOR INFILLING ABOVE NEW BEAMS AND LINTELS AFTER ERECTION SHOUDL BE 3:1 SAND:CEMENT MORTAR USING MINIMUM WATER. THE MORTAR SHOULD BE DRIVEN IN PLACE USING STEEL OR TIMBER DRIFT TO FULLY RAM THE MORTAR IN PLACE. CARE SHOULD BE TAKEN TO ENSURE THAT THE JOINT IS COMPLETELY FILLED AND WELL COMPACTED TO ENSURE FULL AND EQUAL LOAD TRANSFER

10.3 CONTRACTOR TO PRESENT FULL METHOD STATEMENT FOR PROPPING AND CREATING NEW OPENINGS IN EXISTING STRUCTURE

10.4 IT IS RECOMMENDED THAT THE CONTRACTOR CARRY OUT A DILAPIDATION SURVEY OF ALL ADJACENT PROPERTIES AND THAT ALL WORK IS PERFORMED WITH MINIMUM VIBRATION

11. GLAZING SYSTEMS

11.1 ALL GLAZING PANELS < 2m² TO BE DESIGN IN ACCORDANCE WITH BS 6262:2005 PARTS 1 TO 7

11.2 ALL GLAZING PANELS GREATER THAN 2m² TO BE DESIGN IN ACCORDANCE WITH BS 6262:2005 PARTS 1 TO 7 AND DESIGN FORWARD TO SFSTRUCTURES 2 WEEKS PRIOR TO FABRICATION FOR REVIEW. DESIGN MUST INCLUDE JUSTIFICATION OF GLASS THICKNESS / SPECIFICATION AND WELL AS JUSTIFICATION FOR ALL MULLIONS, TRANSOMS AND FIXINGS BACK TO PRIMARY STRUCTURE

11.3 GLAZING TO BE DESIGNED FOR A WIND LOAD OF 1.30kN/m2 UNLESS A LOWER LOAD APPROPRIATE TO THE LOCATION IS PROVEN BY RECOGNISED CALCULATION USING BS6262:2005

11.3 GLAZING TO BE TOUGHENED AND BE SUITABLE TO WITHSTAND USER IMPACT LOADING TO COMPLY WITH BUILDING REGULATIONS REQUIREMENTS.

11.4 WHERE GLAZING PANEL ACTS AS A BARRIER THE PANEL SHOULD BE DESIGNED IN ACCORDANCE WITH BS6399 PART 1: TAB 4 USING THE BARRIER LOADS APPLICABLE TO THE BUILDING USE. IF IN DOUBT PLEASE CONSULT SFSTRUCTURES PRIOR TO PROGRESSING DESIGN

11.5 GLAZING INSTALLED TO COMPLY WITH BS6262:2005 - PARTS 1-7 AND BS5516:2004 - PARTS 1 AND 2

11.6 FIXING OF GLAZING PANELS <2m² TO C16 TIMBER STUDS WITH COMPATIBLE LUGS SECURED WITH No. 8 WOOD SCREWS @ 250mm CENTRES. MINIMUM 3No. FIXINGS PERSIDE OF PANEL.

11.7 FIXING OF GLAZING PANELS <2m² TO 7N/mm² MASONRY WITH COMPATIBLE LUGS SECURED WITH HILTI HUS SCREW ANCHORS @ 250mm CENTRES. MINIMUM 3No. FIXINGS PERSIDE OF PANEL.

11.8 FIXING OF GLAZING PANELS <2m² TO LIGHT GUAGE STEEL (1.2 - 3.0mm THK) WITH COMPATIBLE LUGS SECURED WITH ASH & LACY LS 38 TECH SCREWS @ 250mm CENTRES. MINIMUM 3No. FIXINGS PERSIDE OF PANEL.

11.9 FIXING OF GLAZING PANELS <2m² TO HEAVY GUAGE STEEL (4.0 - 12.5.0mm THK) WITH COMPATIBLE LUGS SECURED WITH ASH & LACY HS 38 TECH SCREWS @ 250mm CENTRES. MINIMUM 3No. FIXINGS PERSIDE OF PANEL.

11.10 FIXING OF GLAZING PANELS <2m² TO THERMOPLAN BLOCKS VIA LUG FIXINGS TO BE SECURED WITH 2No. FISCHER UX 6 PLUGS. MINIMUM 2No. LUGS PLACED AT MAXIMUM 300mm CENTRES

12 CONTRACTOR DESIGN REQUIREMENTS

12.1 THE SFS STRUCTURAL CERTIFIER USES THE COMPETENCE OF AN IDENTIFIED THIRD PARTY FOR ALL SPECIALIST CONTRACTOR DESIGNED ELEMENTS REFERENCED IN CLAUSE 14 AND AS IDENTIFIED ON THE SER STRUCTURAL DESIGN CERTIFICATE. THIS IS SUBJECT TO THE LIMITATIONS SPECIFIED BY THE SER SCHEME.

12.2 THE CONTRACTOR SHALL BE DEEMED TO HAVE VISITED THE SITE BEFORE SUBMITTING HIS TENDER AND SHALL BE WHOLLY RESPONSIBLE FOR ACQUAINTING HIMSELF WITH THE NATURE AND THE POSITION OF THE SITE, SERVICE LOCATIONS, GROUNDWATER LEVELS, SOIL CONDITIONS, OBSTRUCTIONS, THE FULL EXTENT OF THE WORKS AND ALL OTHER INFORMATION RELEVANT TO THE EXECUTION OF THE WORKS.

