Project：Proposed Residential Development
Land West of A577 Neverstitch Road，Skelmersdale （Latham Pastures Phase 2）

Client：Bellway Homes（North West）Ltd
Document：Transport Assessment

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## 1 Introduction

## General

1.1 CBO Transport Ltd [CBO] has been commissioned by Bellway Homes (North West) Ltd to undertake an assessment of the transport matters associated with a proposed residential development at land to the west of the A577 Neverstitch Road, Skelmersdale, known as Lathom Pastures Phase 2.
1.2 This Transport Assessment represents a culmination of this work and is submitted with the planning application for the scheme. A Travel Plan Framework has also been prepared and submitted with the application.
1.3 The site is located to the east of the Lathom Pastures (Phase 1) development on land bound by Old Engine Lane to the north, the A577 Ormskirk Road to the south, the A577 Neverstitch Road to the east and the Lathom Pastures (Phasel) development to the west.
1.4 The proposals would see the site developed to deliver 200 residential units, made up of a mix of dwelling types and bedroom numbers.

## Background

1.5 The proposed site forms part of the wider Lathom Pastures development, which is an area of land that is allocated for residential development for approximately 400 dwellings in the West Lancashire Local Plan. A Development Brief was prepared for the land in August 2014.
1.6 Since the above Development Brief was produced, Bellway Homes has received planning permission for (and are currently constructing) the Lathom Pastures Phase 1 development on the site, comprising a total of 94 dwellings accessed entirely off a new access from Firswood Road.
1.7 Wain Homes also have a scheme for a further 130 dwellings on the area of the site located to the north of Old Engine Lane / the Bellway Phase 2 site which would also be accessed entirely from Firswood Road. This scheme received planning permission on 01st October 2020.

## Discussions with Lancashire County Council

With regard to the requirements of this report, CBO submitted a pre-application Highways and Transport
1.8 Note to, and undertook pre-application scoping discussions with, Lancashire County Council [LCC] Highway officers. This culminated in LCC providing written pre-application advice dated $2^{\text {nd }}$ June 2020. Where appropriate, this report takes on board and makes reference to the comments provided in this pre-application advice.

It has been agreed through this pre-application exercise that a full Transport Assessment is required in
1.9 this instance. LCC has also requested / agreed the following:

- That the primary access to the site be taken from the A577 Neverstitch Road. A construction access can also be provided from the A577 Neverstitch Road;
- That a table of amenities be included in the repot identifying amenities and their walk/cycle distances from the site (via safe/suitable paths and not crow-fly distances);
- That an assessment year of application plus 5 years is appropriate for considering the traffic impact on the local road network;
- That the Lathom Pastures Phase 1 scheme and the Wain Homes scheme be included as committed development;
- That the study area for traffic analysis include the site access from the A577 Neverstitch Road, together with the A577 Neverstitch Road / A577 Ormskirk Road / B5312 Railway Road / Witham Road / Ormskirk Road roundabout to the south.
1.10 LCC has also requested that a Travel Plan Framework be submitted with the application. This document has therefore been produced and is submitted separately with the application.


## Scope of Report

1.11 In light of the above, the purpose of this report is to provide LCC with the necessary information to support the proposals and consider their transport implications. In order to provide this information, this report has been produced in 7 sections including this introduction.
1.12 Section 2 considers planning policy, whilst Section 3 reviews existing conditions and provides details of the study area from a highways perspective. Section 4 then considers the accessibility of the site by the sustainable modes and linkage to the surrounding area.
1.13 Section 5 goes on to detail the development proposals, whilst Section 6 considers the traffic generations associated with the scheme.
1.14 Section 7 considers the off-site traffic impact of the scheme, before going on to provide details of the traffic flows used to undertake operational assessments, together with the findings of these assessments.
1.15 The conclusions and recommendations of the report are included in Section 8.

## 2 Planning Policy and Guidance

## National Planning Policy Framework

2.1 National planning policy in relation to transport matters is set out in the revised National Planning Policy Framework [NPPF] February 2019. Paragraph 10 of the NPPF states that:
"So that sustainable development is pursued in a positive way, at the heart of the Framework is a presumption in favour of sustainable development."
2.2 In relation to decision taking, it states at paragraph 11 (d) that permission should be granted unless "any adverse impacts of doing so would significantly and demonstrably outweigh the benefits, when assessed against the policies in this Framework taken as a whole".
2.3 With regard to transport, the policies are contained within paragraphs 102 to 111 at Section 9 and all relate to the promotion of sustainable transport. In considering development proposals, paragraph 108 states that:
"In assessing sites that may be allocated for development in plans, or specific applications for development, it should be ensured that:
a) appropriate opportunities to promote sustainable transport modes can be - or have been - taken up, given the type of development and its location;
b) safe and suitable access to the site can be achieved for all users; and
c) any significant impacts from the development on the transport network (in terms of capacity and congestion), or on highway safety, can be cost effectively mitigated to an acceptable degree"
2.4 Paragraph 109 then goes on to state that:
"Development should only be prevented or refused on highways grounds if there would be an unacceptable impact on highway safety, or the residual cumulative impacts on the road network would be severe"
2.5 Within this context, paragraph 110 states that applications should:
"a) give priority first to pedestrian and cycle movements, both within the scheme and with neighbouring areas; and second - so far as possible - to facilitating access to high quality public transport, with layouts that maximise the catchment area for bus or other public transport services, and appropriate facilities that encourage public transport use;

And
.....d) allow for the efficient delivery of goods, and access by service and emergency vehicles
2.6 Paragraph 111 then states that all "developments that will generate significant amounts of movement should be required to provide a travel plan, and the application should be supported by a transport statement or transport assessment so that the likely impacts of the proposal can be assessed".

## Planning Policy Guidance

2.7 Planning Policy Guidance was updated on the $6^{\text {th }}$ March 2014 in relation to "Travel plans, transport assessments and statements in decision-taking".
2.8 In considering the over-arching principles that should be taken into account in preparing a Transport Assessment, this guidance suggests they should be:

- "proportionate to the size and scope of the proposed development to which they relate and build on existing information wherever possible;
- established at the earliest practicable possible stage of a development proposal;
- be tailored to particular local;
- be brought forward through collaborative ongoing working between the Local Planning Authority/ Transport Authority, transport operators, Rail Network Operators, Highways Agency where there may be implications for the strategic road network and other relevant bodies."
2.9 This report accords with this guidance.


## Local Policy and Development Brief

2.10 As set out in the introduction, the site forms part of the wider Lathom Pastures development. This development sits in an area of land that is allocated for residential development for approximately 400 dwellings under Policy RSI (a) (iv) of the West Lancashire Local Plan.
2.11 This Policy states that:
"Development of sites (i) - (v) above should conform to masterplans or development briefs to be prepared for each site".
2.12 The Development Brief for this land was prepared in August 2014.
2.13 At page 11 under the heading 'Transport', this Development Brief states that:
"The main access to the Firswood Road development site should be taken off Neverstitch Road between Old Engine Lane and the Ormskirk Road roundabout, at a point where maximum visibility can be achieved"
2.14 And that:
"A minor secondary access to the site may be located on the southern part of Firswood Road (south of the bridge over the disused railway line)".
2.15 It then goes on to state that:
"If it is possible, the creation of a more significant secondary access directly onto Blaguegate Lane would be preferred, although it is acknowledged that this would be reliant on a landowner(s) outside of the site boundary being willing to sell their land".
2.16 Since the above Development Brief was produced, Bellway have received planning permission for (and are currently constructing) the Lathom Pastures Phase 1 development on the site, comprising a total of 94 dwellings accessed entirely off a new access from Firswood Road.
2.17 Wain Homes also have a scheme for a further 130 dwellings on the area of the site located to the north of Old Engine Lane / the Bellway Phase 2 site which would also be accessed entirely from Firswood Road. This scheme received planning permission on 01st October 2020.