12.3 CONTRACTOR DESIGNED ELEMENTS - INFORMATION TO BE PROVIDED TO APPROVED CERTIFIER OF DESIGN (BUILDING STRUCTURES) THE CONTRACTOR SHALL BE RESPONSIBLE FOR PROVIDING FULLY CHECKED STRUCTURAL DESIGN CALCULATIONS, DRAWINGS AND CONFIRMATION OF DESIGNER COMPETENCE TO THE APPROVED CERTIFIER OF DESIGN (BUILDING STRUCTURES). THIS WILL BE PRESENTED IN FULL, FOR REVIEW, PRIOR TO THE CERTIFICATE BEING SIGNED AND SUBMITTED TO BUILDING CONTROL AND PRIOR TO ANY WORK COMMENCING ON SITE. ALL CONTRACTOR DESIGNED ELEMENTS MUST BE UNDERTAKEN BY AND/OR CHECKED/APPROVED BY A SUITABLY EXPERIENCED CHARTERED CIVIL OR STRUCTURAL ENGINEER. ALL OF THE ABOVE TO BE PROVIDED IN A TIMELY MANNER, TO ASSIST IN OBTAINING BUILDING REGULATIONS APPROVAL AND SIGN OFF UNDER THE BUILDING (SCOTLAND) ACT 2003 .

12.4 THE CONTRACTOR MAY PROVIDE COSTINGS FOR ALTERNATIVE DESIGN SOLUTIONS, IF IT CAN BE PROVEN THAT THE PROPOSED DESIGN WILL PROVIDE THE REQUIRED CAPACITY AND MEET ALL THE REQUIREMENTS OF THE ENGINEERS SPECIFICATION, THE CLIENTS REQUIREMENTS AND BE SUPPORTED WITH CALCULATIONS AND DRAWINGS UNDERTAKEN BY AND/OR CHECKED/APPROVED BY A SUITABLY EXPERIENCED CHARTERED CIVIL OR STRUCTURAL ENGINEER.

13. CONTRACTOR DESIGNED ELEMENTS

13.1 WITH REFERENCE TO NOTE 13, THE FOLLOWING ITEMS, WHILE FORMING PART OF THE PROJECT STRUCTURAL DESIGN COVERED BY THE SFS SER CERTIFICATE, ARE SUBJECT TO DETAILED DESIGN BY A SPECIALIST CONTRACTOR IN ACCORDANCE WITH THE SFS PERFORMANCE SPECIFICATION.

· GLAZING PANELS GREATER THAN 2m² IN ELEVATION

TIMBER FRAME NAILING SCHEDULE (REFER ALSO TO TIMBER FRAME NAILING SCHEDULE DIAGRAM BELOW)

TIMBER FRAMING – 100mm x 4mm

1	Studs to head and sole plate	2No. end nailed each end
2	Noggings to studs	2No. toe nailed each end
3	Studs to cripple	600mm c/c, face nailed staggered
4	Lintel (bearing on cripple stud)	4No. end nailed each end
5	Lintel / lintel	300mm c/c, face nailed staggered
6	Plate below opening to cripple	2No. end nailed each end
7	Studs below opening to plate above	2No. end nailed
8	Beam: Joist / joist	300mm c/c, face nailed staggered

PLYWOOD / OSB NAILING – 75mm x 3.35mm

9	Gable panels	3.35mm at 100mm c/c perimeter, 200mm c/c internal
10	Front and back wall panels	3.35mm at 100mm c/c perimeter, 200mm c/c internal
11	Internal sheathed panels	3.35mm at 100mm c/c perimeter, 200mm c/c internal

FOUNDATION NAIL SIZE NUMBER OR SPACING

13	Wall plate / brick base	M10 HILTI HY50 resin anchor 800mm centres
14	Wall plate / half lapped joint	100mm X 4mm 2No. toe nailed
15	Anchor strap to stud	100mm X 4mm Stainless Steel 6No. face nailed

WALL FRAMING

18	G.F storey sole plate / wall plate	100mm x 4mm ring shank 150mm c/c face nailed
19	Stud / stud	100mm x 4mm 200mm c/c
20	Stud / corner post	100mm x 4mm 200mm c/c
21	Head binder / panel	100mm x 4mm 300mm c/c face nailed, staggered
22	1st floor header joist / 1st floor joist	100mm x 4mm 2No. face nailed
23	1st floor header joist / head binder	100mm x 4mm ring shank 450mm c/c toe nailed
24	Upper storey sole plate / header joist	100mm x 4mm ring shank 450mm c/c face nailed
25	Roof head binder / panel	100mm x 4mm 350mm c/c face nailed
26	Stud / stud – int' sheathed wall to ext'	100mm x 4mm 300mm c/c face nailed staggered

ROOF FRAMING

28	Roof truss / head binder	45mm x 4mm 12No by truss clip (6+6)
29	Gable truss / head binder	100mm x 4mm 450mm c/c toe nailed, staggered
30	End gable tie / gable truss	100mm x 4mm 3No. top nailed
31	End gable tie / ceiling tie	100mm x 4mm 1No. top nailed per ceiling tie

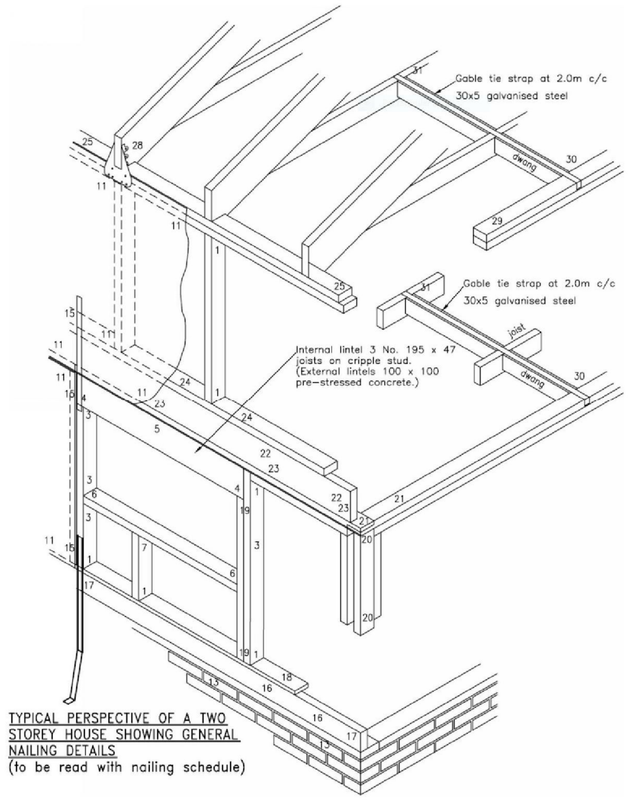
FIRST FLOOR DIAPHRAGM

32	Chipboard joist or dwang	60mm x 3mm deformed	75mm c/c
33	Chipboard / joist or dwang parallel to front and back walls	60mm x 3mm deformed	150mm c/c

NOTE: All nails to be sheradised unless noted otherwise

DISPROPORTIONATE COLLAPSE NAILING
In accordance with UKTFA technical bulletin 3 and Trada publication 'Timer Frame Housing: UK Structural Recommendations 75mm x 3.35mm diameter nails at 300mm centres between the lower rails of the upper wall panels and the upper rails of the panels below is sufficient to provide effective horizontal tying required.

TIMBER FRAME NAILING SCHEDULE DIAGRAM



Status: TENDER

Client: MR & MRS BULL

Project: QUEEN VICTORIA DRIVE

Title: GENERAL NOTES

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Design: KHA	CAD: KHA	5
Chk'd: SF	App'd: SF	
Date: SEPT 2020	Scale: 1 : 10 @ A1	

No. 00*** S/010 Rev: 01

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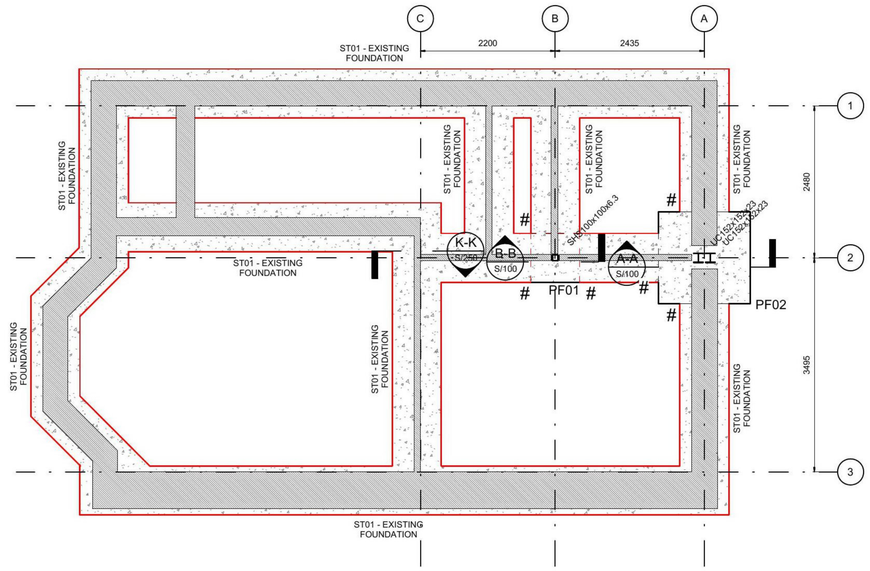

FOUNDATION LAYOUT

SCALE 1 : 50

TOF = SAME AS EXISTING TOF LEVEL OR -0.450 MIN. BELOW GROUND LEVEL (WHICH EVER IS DEEPER)

REFER TO SF STRUCTURES GENERAL NOTES SPECIFICATION FOR FURTHER DETAILS

NOTES

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5. REFER TO SCHEDULE 1 OF THE SER CERTIFICATE FOR CONFIRMATION OF THE CONTRACTOR DESIGNED ELEMENTS.

FOUNDATION LEGEND

TOP OF FOUNDATION LEVEL . . . TOF .....

STRIP FOUNDATION SF01

SLAB LEGEND

STRUCTURAL SLAB LEVEL . . . . . SSL 0.000

TIMBER & MASONRY LEGEND

DENOTES EXTENT OF 47x97mm C16 TIMBERS @ 600mm C/C INTERNAL PARTITIONS WITH 12.5mm PLASTERBOARD TO EACH FACE. REFER TO C&S SPECIFICATION FOR FURTHER DETAILS AND NAILING SCHEDULE.