## 3 Existing Conditions and Study Area

## Site Description

3.1 The site is located to the east of the Lathom Pastures (Phase 1) development on land bound by Old Engine Lane to the north, the A577 Ormskirk Road to the south, the A577 Neverstitch Road to the east and the Lathom Pastures (Phasel) development to the west. The location of the site is shown in Figure 3.1.
3.2 The site is made up of a number of open fields, some of which include vacant buildings. There is also a dismantled railway line running approximately east - west through the site from the A577 Neverstitch Road to the east to Firswood Road to the west.
3.3 Vehicular access to the site is currently provided at its north eastern corner via Old Engine Lane. This lane is a private, unadopted road which heads west from the A577 Neverstitch Road along the northern boundary of the site. This lane also forms public footpath number 26.
3.4 There is currently no vehicular access provided to the site from the A577 Neverstitch Road or Firswood Road.

## Highway Study Area and Observed Traffic Flows / Speeds

## Traffic Flows

3.5 Through discussions with LCC, it has been agreed that the study area in terms of considering traffic impact be made up of the following local junctions:

- The proposed site access from the A577 Neverstitch Road; and
- The A577 Neverstitch Road / A577 Ormskirk Road / B5312 Railway Road / Witham Road / Ormskirk Road roundabout to the south.
3.6 In order, therefore, to ascertain existing traffic flows at the junction identified above and past the proposed site, surveys were undertaken on two occasions.
3.7 Two automatic traffic counters [ATC] were placed outside and north of the site frontage on the A577 Neverstitch Road for a period of 7 days between Sunday 19th January and Saturday $25^{\text {th }}$ January 2020. The northern ATC was located just north of the A577 Neverstitch Road / Old Engine Lane junction. The southern ATC was located approximately 110 metres south of this junction / approximately 220 metres north of the A577 Neverstitch Road / A577 Ormskirk Road / B5312 Railway Road / Witham Road / Ormskirk Road roundabout.
3.8 Based on the above ATC surveys, the weekday morning and evening peak hours on the A577 Neverstitch Road past the site frontage were identified as being 8:00-9:00 and 15:00-16:00 respectively.
3.9 In addition to the above, a fully classified turning count was undertaken by an independent survey company on Tuesday 17 ${ }^{\text {th }}$ March 2020 between the hours 7:00-10:00 and 16:00-19:00 at the A577 Neverstitch Road / A577 Ormskirk Road / B5312 Railway Road / Witham Road / Ormskirk Road roundabout.
3.10 Based on the above fully classified turning count, the weekday morning and evening peak hours at the A577 Neverstitch Road / A577 Ormskirk Road / B5312 Railway Road / Witham Road / Ormskirk Road roundabout were identified as being 8:00-9:00 and 16:15-17:15 respectively.
3.11 In light of the above, Figure 3.2 shows the observed 2020 weekday morning peak traffic flows for the hour 8:00-9:00 at both junctions. Figure 3.2 also shows the observed 2020 weekday evening peak traffic flows for the hour 15:00-16:00 on the A577 Neverstitch Road past the site frontage and for the hour 16:15-17:15 at the A577 Neverstitch Road / A577 Ormskirk Road / B5312 Railway Road / Witham Road / Ormskirk Road roundabout.
3.12 With regard to the turning count at the A577 Neverstitch Road / A577 Ormskirk Road / B5312 Railway Road / Witham Road / Ormskirk Road roundabout, this was undertaken 6 days before the UK went into a national lockdown due to the COVID-19 pandemic. As a result and using the ATC results from January 2020 on the A577 Neverstitch Road, consideration has been given to whether this could have impacted on the resultant traffic flows. Table 3.1 below therefore sets out the flows observed on the A577 Neverstitch Road in March 2020 and compares these flows to those observed in January 2020.

Table 3.1: A577 Neverstitch Road Traffic Survey Comparisons

| Direction | 8:00-9:00 |  |  |  |  | 16:00-17:00 |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Tue $2{ }^{\text {st }}$ Jan 20 | 5 Day Ave Jan 20 | Tue $17^{\text {th }}$ <br> Mar 20 | Percentage Change |  | Tue $21^{\text {st }}$ Jan 20 | 5 Day Ave Jan 20 | Tue $17^{\text {th }}$ <br> Mar 20 | Percentage Change |  |
|  |  |  |  | Tue | 5 day |  |  |  | Tue | 5 day |
| Northbound | 325 | 320 | 320 | -1.5\% | 0\% | 383 | 405 | 339 | -11.5\% | -16.3\% |
| Southbound | 490 | 479 | 475 | -3.1\% | -0.8\% | 395 | 379 | 343 | -13.2\% | -9.5\% |

3.13 Based on the above, it is suggested that the COVID-19 pandemic was unlikely to be having an impact on weekday morning peak flows during the hour 8:00-9:00. During the weekday evening peak hour of 16:00-17:00, the above suggests the COVID-19 pandemic might have been having more of an impact.
3.14 In light of the above and for robustness, the observed traffic flows shown in Figure 3.2 for the A577 Neverstitch Road / A577 Ormskirk Road / B5312 Railway Road / Witham Road / Ormskirk Road roundabout have been increased by $5 \%$ during the weekday morning peak and by $20 \%$ during the weekday evening peak. These adjusted 2020 traffic flows are shown in Figure 3.3.

## Traffic Speeds

3.15 In addition to the above, the ATC's collected speed data on the A577 Neverstitch Road for the seven day survey period. A copy of the northbound data from the southern ATC and the southbound data from the northern ATC is included at Appendix A, whilst the average results over the 7 day period are set out below in Table 3.2. These datasets are considered appropriate for considering visibility provision in both directions at a site access from the A577 Neverstitch Road.

Table 3.2: A577 Neverstitch Road Speed Survey Results

|  | $85^{\text {th }}$ Percentile Speed (mph) |  |
| :--- | :--- | :--- |
|  | Northbound | Southbound |
| ATC 7 day raw data average: 24 hours | 43.5 mph | 42.1 mph |
| ATC 7 day 'average of average': 24 hours | $\mathbf{4 4 . 3 \mathrm { mph }}$ | $\mathbf{4 2 . 8 \mathrm { mph }}$ |
| ATC 7 day raw data average: $10: 00-12: 00$ | 43.4 mph | 41.4 mph |
| ATC 7 day raw data average: $14: 00-16: 00$ | 43.5 mph | 43.0 mph |
| ATC 7 day 'average of average': 7:00 - 19:00 | 43.4 mph | 41.7 mph |
| ATC 7 day 'average of average': $6: 00-22: 00$ | 43.4 mph | 42.1 mph |

3.16 Based on the above, the $85^{\text {th }}$ percentile speeds derived from the 'average of average' 7 day, 24 hour data has been used for the consideration of visibility requirements at any A577 Neverstitch Road access, namely 44.3 mph northbound and 42.8 mph southbound.

## Highway Network

A577 Neverstitch Road
3.17 The A577 Neverstitch Road runs along the eastern boundary of the proposed site. As it passes the site, the route is in the order of 7.3 metres in width and curves on a circa 180 metre centreline radius bend, with the site being located on the outside of this bend.
3.18 At the south east corner of the site, the A577 Neverstitch Road passes through a circa 70 metre inscribed circle diameter [ICD] 5 arm roundabout at the junction of the A577 Neverstitch Road / A577 Ormskirk Road / B5312 Railway Road / Witham Road / Ormskirk Road. Approximately 550 metres to the northeast of this roundabout (i.e. approximately 200 metres northeast of the northern site boundary), the A577 Neverstitch Road passes through a circa 70 metre ICD 4 arm roundabout at the junction of the A577 Neverstitch Road / School Lane / a private access.
3.19 This 550 metre section of the A577 Neverstitch Road is lit and subject to a 40 mph speed limit.
3.20 Just to the north of the site and approximately 200 metres southwest of the northern roundabout referred to above, Old Engine Lane joins the A577 Neverstitch Road as the minor arm of a simple priority junction.
3.21 Considering the wider context and after passing through the A577 Neverstitch Road / School Lane / a private access roundabout to the north east, the A577 continues to a roundabout junction with the A5068 and Glenburn Road. From this junction, the A5068 heads south into Skelmersdale, whilst Glenburn Road heads north and provides access to the A5209.

## A577 Ormskirk Road

3.22 The A577 Ormskirk Road heads west from the A577 Neverstitch Road / A577 Ormskirk Road / B5312 Railway Road / Witham Road / Ormskirk Road roundabout referred to above and passes a circa 10 metre length of site frontage which starts approximately 60 metres west of the roundabout. As it passes this site frontage, the A577 Ormskirk Road is in the order of 6.5 metres in width, lit and subject to a 30 mph speed limit.
3.23 The A577 Ormskirk Road (which becomes Blaguegate Lane) then heads west for a distance of approximately 550 metres, where it is joined by Firswood Road. From here, the A577 continues west into Ormskirk.

## B5312 Railway Road

3.24 The B5312 Railway Road heads south from the A577 Neverstitch Road / A577 Ormskirk Road / B5312 Railway Road / Witham Road / Ormskirk Road roundabout referred to above. It then continues around the south side of Skelmersdale and, via the A5068, provides access to the M58 at Junction 4.
3.25 Approximately 400 metres south of the A577 Neverstitch Road / A577 Ormskirk Road / B5312 Railway Road / Witham Road / Ormskirk Road roundabout, the B5312 Railway Road is joined by the B5312 Liverpool Road via a three arm mini roundabout arrangement. From this junction, the B5312 Liverpool Road heads west to the A570, which provides access to the M58 Junction 3 before heading south to St Helens.

Summary
3.26 Based on the above, the site is considered to be well connected to the local highway network.

## Highway Conditions

3.27 Morning peak period observations were undertaken in January 2020. These showed that flows along the A577 Neverstitch Road and A577 Ormskirk Road were modest and that there was no evidence of any delay or queuing at the A577 Neverstitch Road / A577 Ormskirk Road / B5312 Railway Road / Witham Road / Ormskirk Road roundabout to the south of the site or at the A577 Neverstitch Road / School Lane / a private access roundabout to the north of the site. There was also no evidence of any notable delay or queuing at the junctions to the north, south, east and west of these junctions serving the wider Skelmersdale area.

## Personal Injury Accident Record

3.28 Accident data that is freely available on LCC's MARIO website for the study area agreed with LCC shows there has been one personal injury accident recorded on the 550 metre section of the A577 Neverstitch Road past the site.
3.29 This accident occurred approximately 110 metres south of the A577 Neverstitch Road / Old Engine Lane junction in January 2016. The accident was slight in nature and did not involve a pedestrian or a cyclist.
3.30 Considering the A577 Neverstitch Road / A577 Ormskirk Road / B5312 Railway Road / Witham Road / Ormskirk Road roundabout, this freely available information shows that there has been a total of 6 accidents recorded, none of which involved a pedestrian or a cyclist.
3.31 Three of these accidents occurred on the roundabout between the A577 Neverstitch Road and Oldham Road. These accidents occurred in July 2016, April 2017 and November 2017 and were all slight in nature.
3.32 The remaining three accidents occurred on Witham Road, the B5312 Railway Road exit from the roundabout and the B5312 Railway Road approach. The accident on the B5312 Railway Road roundabout exit occurred in December 2015 and was a serious injury accident. The accidents on Witham Road and the B5312 Railway Road approach occurred in May 2014 and January 2014 respectively and were both sight injury accidents.

## Summary

3.33 Based on the above, there is no notable accident issue along the A577 Neverstitch Road which the introduction of a new access to the proposed development would exacerbate.
3.34 With regard to the A577 Neverstitch Road / A577 Ormskirk Road / B5312 Railway Road / Witham Road / Ormskirk Road roundabout and taking account of the level of traffic flow associated with the proposed development and its impact as set out later in this report, it is also the case that there is are no notable accident issues within the study area linked to highway layout which traffic from the proposed development would exacerbate.

## 4 Accessibility by the Sustainable Modes

## Pedestrian \& Cycle Infrastrucłure

## Existing

4.1 The A577 Neverstitch Road does not include any pedestrian facilities along the site frontage. However, there is a short section of footway on the south side of the A577 Neverstitch Road approximately 60 metres north of Old Engine Lane which links to a westbound / southbound bus stop and the local pedestrian network to the east / Skelmersdale town centre via Turnberry. There is also an underpass in this location which provides a link to an eastbound / northbound bus stop on the opposite side of the A577 Neverstitch Road.
4.2 To the south, there are footways provided across the A577 Neverstitch Road arm of the A577 Neverstitch Road / A577 Ormskirk Road / B5312 Railway Road / Witham Road / Ormskirk Road roundabout, which link to footways on all other arms of the roundabout. From this roundabout, pedestrians can access the Skelmersdale area to the east, together with facilities along the A577 Ormskirk Road to the west.
4.3 In terms of cycle provision, there are no dedicated facilities in the immediate vicinity of the site. However, the local highway network is considered to be conducive to cycle use.

## Proposed

4.4 As set out later in Section 5, it is proposed as part of the development to fill the gaps in the local pedestrian network referred to above to provide:

- A footway on the west side of the A577 Neverstitch Road running from the existing Neverstitch Road footway at the A577 Neverstitch Road / A577 Ormskirk Road / B5312 Railway Road / Witham Road / Ormskirk Road roundabout to a point circa 70 metres southwest of Old Engine Lane;
- A pedestrian crossing point on the A577 Neverstitch Road located approximately 70 metres southwest of Old Engine Lane;
- A footway on the east side of the A577 Neverstitch Road running from the above crossing point to the existing footway on the south side of the A77 Neverstitch Road;
- Pedestrian links to this new footway from the development at the end of the dismantled railway line, the new site access from the A577 Neverstitch Road and opposite the new pedestrian crossing point;
- A pedestrian link from the development to the existing footway on the A577 Ormskirk Road.
4.5 These provisions will link the development to the surrounding pedestrian network.


## Accessibility for Pedestrians

4.6 Considering the wider accessibility for pedestrians, it is commonly accepted that walking has the greatest potential to replace short car trips, particularly those under two kilometres, whilst the Institution of Highways and Transportation "Guidelines for Providing for Journeys on Foot" states that "Walking accounts for over a quarter of all journeys and four fifths of journeys less than one mile".
4.7 The "Guidelines for Providing for Journeys on Foot" also includes a table which suggests that 800 metres is an acceptable maximum walking distance in a town centre and that 1.2 km is an acceptable maximum walking distance elsewhere. It also suggests a distance of 2 km is an acceptable maximum for commuter and education journeys, although statutory guidance indicates that just over 3 km is an acceptable walk distance for primary school pupils, with secondary school pupils being expected to walk up to 5 km .
4.8 In the context of the above, Figure 4.1 shows the areas, services and facilities within these catchments. These are discussed below, with the distances quoted being based on walking route distances from an approximate centre of the site via available routes and not distances as the crow files.

## Local Schools

4.9 At approximately 700 metres and 800 metres respectively from the site, Figure 4.1 shows Broomfield Primary School and St. Richard's Catholic Primary School are the closest to the site and within the 1.2 km catchment. There are also two primary schools within the 2 km catchment, namely Skelmersdale Trinity C of E / Methodist Primary School and Crow Orchard Primary School, which are 1.5 km and 1.7 km respectively from the site.
4.10 The above schools are therefore all within the 2 km distance deemed acceptable by the IHT guidelines and well within the 3 km statutory walk distance. In addition and as also shown in Figure 4.1, St. Edmund's Catholic Primary School falls just outside the 2 km catchment but, at a 2.1 km walk, would still be accessible on foot.
4.11 With regard to high schools and as shown in Figure 4.1, West Lancashire Community High School is located 500 metres to the east of the site. This school is therefore within the 2 km distance deemed acceptable by the IHT guidelines and well within the 5 km statutory walk distance.
4.12 In the context of the above, it is suggested that the site is well placed to allow future pupils to walk to and from school.