DENOTES EXISTING WALLS

APPROVED FORMATION (RAISING LEVELS) NOTE:

WHERE FORMATION LEVEL TO BE RAISED CONTRACTOR TO STRIP BACK TO SUITABLE FORMATION STRATA PRIOR TO BUILDING UP WITH CLASS 6F2 / MOT TYPE 1 SUB BASE LAID IN 150mm LAYERS EACH COMPACTED WITH MINIMUM 8 PASSES WITH A 1.5t ROLLER. SUB BASE TO EXTEND MINIMUM 2m BEYOND EDGE OF ANY STRIP OR PAD FOUNDATION.

IF FILL DEPTH BELOW SLAB EXCEEDS 600mm ENGINEER TO BE NOTIFIED.

ALL FORMATION LEVELS SUBJECT TO ENGINEERS INSPECTION.

BLOCKWORK BELOW DPC:

BRICK QUALITY DESIGNATION F2 AND S2. BLOCKWORK WITH MINIMUM CRUSHING STRENGTH OF 7.3N/mm². BOTH IN ACCORDANCE WITH BS EN 771-1. MORTAR TO BE DESIGNATION M6 (CLASS (ii)) i.e. 1:3 MASONRY SAND CEMENT IN ACCORDANCE WITH BS EN998-2

CAVITY WIDTH TO ARCHITECTS SPECIFICATION

RADON NOTE:

SITE IS LOCATED IN A LOWEST RADON PROBABILITY AREA (<1%), TAKEN FROM THE INTERACTIVE MAP OF RADON IN SCOTLAND.

IN ACCORDANCE WITH BRE REPORT 211 - RADON GUIDANCE, TO ACHIEVE BASIC RADON PROTECTION ALL JOINTS AND PENETRATIONS TO BE PROPERLY SEALED AND EDGES LAPPED WITH DPC. MINIMUM 1200g DPM TO BE USED BENEATH SLAB

COAL MINING REPORT:

SITE IS CONSIDERED A LOW RISK FROM HISTORICAL COAL MINING ACTIVITIES, TAKEN FROM THE COAL AUTHORITY INTERACTIVE MAP OF MINING REPORT AREAS.

NO COAL MINING REPORT REQUIRED.

STRIP FOUNDATION DOWEL DETAIL:

NEW STRIP FOUNDATION DOWELLED INTO THE EXISTING STONE FOUNDATION WITH MINIMUM 3No. 500mm LONG, H12 REINFORCEMENT. BARS TO BE DOWELLED MINIMUM 150mm INTO EXISTING FOUNDATION AND FIXED WITH HILTI HY270 RESIN ALL TO MANUFACTURERS GUIDELINES.

BEARING PRESSURE NOTE:

BEARING PRESSURE ASSUMED 150kN/m² AT TIME OF GOING TO BUILDING WARRANT . BEARING PRESSURE TO BE VALIDATED THRO SITE INSPECTION. ALL FORMATIONS ARE PROVISIONAL AND SUBJECT TO INSPECTION BY ENGINEER. BEARING STRATA CONSIDERED TO COMPRISE OF GLACIAL TILL - CLAY SAND, GRAVEL WITH POTENTIAL BOULDERS.

CONTRACTOR TO CONTACT SFSTRUCTURES SHOULD UNEXPECTED GROUND CONDITIONS BE ENCOUNTERED ANYWHERE ACROSS THE SITE.

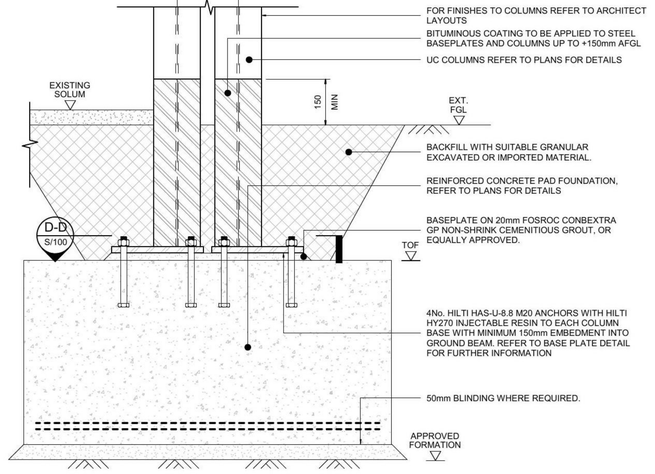
TRIAL PITTING:

CONTRACTOR TO ALLOW FOR CARRYING OUT TRIAL PIT INVESTIGATION ALONGSIDE ANY EXISTING WALLS TO ESTABLISH FOUNDATION MAKE UP, DEPTH TO FORMATION AND SOIL CONDITIONS PRIOR TO ORDERING MATERIALS AND COMMENCING MAIN WORKS.

DIMENSIONS NOTE:

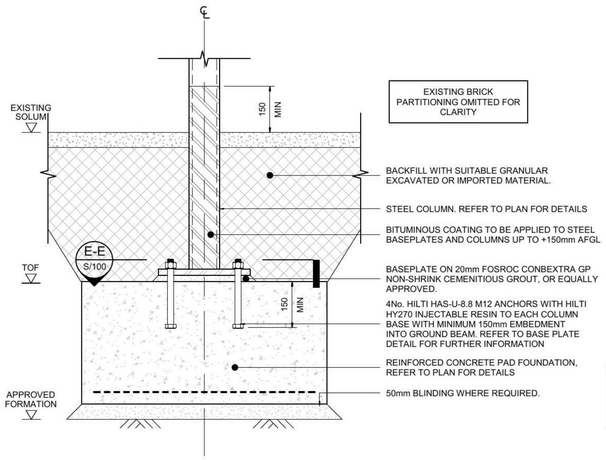
ALL DIMENSIONS SETTING OUT FOUNDATIONS ARE TO CENTRELINE OF STRIP FOUNDATIONS.