## Convenience Stores and Local Shops

4.13 As shown in Figure 4.1 and approximately 600 metres from the site, there is a convenience store located on the east side of the A577 Neverstitch Road / A577 Ormskirk Road / B5312 Railway Road / Witham Road / Ormskirk Road roundabout. In addition and approximately 800 metres from the site, and as also shown in Figure 4.1, there is convenience store located at the southern end of Sandy Lane. This area forms a local centre which includes a cluster of shops including a pharmacy, a betting shop and a takeaway.
4.14 These provisions allow for 'convenience' shopping within an acceptable walk distance of the proposed site for future residents.

## Post office

4.15 As shown in Figure 4.1, the closest Post Office to the site is located in the Sandy Lane area referred to above. At around 800 metres from the site, it is accessible on foot.

Doctors / Health Facilities
4.16 As shown in Figure 4.1, Sandy Lane Health Centre is located to the south east of the site, again in the Sandy Lane area referred to above. At around 850 metres from the site, it is considered that there is access to health facilities within walking distance.
Local Employment Areas
4.17 As shown in Figure 4.1, Stanley Industrial Estate and West Gillibrand Industrial Estate are within a 2 km walking catchment. There is therefore considered to be access to local employment areas within walking distance of the site.

Leisure Facilities
4.18 As shown in Figure 4.1, JMO Sports Park is located approximately 1 km to the south of the site. There is therefore considered to be access to local leisure facilities within walking distance of the site.

## Bus Stops

4.19 As shown in Figure 4.1, the closest bus stops to the scheme are located on the A577 Neverstitch Road and A577 Ormskirk Road. The stops on the A577 Neverstitch Road are located approximately 350 metres from the site for the eastbound stop and approximately 220 metres from the site for the westbound stop. The stops on the A577 Ormskirk Road are located approximately 600 metres from the site for the eastbound stop and approximately 550 metres from the site for the westbound stop. As set out later in this section, these stops are considered to be accessible from the site.

## Summary

4.20 The above demonstrates that the site is accessible for pedestrians, with walking representing a realistic mode of transport for future residents undertaking educational journeys and those making use of the various local shops, facilities and employment areas in the local area. The site also offers the opportunity for future residents to connect to the local bus network on foot.

## Accessibility for Cyclists

4.21 Considering the accessibility of the site by bicycle, it is commonly accepted that cycling also has the potential to substitute for short car trips, particularly those under five kilometres, and to form part of a longer journey by public transport.
4.22 As can be seen from Figure 4.2, the 5 km cycling catchment for the site takes in the whole of the Skelmersdale area, the Lathom and Westhead areas to the west and part of the large employment area to the south of the M58. Whilst outside the 5 km catchment, Ormskirk and Edge Hill University are around a 5 to 6 km ride from the site and could therefore be accessible by bike for some future residents.
4.23 In addition, Ormskirk railway station is around a 6 km cycle ride from the site and could also be accessible by bike for some future residents of the site.
4.24 The site is therefore well placed for residents to travel by bicycle for educational, employment, retail and leisure trips. Future residents could also travel by bicycle to access the local rail network.

## Summary of Amenities withing Walking / Cycling Distance

4.25 Based on the findings set out above and as requested by LCC, Table 4.1 below summarises the local amenities and their walk / cycle distance from the site.
Table 4.1: Local Amenities and Their Walk / Cycle Distance from the Proposed Site

| Amenity | Distance |
| :--- | :--- |
| Broomfield Primary School | 700 m |
| St. Richard's Catholic Primary School | 800 m |
| Skelmersdale Trinity C of E / Methodist Primary School | 1.5 km |
| Crow Orchard Primary School | 1.7 km |
| West Lancashire Community High School | 500 m |
| Convenience store: A577 / Witham Road roundabout | 600 m |
| Convenience store: Sandy Lane | 800 m |
| Post Office | 800 m |
| Local shops / facilities at Sandy Lane | Circa 800m |
| Sandy Lane Health Centre | 850 m |
| Pharmacy | 850 m |
| Local Employment area: Stanley Industrial Estate | Within 2km |
| Local employment area: West Gillibrand Industrial Estate | Within 2km |
| Sports Park | 1 km |
| Bus stops | $220 \mathrm{~m}-600 \mathrm{~m}$ |
| Ormskirk railway station | Circa 6km |

## Accessibility by Public Transport

4.26 As shown in Figure 4.1, the closest bus stops to the scheme are located on the A577 Neverstitch Road and A577 Ormskirk Road. The stops on the A577 Neverstitch Road are located approximately 350 metres from the site for the eastbound stop and approximately 220 metres from the site for the westbound stop. The stops on the A577 Ormskirk Road are located approximately 600 metres from the site for the eastbound stop and approximately 550 metres from the site for the westbound stop.
4.27 Based on these distances, the stops on the A577 Neverstitch Road are within the recognised 400 metre walk distance, whilst the stops on the A577 Ormskirk Road fall around 200 metres outside this distance. However, at total 7 to 8 minute walk time to the stops on the A577 Ormskirk Road, it is considered that this additional circa 200 metre walk distance would not deter future residents from using these stops.
4.28 In light of the above, Table 4.2 below shows the buses serving the site.

Table 4.2: Local Bus Services Serving the Proposed Site

| No. | Route | Frequency |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | AM Peak | Off Peak | PM <br> Peak | Sat | Sun |
| A577 Neverstitch Road |  |  |  |  |  |  |
| 310 | Skelmersdale - Ormskirk - Aughton Maghull - Aintree - Walton - Liverpool | Every 30mins | Every 30mins | Every 30mins | Every 30mins | Hourly |
| 375 / 385 | Southport - Ormskirk - Skelmersdale Wigan | Every 30mins | Every 30mins | Every 30mins | Every 30mins | Hourly |
| A577 Ormskirk Road |  |  |  |  |  |  |
| 310 | As above |  |  |  |  |  |
| 375 / 385 | As above |  |  |  |  |  |

4.29 As can be seen from the above, the 310 and $375 / 385$ buses passing the site offer frequent services to local towns. With journey times in the order of 25 to 30 minutes to Ormskirk, 40 minutes to Wigan and 70 minutes to Southport, these buses offer an opportunity for travel by bus to local employment, retail and leisure destinations.
4.30 The site is therefore connected to the local bus network and offers opportunities for future residents to travel via this mode for a variety of journey purposes.
Rail
4.31 As previously identified, Ormskirk railway station is around a 6 km cycle ride from the site and could be accessible by bicycle for some future residents of the site.
4.32 Via the Merseyrail Northern line, Ormskirk station provides access to regular services between Ormskirk and Hunts Cross, calling at stations in between including Aintree, Liverpool Central and Liverpool South. Northern Rail also operates a regular service between Ormskirk and Preston, calling a stations in between including Burscough Junction.

## 5 Development Proposals

## Development Proposals

5.1 The proposals include the development of a new residential scheme. The scheme will provide a total of 200 residential units, made up of a mix of dwelling types and bedroom numbers as follows:

- 10 No. 1 bed apartments;
- 12 No. 2 bed apartments;
- 9 No. 2 bed houses;
- 115 No. 3 bed houses; and
- 54 No. 4 bed houses.
5.2 All of these dwellings would be served by a new access from the A577 Neverstitch Road.
5.3 The proposed site layout for the scheme is reproduced at Appendix $\mathbf{B}$.