ALL DIMENSIONS SETTING OUT FOUNDATIONS TO BE CHECKED AND REFERENCED AGAINST LATEST ARCHITECTS DRAWINGS.



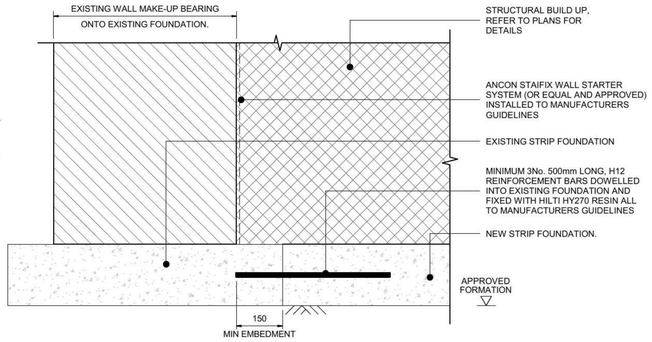
SECTION A-A - TYPICAL SECTION THRO' GROUND BEAM / COLUMN BASE

SCALE 1 : 10



SECTION B-B - TYPICAL SHS PAD FOUNDATION DETAIL

SCALE 1 : 10



SECTION C-C - TYPICAL DOWELLED FOUNDATION DETAIL (#)

SCALE 1 : 10

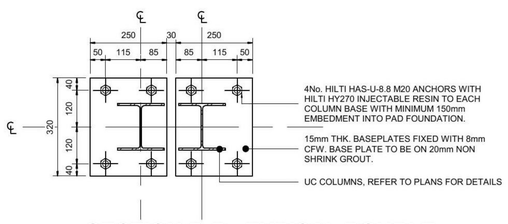
STRIP FOUNDATION TABLE (C32/40)

REF.	WIDTH x DEPTH (mm)
ST01	EXISTING FOUNDATION

PAD FOUNDATION TABLE (C32/40)

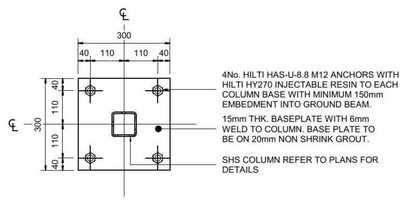
REF.	WIDTH (mm)	LENGTH (mm)	DEPTH (mm)
PF01	800	800	400
PF02	1500	1500	400

NOTE: PF01 TO HAVE A252 MESH REINFORCEMENT TO BOTTOM FACE. PF02 TO HAVE 2No. LAYERS OF A393 MESH TO BOTTOM FACE. ALL REINFORCEMENT PLACED WITH 40mm COVER TO ALL SIDES



SECTION D-D - TYPICAL DOUBLE COLUMN BASE PLATE DETAIL

SCALE 1 : 10



SECTION E-E - TYPICAL SHS BASE PLATE DETAIL

SCALE 1 : 10

ITEM	RESIDUAL HAZARDS IDENTIFIED - SUBSTRUCTURE DESCRIPTION
1	UNSTABLE GROUND DUE TO SLOPES, EXCAVATIONS AND EARTHWORKS ON SITE. CONTRACTOR TO ENSURE ALL SLOPES AND EXCAVATIONS ARE STABLE
2	POTENTIAL TO UNDERMINE EXISTING BUILDING FOUNDATIONS WHILST EXCAVATING NEW FOUNDATIONS. CONTRACTOR TO IDENTIFY EXISTING FOUNDATION DEPTH AND ADVISE ENGINEER OF ANY ISSUES

Status: TENDER

Client: MR & MRS BULL

Project: QUEEN VICTORIA DRIVE

Title: FOUNDATION LAYOUTS

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Design: KHA	CAD: KHA
Chk'd: SF	App'd: SF
Date: SEPT 2020	Scale: As indicated @ A1

No. 00*** S/100 Rev: 01

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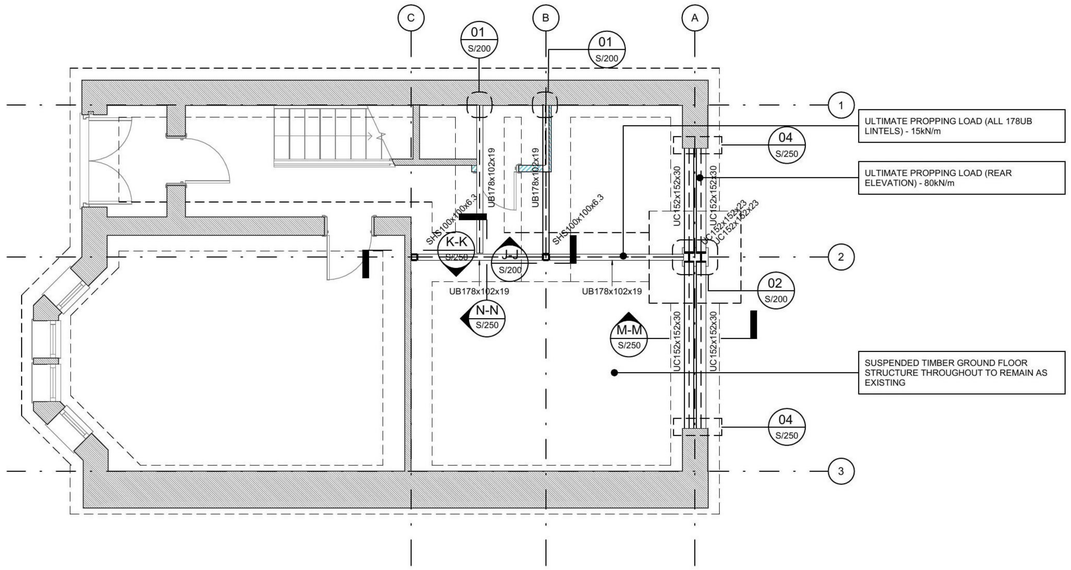