## Site Access

5.4 As shown on the proposed site layout at Appendix B, access to the site would be provided via a new priority junction arrangement from the A577 Neverstitch Road. This access arrangement is also shown in more detail on CBO drawing number CBO-0603-002 Rev D at Appendix C, whilst the below sets out how this access arrangement has been derived.

## Access geometry

5.5 Given that the A577 Neverstitch Road is an 'A' class road that is subject to a 40 mph speed limit, and that the observed $85^{\text {th }}$ percentile speeds referred to in Section 3 are comparable to the design speed $(43.75 \mathrm{mph})$ for such a speed limit, it is considered that the design standards set out in the Design Manual for Roads \& Bridges [DMRB] are appropriate in this location. These are also the design standards referred to in the Development Brief for the site.
5.6 Again given the observed speeds on the A577 Neverstitch Road, and giving due cognisance to its "A road" classification, the bend in the route which restricts forward visibility and the scale of development, it is considered that a 'ghost island' priority junction is appropriate in this instance.
5.7 With regard to the geometry of this ghost island arrangement and again based on the status and alignment of the A577 Neverstitch Road, it is considered appropriate to provide through lanes at the maximum 3.65 metre width and the right turn lane at the desirable 3.5 metre width. It is also considered that the length of the right turn lane should be provided based on the 40 mph speed limit design speed of 43.75 mph . With regard to the minor arm (i.e. the site access), it is considered that this should join the A577 Neverstitch Road via 10 metre radii.

## Visibility provision

5.8 With regard to visibility provision at the access, DMRB indicates that a ' $y$ ' distance of 120 metres is required for a design speed of 43.75 mph based on a 40 mph speed limit. Applying the requirements of DMRB to the observed $85^{\text {th }}$ percentile speeds set out in Table 3.2, the observed 44.3 mph northbound speed requires a ' $y$ ' distance of 122 metres, whilst the observed 42.8 mph southbound speed requires a ' $y$ ' distance of 115 metres.
5.9 In terms of the ' $x$ ' distance, a 2.4 metre provision is considered sufficient.

## Wain Homes Scheme

5.10 In addition to the above, there is an improvement proposed as part of the Wain Homes development that would see a pedestrian crossing with a refuge island and an element of carriageway widening implemented to the north of the A577 Neverstitch Road / Old Engine Lane junction. It is therefore considered appropriate to take due cognisance of this improvement as part of any access proposals.

## Proposed access

5.11 Based on the above, drawing CBO-0603-002 Rev D at Appendix C shows the proposed access arrangement for the development. This shows a 5.5 metre wide access with 2 metre footways to both sides located approximately 130 metres southwest of the A577 Neverstitch Road / Old Engine Lane junction, together with widening of the A577 Neverstitch Road on the western side and within the adopted highway / proposed site.
5.12 Drawing CBO-0603-002 Rev D also shows that, with the access positioned in the proposed location, the works associated with the Wain Homes pedestrian improvement could be implemented.
5.13 With regard to visibility, drawing CBO-0603-002 Rev D shows the necessary provisions can be made for vehicles leaving the access.
5.14 Considering drivers travelling south on the A577 Neverstitch Road seeing vehicles waiting to turn right into the site access, drawing CBO-0603-002 Rev D shows a 107.5 metre forward visibility provision is achievable, which equates to a design speed of 41.15 mph .
5.15 Although this design speed is circa 1.5 mph below the observed 24 hour $85^{\text {th }}$ percentile southbound speed, it is only 0.5 mph below the 7:00 to 19:00 observed $85^{\text {th }}$ percentile speed ( 41.7 mph ) and just under 1 mph below the $6: 00$ to $22: 00$ observed $85^{\text {th }}$ percentile speed $(42.1 \mathrm{mph})$ shown in Table 3.2 . Furthermore, it is the case that these right turning vehicles would not impede through traffic and that any vehicle approaching and turning right would be slowing down in the deceleration lane in any event to turn into the access. It is therefore considered reasonable to accept this small departure in relation to the forward visibility provision on the southbound approach to the proposed access.
5.16 In terms of drivers waiting to turn right into the access seeing vehicles approaching from the south, drawing CBO-0603-002 Rev D shows a 106.1 metre forward visibility provision is achievable to a point 1 metre from the nearside kerbline, which equates to an approach speed of 40.9 mph . Measuring to the centreline, a provision of 116.5 metres is achievable, whilst a provision of 122 metres is achievable to a point 1 metre into the northbound lane. These provisions equate to approach speeds of 43.0 mph and 44.3 mph respectively.
5.17 With the central island shown on drawing CBO-0603-002 Rev $D$ to the south of the access, it is appropriate to measure to the centreline / the centre of the northbound carriageway on the basis that approaching vehicles would not be overtaking. Adopting this approach, the 43.0 mph speed associated with the forward visibility provision to the centreline is 1.3 mph below the observed 24 hour $85^{\text {th }}$ percentile northbound speed. However, the speed associated with the forward visibility to the centreline ( 43.0 mph ) is comparable to the northbound speeds for all other time periods set out in Table 3.2 ( 43.4 mph to 43.5 mph ), whilst the forward visibility to the centre of the northbound lane is appropriate for the 44.3 mph approach speed. It is therefore considered reasonable to accept this small departure in relation to the forward visibility provision for vehicles turning right into the access seeing oncoming vehicles.
5.18 In light of the above and in relation to the Development Brief for the site, it is considered that the proposals provide an access from the A577 Neverstitch Road "at a point where maximum visibility can be achieved" and that they therefore accord with the requirements of the brief.

## Swept Path Analysis

5.19 Following the receipt of confirmation from LCC regarding the required vehicle dimensions, swept path analysis has been carried out based on a 9.9 m long, 3 axle Econic refuse vehicle entering and exiting the proposed access at 10 mph . The findings of these swept path analyses are included in Figures 5.1 and 5.2 , which show that this refuse vehicle can suitably negotiate the access.

## Summary

5.20 Based on the above, the proposed access arrangements from the A577 Neverstitch Road are considered acceptable and appropriate from a design, safety and operational perspective. The principle of these access arrangements has also been agreed with LCC through the pre-application process.

## Construction Access

5.2 It is proposed to provide a construction access in the early stages of the delivery of the development in the location of the turning head outside plots 190 and 195 of the scheme layout at Appendix B . It is envisaged that this construction access would sit next to the main site access discussed above. Drawing CBO-0603-003 Rev A at Appendix C therefore shows the achievable visibility provision at a construction access in this location.
5.22 As can be seen from Drawing CBO-0603-003 Rev A, a large construction vehicle could wait in the hatching formed by the main site access when wating to turn into the construction access. There is, therefore, no issue with drivers travelling south on the A577 Neverstitch Road seeing these waiting vehicles.
5.23 With regard to visibility from the construction access, drawing CBO-0603-003 Rev A shows the necessary provisions can be made for vehicles leaving the access
5.24 In terms of construction vehicle drivers waiting to turn right into the construction access seeing vehicles approaching from the south, drawing CBO-0603-003 Rev A shows a 114.5 forward visibility provision is achievable when measuring to the centreline. This provision equates to approach speeds of 42.6 mph .
5.25 The 42.6 mph speed associated with the forward visibility provision to the centreline is just 0.8 to 0.9 mph below the observed working day $85^{\text {th }}$ percentile northbound speeds. As with the main site access, it is considered reasonable to accept this small departure in relation to the forward visibility provision for vehicles turning right into the construction access seeing oncoming vehicles.

## Summary

5.26 Based on the above, the proposed construction access arrangement from the A577 Neverstitch Road is considered acceptable. LCC has also indicated that they would not be averse to such a construction access.