GROUND FLOOR LAYOUT

SCALE 1 : 50

SSL = REFER TO ARCHITECTS DRAWINGS

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5. REFER TO SCHEDULE 1 OF THE SER CERTIFICATE FOR CONFIRMATION OF THE CONTRACTOR DESIGNED ELEMENTS.

SLAB LEGEND

STRUCTURAL SLAB LEVEL . . . . . SSL 0.000

TIMBER & MASONRY LEGEND

DENOTES EXTENT OF 47x97mm C16 TIMBERS @ 600mm C/C INTERNAL PARTITIONS WITH 12.5mm PLASTERBOARD TO EACH FACE. REFER TO C&S SPECIFICATION FOR FURTHER DETAILS AND NAILING SCHEDULE.

DENOTES EXISTING WALLS

DE-BONDING JOINT NOTE:

ALL NEW BLOCKWORK JUNCTIONS WITH EXISTING MASONRY/STONE WALLS TO BE SECURED WITH ANCON STAIFIX SYSTEM FIXED IN ACCORDANCE WITH THE MANUFACTURERS GUIDELINES.

TIMBER POSTS FIXED TO EXPOSED FACE OF EXISTING WALL WITH HILTI HRD-C 8 PLASTIC FRAME ANCHORS AT MAX. 300mm CENTRES. ANCHORS TO ACHIEVE MINIMUM 65mm ANCHORAGE DEPTH. ALL TO MANUFACTURERS GUIDELINES.

BLOCKWORK ABOVE DPC:

OUTER LEAF TO BE FORMED WITH 100mm, 7.3N/mm2 BLOCKWORK WITH M4 (CLASS iii) MORTAR. INNER LEAF TO BE FORMED WITH 140mm, 7.3N/mm2 BLOCKWORK WITH M4 (CLASS iii) MORTAR U.N.O

CAVITY WIDTH TO ARCHITECTS SPECIFICATION

WALL TIE SPECIFICATION:

WALL TIES TO BE STAINLESS STEEL (CULLEN FT-50 OR SIMILAR) THAT COMPLIES TO TYPE 1 OR 2 OF DD140:PART2. WALL TIES SIZED TO ACHIEVE MINIMUM 60mm EMBEDMENT IN MORTAR COURSE.

FISH TIE AND BUTTERFLY TIES ARE NOT ACCEPTABLE UNDER ANY CIRCUMSTANCES.

WALL TIES TO BE SPACED AT 600mm CENTRES HORIZONTALLY AND 450mm CENTRES VERTICALLY IN A STAGGERED PATTERN.

THE VERTICAL EDGES OF OPENINGS AND ANY VERTICAL NON RETURNED, UNBONDED EDGES SHOULD HAVE WALL TIES PLACED AT 225mm VERTICAL CENTRES NON MORE THAN 200mm FROM EDGE OF OPENING.

NAILING SCHEDULE / SPECIFICATION:

REFER TO SF STRUCTURES SPECIFICATION FOR FULL NAILING & FIXING SCHEDULE AND FIXING SPECIFICATION

FIRE PROTECTION:

BUILDING FIRE RATING = 30mins

ALL STRUCTURAL ELEMENTS PROTECTED BY 1No. LAYER OF 12.5mm PLASTERBOARD.

LINTEL NOTE:

REFER TO SFS CIVIL & STRUCTURAL SPECIFICATION FOR SIZE AND NUMBER OF TIMBER LINTELS AND CORRESPONDING CRIPPLE STUDS.

BLOCKWORK MOVEMENT JOINTS:

REFER TO SF STRUCTURES CIVIL & STRUCTURAL SPECIFICATION 'EXTERNAL BLCOKWORK' AND CORRESPONDING DETAIL FOR MOVEMENT JOINT REQUIREMENTS.

POSITIONS AS NOTED ON ARCHITECTS DRAWINGS.

GLAZING DESIGN LOADS

ALL GLAZING LOWER THAN 1100mm FROM FINISHED FLOOR LEVEL TO BE DESIGNED IN ACCORDANCE WITH BARRIER LOADS FROM BS6180 (AND BS EN 1991-1-1) FOR 'DOMESTIC AND RESIDENTIAL ACTIVITIES'

HORIZONTAL UNIFORMLY DISTRIBUTED LINE LOAD = 0.36kN/m
UNIFORMLY DISTRIBUTED LOAD APPLIED TO INFIL = 0.50kN/m2
POINT LOAD APPLIED TO PART OF INFIL = 0.25kN