## Internal Site Layout

5.27 Within the site and as shown on the proposed site layout at Appendix B, the new access from the A577 Neverstitch Road would continue west for a distance of approximately 65 metres. At this point, the access would form a T junction arrangement, with routes heading north and south.
5.28 Approximately 30 metres north of this $T$ junction arrangement, the access road would form another $T$ junction, with a cul-de-sac heading north and the main access heading west, with this main access serving a loop road arrangement and a number of cul-de-sacs.
5.29 Approximately 100 metres south of the first $T$ junction arrangement, the access road would form another T junction, with a cul-de-sac heading northwest and the main access heading southeast, with this main access serving a loop road arrangement and a cul-de-sac.
5.30 All of the above routes would be 5.5 metres in width with 2 metre footways to both sides.
5.31 With regard to servicing and access by refuse vehicles, the layout has been tracked using a 9.9 m long, 3 axle Econic refuse vehicle. Figures 5.3 and 5.4 show this vehicle can negotiate the streets within the scheme, whilst Figures 5.5 to 5.7 show this vehicle negotiating the turning heads within the site.

## Pedestrian and Cycle Provision

5.32 As set out above, access to the site would be provided via a new priority junction arrangement from the A577 Neverstitch Road. This access would include 2 metre wide footways to both sides. These footways would continue throughout the site on both sides of the highway network.
5.33 In addition to these internal provisions and as set out in Section 4, and as shown on the proposed site layout at Appendix B and the access drawing CBO drawing number CBO-0603-002 Rev D at Appendix C, there are a number of new pedestrian facilities proposed as part of the development to link the site to the surrounding network. These new facilities include:

- A footway on the west side of the A577 Neverstitch Road running from the existing Neverstitch Road footway at the A577 Neverstitch Road / A577 Ormskirk Road / B5312 Railway Road / Witham Road / Ormskirk Road roundabout to a point circa 70 metres southwest of Old Engine Lane;
- A pedestrian crossing point on the A577 Neverstitch Road located approximately 70 metres southwest of Old Engine Lane;
- A footway on the east side of the A577 Neverstitch Road running from the above crossing point to the existing footway on the south side of the A77 Neverstitch Road;
- Pedestrian links to this new footway from the development at the end of the dismantled railway line, the new site access from the A577 Neverstitch Road and opposite the new pedestrian crossing point;
- A pedestrian link from the development to the existing footway on the A577 Ormskirk Road.
5.34 With regard to cycle provision, the internal site layout is considered to be conducive to cycle use.


## 6 Traffic Generation and Assignment

## TRICS Derived Trip Rates

6.1 In order to determine the trip rates associated with the proposed residential development, the TRICS database has been interrogated for the sub land use of 'Houses Privately Owned'. To achieve a broad dataset, sites from London and Ireland have been removed and the sites chosen based on a 100 to 300 dwelling range. Multi modal sites have also been utilised, whilst sites including bungalows and low average bed numbers / rates have been removed. TRICS outputs for this interrogation are included in Appendix D.
6.2 Based on this methodology the resultant trip rates for the traditional 8:00-9:00 weekday morning and 17:00-18:00 weekday evening peak hours are shown below in Table 6.1. In addition, Table 6.1 also reproduces the trip rates used and agreed for the previously referred to Lathom Pastures Phase 1 development and the Wain Homes application.

Table 6.1: Proposed Development Trip Rates

|  | Weekday Morning Peak |  |  | Weekday Evening Peak |  |  |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- |
|  | Arrivals | Departures | Total | Arrivals | Departures | Total |
| TRICS Rates | 0.152 | 0.387 | 0.539 | 0.352 | 0.154 | 0.506 |
| 'Agreed' Rates | 0.153 | 0.427 | 0.580 | 0.396 | 0.238 | 0.634 |

6.3 As can be seen from the above, the trip rates derived from the TRICS interrogation are comparable to those previously agreed during the morning peak. For the evening peak, the previously agreed rates are notably higher. As a result and for robustness, the previously agreed trip rates have been used to consider the traffic generation associated with the proposed development. This approach has been agreed with LCC.
6.4 It should be noted that, whilst the referred to trip rates correspond to the traditional 8:00-9:00 weekday morning and 17:00-18:00 weekday evening peak hours, as set out in Section 3 the network evening peak occurs earlier in the afternoon, namely at 15:00-16:00 on the A577 Neverstitch Road past the site and at 16:15-17:15 at the A577 Neverstitch Road / A577 Ormskirk Road / B5312 Railway Road / Witham Road / Ormskirk Road roundabout. It is also the case that no allowance has been made in the above trip rates for the lower trip rates that would be associated with the apartment element of the scheme.
6.5 The application of the previously agreed trip rates therefore represents a robust approach.

## Resultant Traffic Generation

6.6 Applying the above trip rates to the proposed 200 dwelling development, Table 6.2 below sets out the resultant traffic generation.

Table 6.2: Proposed Development Traffic Generations

|  | Weekday Morning Peak |  | Weekday Evening Peak |  |  |  |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- |
|  | Arrivals | Departures | Total | Arrivals | Departures | Total |
| Traffic Generation | 31 | 85 | 116 | 79 | 48 | 127 |

## Traffic Distribution and Assignment

6.7 Trip distribution has been based on the 2011 census journey to work [JTW] information using journeys to work made by car drivers. The distribution is based on data at a middle super output area level [MSOA].
6.8 Two MSOAs have been used in the analysis, West Lancashire 009 and 011 . Whilst the site lies just to the west of these two areas, they are considered to provide the best approximation of the JTW pattern from the site. Work destinations from these two areas have been amalgamated to calculate the work based trip distributions and assignment for residential traffic leaving the site to travel to work.
6.9 Having determined the destination of car driver based journeys, these journeys have then been manually assigned to the study network based on the following routes / destinations:

- Route 1: South along the A577 Neverstitch Road then west along the A577 Ormskirk Road;
- Route 2: South along the A577 Neverstitch Road / B5312 Railway Road then east towards Skelmersdale;
- Route 3: South along the A577 Neverstitch Road / B5312 Railway Road then west on B5312 Liverpool Road towards M58 Jct 3:
- Route 4: North along the A577 Neverstitch Road then north at Half Mile Island;
- Route 5: North along the A577 Neverstitch Road then east at Half Mile Island;
- Route 6: North along the A577 Neverstitch Road then south at Half Mile Island.
6.10 The destination and origin areas associated with the above routes are shown in the information included at Appendix $\mathbf{E}$, which provides the raw ward data used to derive the distribution.
6.11 Based on the methodology detailed above, Table 6.3 below shows the percentage of development related traffic assigned to each of the six routes.

Table 6.3: Trip Distribution and Assignment

| Route | Origin / Destination | Percentage |
| :--- | :--- | :--- |
| 1 | South along the A577 Neverstitch Road then west along the A577 Ormskirk <br> Road | $22 \%$ |
| 2 | South along the A577 Neverstitch Road / B5312 Railway Road then east <br> towards Skelmersdale | $31 \%$ |
| 3 | South along the A577 Neverstitch Road / B5312 Railway Road then west on <br> B5312 Liverpool Road towards M58 Jct 3 | $16 \%$ |
| 4 | North along the A577 Neverstitch Road then north at Half Mile Island | $6 \%$ |
| 5 | North along the A577 Neverstitch Road then east at Half Mile Island | $18 \%$ |
| 6 | North along the A577 Neverstitch Road then south at Half Mile Island | $7 \%$ |
| Total |  | $100 \%$ |

6.12 Adopting the above methodology, which LCC has confirmed does not seem unreasonable, the percentage traffic assignments within the study area are shown graphically in Figure 6.1.

## Resultant Development Traffic Flows

6.13 Based on the traffic generations shown in Table 6.2 and the assignments discussed above, the weekday morning and evening peak traffic flows associated with the proposed development are shown diagrammatically in Figure 6.2

## 7 Off-Site Traffic Impact and Operational Assessments

Off-Site Traffic Impact<br>A577 Neverstitch Rd / A577 Ormskirk Rd / B5312 Railway Rd / Witham Rd / Ormskirk Rd

7.1 As set out earlier in the report, LCC has requested that the impact of the proposed development be considered at the A577 Neverstitch Road / A577 Ormskirk Road / B5312 Railway Road / Witham Road / Ormskirk Road roundabout to the south.
7.2 In light of the above, the traffic impact of the proposals at this junction, compared to the observed traffic flows and not adjusted or future year assessment flows in order to provide further robustness, are shown in Figure 7.1.
7.3 Figure 7.1 shows that the total percentage impact of the proposed development would be $4.9 \%$ during the weekday morning peak and $6.1 \%$ during the weekday evening peak at the A577 Neverstitch Road / A577 Ormskirk Road / B5312 Railway Road / Witham Road / Ormskirk Road roundabout.
7.4 This figure also shows that, during the weekday morning peak, the maximum single arm impact is $12.4 \%$ on the A577 Neverstitch Road, with the A577 Ormskirk Road and B5312 Railway Road arms seeing impacts of between circa $2 \%$ and $3 \%$. During the weekday evening peak, the maximum single arm impact is $9.6 \%$ on the A577 Neverstitch Road, with the A577 Ormskirk Road and B5312 Railway Road arms seeing impacts of between circa $5 \%$ and $10 \%$.
7.5 Based on these increases, it is considered appropriate to undertake operational assessments of this junction.
7.6 In light of the above, the remainder of this section provides details of the traffic flows used to undertake these assessments, together with the resultant model findings.

## Committed Development

7.7 Following pre-application discussions with LCC, they have indicated that there are two committed developments in the study area:

- The Lathom Pastures Phase 1 scheme and accessed from Firswood Road; and
- The Wain Homes scheme located to the north of the Lathom Pastures Phase 1 scheme and also accessed from Firswood Road.
7.8 Traffic flows for these two developments have therefore been taken from the respective Transport Assessments that supported the applications.
7.9 Based on the above, Figure 7.2 shows the Lathom Pastures Phase 1 committed development peak hour flows, whilst Figure 7.3 shows the Wain Homes committed development peak hour flows.


## Future Years and Traffic Growth

Future Years
7.10 The now withdrawn Guidance on Transport Assessment [GTA] document stated that, on local roads, future year assessments should be carried out based on a year 5 years after the year of registration of the planning application. On this basis, assessment of the potential impact on the surrounding highway network should be considered at 2025.
7.11 In order to derive factors for this future year the "RTF 2018 Scenario 1 - Reference" Indices, adjusted to local conditions using the geographical parameters within the TEMPRO program, has been utilised. Based on the geographical parameter of 'West Lancashire 014' and the 'all' roads parameter within TEMPRO 7.2, Table 7.1 below details the growth factors to be applied to the 2020 observed traffic flows set out in Section 3 to derive a set of assessment year background traffic flows.
Table 7.1: TEMPRO Adjusted Growth Factors for West Lancashire

|  | Weekday Morning Peak | Weekday Evening Peak |
| :--- | :--- | :--- |
| $2020-2025$ | 1.0539 | 1.0522 |

## Assessment Year Traffic Flows

7.12 Giving due cognisance to the above, Figure 7.4 shows the 2025 TEMPRO growthed background traffic flows for the weekday morning and evening peaks.
7.13 From these background flows, Figure 7.5 shows the 2025 base flows for the weekday morning and evening peaks respectively with the committed development traffic flows added.
7.14 Following on from the above, 2025 base plus proposed development traffic flows are shown in Figure 7.6 for the weekday morning and evening peaks.

Operational Assessments: Proposed A577 Neverstitch Road Site Access Junction
7.15 Based on the identified 2025 base plus proposed development traffic flows and the proposed access junction detailed in Section 5, Junctions 9 assessments have been carried out for the A577 Neverstitch Road site access. Table 7.2 below shows the results of these operational assessments, whilst the full LINSIG outputs are included at Appendix $\mathbf{F}$ for information.

Table 7.2: Proposed A577 Neverstitch Road Site Access: Junctions 9 Results

|  | Weekday Morning Peak |  | Weekday Evening Peak |  |
| :--- | :--- | :--- | :--- | :--- | :--- |
|  | RFC | Q | RFC | Q |
| Site Access: Left turn | 0.056 | 0 | 0.033 | 0 |
| Site Access: Right turn | 0.203 | 0 | 0.122 | 0 |
| Neverstitch Road: Right turn | 0.017 | 0 | 0.049 | 0 |

RFC: Ratio of Flow to Capacity. Q: Queve length in vehs
7.16 As can be seen from the above table the Junctions 9 modelling indicates that the maximum RFC value at the proposed site access would be 0.203 during the weekday morning peak and 0.122 during the weekday evening peak. These levels of RFC are significantly below the traditional 0.850 design capacity. There would also be no queuing during either peak.
7.17 Based on these results it is suggested that there would be no operational issues associated with the proposed site access and that the proposed access arrangement would be more than sufficient to safely serve the proposed development.

Operational Assessments: A577 Neverstitch Road / A577 Ormskirk Road / B5312 Railway Road / Witham Road / Ormskirk Road Roundabout
7.18 Based on the adjusted 2020, 2025 base and 2025 base plus proposed development traffic flows, Junctions 9 assessments have been carried out for the A577 Neverstitch Road / A577 Ormskirk Road / B5312 Railway Road / Witham Road / Ormskirk Road roundabout. Table 7.3 below shows the results of these operational assessments, whilst the full Junctions 9 outputs are included at Appendix $\mathbf{G}$ for information.

Table 7.3: A577 Neverstitch Road / A577 Ormskirk Road / B5312 Railway Road / Witham Road / Ormskirk Road Roundabout: Junctions 9 Results

|  | Existing |  | 2025 Base |  | 2025 Base Plus Dev |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | RFC | Q | RFC | Q | RFC | Q |
| Weekday Morning Peak |  |  |  |  |  |  |
| A577 Neverstitch Road | 0.372 | 1 | 0.406 | 1 | 0.447 | 1 |
| Witham Road | 0.297 | 0 | 0.327 | 1 | 0.336 | 1 |
| B5312 Railway Road | 0.293 | 0 | 0.317 | 1 | 0.326 | 1 |
| A577 Ormskirk Road | 0.396 | 1 | 0.475 | 1 | 0.484 | 1 |
| Weekday Evening Peak |  |  |  |  |  |  |
| A577 Neverstitch Road | 0.282 | 0 | 0.307 | 0 | 0.329 | 1 |
| Witham Road | 0.351 | 1 | 0.403 | 1 | 0.408 | 1 |
| B5312 Railway Road | 0.248 | 0 | 0.283 | 0 | 0.303 | 0 |
| A577 Ormskirk Road | 0.465 | 1 | 0.534 | 1 | 0.558 | 1 |

RFC: Ratio of Flow to Capacity. $Q$ : Queue length in vehs
7.19 As can be seen from the above table the Junctions 9 modelling indicates that at a five year future year and with the development in place, and despite the onerous traffic flows used in the assessments, the maximum RFC value at the A577 Neverstitch Road / A577 Ormskirk Road / B5312 Railway Road / Witham Road / Ormskirk Road roundabout would be 0.484 during the weekday morning peak and 0.558 during the weekday evening peak. These levels of RFC remain significantly below the traditional 0.850 design capacity. There would also be minimal queuing.
7.20 Based on these results it is suggested that there would be no operational issues at the A577 Neverstitch Road / A577 Ormskirk Road / B5312 Railway Road / Witham Road / Ormskirk Road roundabout and that the existing junction arrangements are sufficient to serve the proposed development.