DESIGN CODES AS PER SF STRUCTURES GENERAL NOTES DRAWING

WIND LOAD AS PER SF STRUCTURES GENERAL NOTES DRAWING

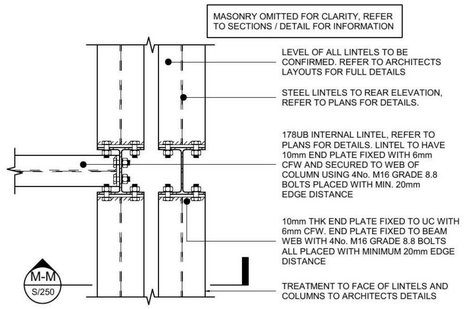
2No. 300MM LONG, TYPE E (100mm x 100mm) ROBESLEE LINTELS TO BE PLACED AS BEARING PADS. BEARING PADS TO BE PLACED ON BED OF 1:3 s/c MORTAR TO ENSURE FULL AND EQUAL BEARING ONTO EXISTING STONEWORK BELOW. ALL VOIDS AROUND STONEWORK TO BE PACKED WITH MORTAR.

STEEL UB / UC LINTEL, REFER TO PLANS FOR DETAILS. LINTEL TO ACHIEVE 200mm MIN BEARING ONTO BEARING PAD AND BUILT TIGHTLY INTO WALL USING EXISTING STONE OR ENGINEERING BRICKWORK. BRICKWORK CONSTRUCTED AROUND BEAM TO ENSURE FULL LATERAL STABILITY. ALL VOIDS TO BE PACKED WITH 1:3 S/C MORTAR. ALL LINTELS TO BE MEASURED ON SITE. FINISHES TO ARCHITECTS DETAILS

SIZE, STATE AND FORM OF EXISTING STONEWORK TO BE VISUALLY INSPECTED PRIOR TO LOADING TO ENSURE SUPPORTING STONEWORK BELOW NEW BEARING PAD IS SOUND AND STABLE. ALL VOIDS, LOOSE STONE AND STONES SMALLER THAN 100MM WIDE TO BE REMOVED AND REPLACED ENSURING ALL NEW STONEWORK IS ADEQUATELY TIED INTO THE EXISTING STONE WALL.

DETAIL 01 - SINGLE STEEL LINTEL BEARING ON STONE WALL (PERP.)

SCALE 1 : 10



LEVEL OF ALL LINTELS TO BE CONFIRMED. REFER TO ARCHITECTS LAYOUTS FOR FULL DETAILS

STEEL LINTELS TO REAR ELEVATION, REFER TO PLANS FOR DETAILS.

178UB INTERNAL LINTEL, REFER TO PLANS FOR DETAILS. LINTEL TO HAVE 10mm END PLATE FIXED WITH 6mm CFW AND SECURED TO WEB OF COLUMN USING 4No. M16 GRADE 8.8 BOLTS PLACED WITH MIN. 20mm EDGE DISTANCE

10mm THK END PLATE FIXED TO UC WITH 6mm CFW. END PLATE FIXED TO BEAM WEB WITH 4No. M16 GRADE 8.8 BOLTS ALL PLACED WITH MINIMUM 20mm EDGE DISTANCE

TREATMENT TO FACE OF LINTELS AND COLUMNS TO ARCHITECTS DETAILS

DETAIL 02 - STEEL LINTELS TO DOUBLE STEEL COLUMN DETAIL

SCALE 1 : 10

SECTION J-J - TRIPLE UB STEEL BEAM TO SHS POST CONNECTION DETAIL

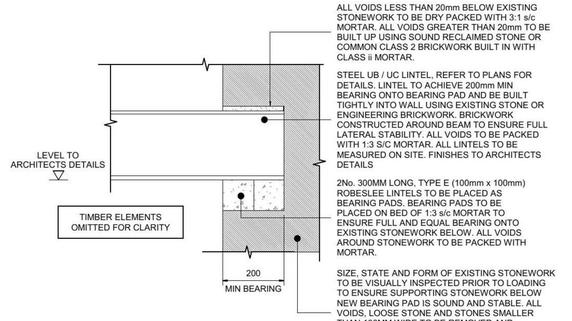
SCALE 1 : 5

DETAIL 03 - SHS POST STEEL ANGLE BRACKET

SCALE 1 : 5

SECTION H-H - SINGLE UB STEEL BEAM TO SHS POST CONNECTION DETAIL

SCALE 1 : 5



SECTION F-F - SINGLE STEEL LINTEL BEARING ON STONE WALL (PERP.)

SCALE 1 : 10

TIMBER LINTEL SCHEDULE			
LINTEL SPAN	**UPTO 1.25m**	**UPTO 1.8m**	**UPTO 2.475m**
LINTEL NUMBER / SIZE	2No. 47x197, C16's	3No. 47x197, C16's	3No. 47x245, C24's

TIMBER CRIPPLE STUD SCHEDULE, C16			
LINTEL SPAN	**UPTO 1.25m**	**UPTO 1.8m**	**UPTO 2.475m**
NUMBER OF 47x147, C16 STUDS	2	2	3

BLOCKWORK LINTEL SCHEDULE			
LINTEL / WALL THICKNESS	**UPTO 1.25m**	**UPTO 1.8m**	**UPTO 2.475m**
ROBESLEE LINTEL TYPE / 100mm THK	TYPE C	TYPE C	TYPE K9
ROBESLEE LINTEL TYPE / 140mm THK	TYPE F	TYPE F	TYPE G8

NOTE: LINTELS TO BE PLACED N ACCORDANCE WITH MANUFACTURERS GUIDELINES. CONSULT SF STRUCTURES WHERE EXTERNAL LEAF IS GREATER THAN 140mm

	RESIDUAL HAZARDS IDENTIFIED - SUPERSTRUCTURE
ITEM	DESCRIPTION
1	WORKING AT HEIGHT. CONTRACTOR TO ENSURE APPROPRIATE EDGE PROTECTION IS IN PLACE FOR ALL OPERATIVES AT ALL TIMES
2	TIMBER WALL PANELS POTENTIALLY UNSTABLE DURING THE CONSTRUCTION PHASE. CONTRACTOR TO TEMPORARY PROP PANELS DURING CONSTRUCTION

Status: TENDER

Client: MR & MRS BULL

Project: QUEEN VICTORIA DRIVE

Title: GROUND SLAB LAYOUT

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NOTES

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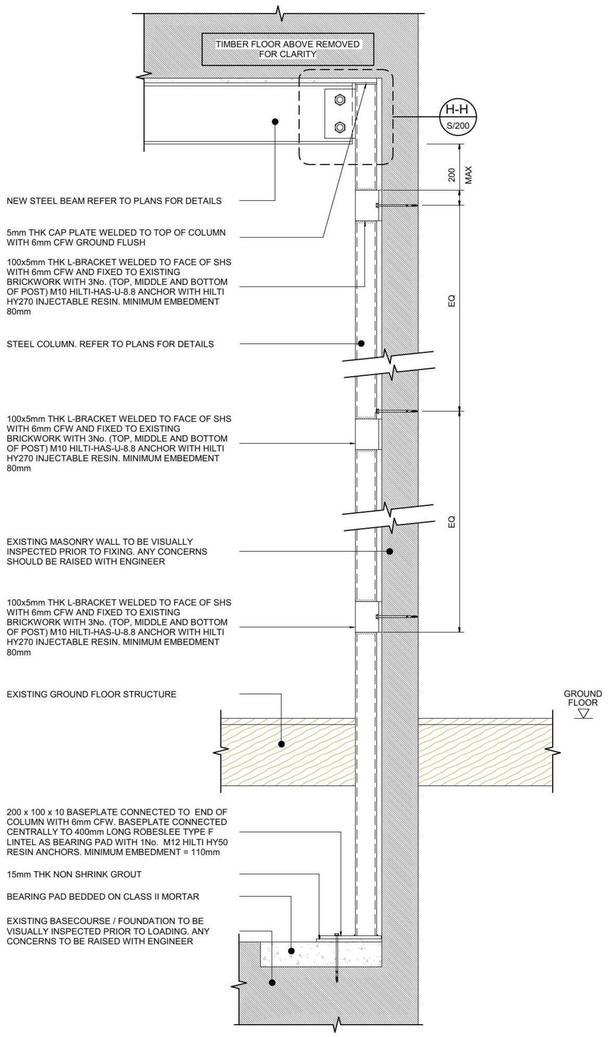
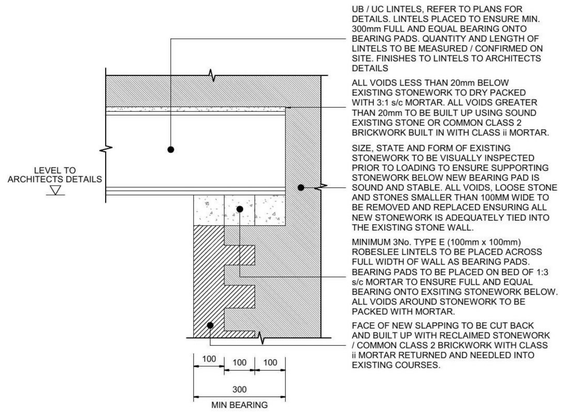


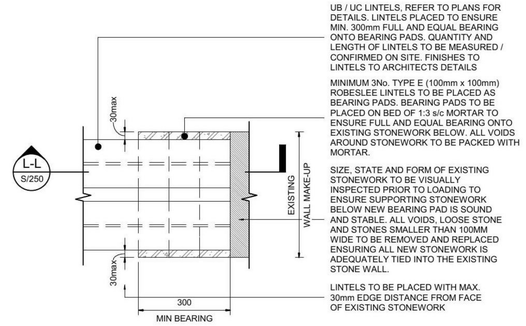
SECTION K-K - STEEL FRAME SLAPPING SUPPORT ON EXISTING WALL

SCALE 1 : 10



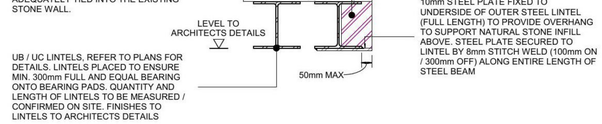
SECTION L-L - STEEL LINTELS BEARING ON STONE WALL (IN PLANE)

SCALE 1 : 10



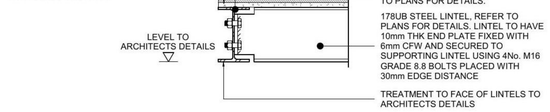
DETAIL 04 - STEEL LINTELS BEARING ON STONE WALL (IN PLANE)

SCALE 1 : 10



SECTION M-M - DOUBLE LINTEL WITH STEEL BASE PLATE OUTER LEAF

SCALE 1 : 10



SECTION N-N - 178UB STEEL LINTEL TO 178UB SUPPORT

SCALE 1 : 10

Status: TENDER

Client: MR & MRS BULL

Project: QUEEN VICTORIA DRIVE

Title: STRUCTURAL DETAILS SHEET 01

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