## Summary

7.21 Based on the modelling presented in this section of the report, it is concluded that there would be no operational issues associated with the proposed site access and that the proposed access arrangement would be more than sufficient to safely serve the proposed development. It is also concluded that there would be no operational issues at the A577 Neverstitch Road / A577 Ormskirk Road / B5312 Railway Road / Witham Road / Ormskirk Road roundabout and that the existing junction arrangements are sufficient to serve the proposed development.

## 8 Summary, Conclusions and Recommendations

## Summary and Conclusions

8.1 Based on the findings of this report it is concluded that:

- The proposed site is allocated for residential development in the West Lancashire Local Plan and, in relation to transport, the scheme complies the Development Brief for the site;


## Sustainability and Linkage

- With the improvements in place that are proposed as part of the development, the proposed site will have good accessibility for pedestrians and cyclists;
- Considering the wider context, the site is well placed in terms of pedestrian connectivity, with walking representing a very realistic alternative to the car for future residents undertaking educational journeys and those making use of the various local shops, facilities and employment areas in the local area. The site also offers the opportunity for future residents to connect to the local bus network on foot;
- Considering cycling in the wider context, the 5 km cycling catchment for the site takes in the whole of the Skelmersdale area, the Lathom and Westhead areas to the west and part of the large employment area to the south of the M58. Whilst outside the 5 km catchment, Ormskirk and Edge Hill University are around a 5 to 6 km ride from the site and could therefore be accessible by bike for some future residents. Ormskirk railway station is also around a 6 km cycle ride from the site and could also be accessible by bike for some future residents of the site. The site is therefore well placed for residents to travel by bicycle for educational, employment, retail and leisure trip;
- With regard to public transport, the site is well placed for future residents to connect to the local bus network on foot. It also offers some opportunity for future residents to connect to the local rail network by bicycle. There are therefore opportunities for future residents to travel by these modes for a variety of journey purposes;


## Site Access, Pedestrian and Cycle Provision

- The proposed access arrangements from the A577 Neverstitch Road are considered acceptable and appropriate from a design, safety and operational perspective. The principle of these access arrangements has also been agreed with LCC through the pre-application process. Furthermore, assessments show that the access junction would be sufficient to serve the proposed development;
- The proposed construction access arrangement from the A577 Neverstitch Road is considered acceptable. LCC has also indicated that they would not be averse to such a construction access;
- The internal highway arrangement is appropriate to suitably serve the proposed development;
- The proposed access from the A577 Neverstitch Road would include 2 metre wide footways to both sides which would continue throughout the site on both sides of the highway network;
- In addition to these internal provisions, there are a number of new pedestrian facilities proposed as part of the development to link the site to the surrounding network. These new facilities include footways and a crossing point on the A577 Neverstitch Road, pedestrian links to these new footways from the development at various points and a pedestrian link from the development to the existing footway on the A577 Ormskirk Road. These provisions will link the development to the surrounding pedestrian network;
- With regard to cycle provision, the internal site layout is considered to be conducive to cycle use;


## Traffic Impact

- Based on the existing accident record and in the context of the increases in traffic flow as a result of the development, it is considered that the proposals would not materially impact on the areas existing highway safety record;
- As agreed with LCC through the pre-application process, operational assessments have been carried out at A577 Neverstitch Road / A577 Ormskirk Road / B5312 Railway Road / Witham Road / Ormskirk Road roundabout. These assessments show that the existing junction arrangements are sufficient at this location to serve the proposed development.


## Recommendations

8.2 In light of the above it is the recommendation of CBO Transport that there are no traffic or transportation grounds on which to refuse the application for the proposed residential development.

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Figure 7.5: 2025 Base Traffic Flows
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Figure 6.1 Proposed Development Traffic Assignment


Figure 6.2 Proposed Development Traffic Flows



Figure 7.2 Committed Development Traffic Flows (Lathom Pastures Phase 1)
$\square$ Weekday Morning Peak Evening Peak


Figure 7.3 Committed Development Traffic Flows (Wain Homes)
$\square$ Weekday Morning Peak Evening Peak





Appendix A: Speed Survey Results

## Skelmersdale ATC 2, Neverswitch Road

Produced by Streetwise Services Ltd.

## S/streetwise

Average Speed
Week 1

|  | 19/01/2020 | 20/01/2020 | 21/01/2020 | 22/01/2020 | 23/01/2020 | 24/01/2020 | 25/01/2020 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Hr Ending | Sunday | Monday | Tuesday | Wednesday | Thursday | Friday | Saturday |
| 1 | 39.5 | 37.8 | 43.8 | 36.1 | 38.9 | 38.2 | 39.5 |
| 2 | 38.8 | 38.6 | 36.3 | 37.6 | 41.9 | 40.1 | 41.1 |
| 3 | 39.1 | 39.7 | 43.0 | 40.0 | 36.8 | 38.6 | 39.3 |
| 4 | 40.8 | 41.1 | 38.0 | 42.6 | 40.2 | 42.5 | 42.2 |
| 5 | 40.5 | 36.8 | 39.0 | 38.4 | 36.9 | 44.1 | 43.3 |
| 6 | 39.2 | 37.8 | 40.0 | 38.5 | 39.4 | 38.7 | 39.0 |
| 7 | 36.6 | 38.1 | 38.7 | 38.0 | 37.8 | 39.8 | 39.6 |
| 8 | 38.6 | 37.5 | 38.0 | 38.4 | 37.2 | 37.0 | 38.7 |
| 9 | 36.0 | 37.7 | 37.7 | 37.7 | 37.5 | 37.2 | 39.0 |
| 10 | 38.3 | 37.5 | 37.5 | 36.4 | 37.7 | 37.4 | 38.8 |
| 11 | 38.1 | 38.0 | 37.5 | 37.4 | 37.9 | 37.6 | 38.5 |
| 12 | 39.1 | 38.4 | 36.9 | 37.5 | 37.2 | 37.0 | 38.8 |
| 13 | 39.0 | 38.2 | - | 37.7 | 37.7 | 37.6 | 39.0 |
| 14 | 39.7 | 38.5 | 37.5 | 37.6 | 38.0 | 38.3 | 39.2 |
| 15 | 39.7 | 37.7 | 37.0 | 37.6 | 36.9 | 37.4 | 38.7 |
| 16 | 39.2 | 38.3 | 37.3 | 37.3 | 37.9 | 37.9 | 38.4 |
| 17 | 38.8 | 36.5 | 37.5 | 36.7 | 38.2 | 37.1 | 38.7 |
| 18 | 39.0 | 38.0 | 38.5 | 37.1 | 37.9 | 37.8 | 38.1 |
| 19 | 39.5 | 38.3 | 38.0 | 38.1 | 38.2 | 39.1 | 38.9 |
| 20 | 39.1 | 39.4 | 38.0 | 38.5 | 38.1 | 38.2 | 38.6 |
| 21 | 38.7 | 38.9 | 39.5 | 37.8 | 39.2 | 39.0 | 39.4 |
| 22 | 39.0 | 40.0 | 38.8 | 37.5 | 39.0 | 39.0 | 39.3 |
| 23 | 40.0 | 39.4 | 39.7 | 36.6 | 38.6 | 38.6 | 39.7 |
| 24 | 39.0 | 40.8 | 38.4 | 38.3 | 39.0 | 39.7 | 39.1 |


| $10-12$ | 38.7 | 38.2 | 37.2 | 37.4 | 37.5 | 37.3 | 38.7 |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |
| $14-16$ | 39.4 | 38.0 | 37.1 | 37.4 | 37.5 | 37.7 | 38.5 |
| $0-24$ | 39.0 | 38.1 | 37.8 | 37.5 | 37.9 | 37.9 | 38.8 |

85th Percentile

|  | 19/01/2020 | 20/01/2020 | 21/01/2020 | 22/01/2020 | 23/01/2020 | 24/01/2020 | 25/01/2020 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Hr Ending | Sunday | Monday | Tuesday | Wednesday | Thursday | Friday | Saturday |
| 1 | 43.7 | 43.6 | 56.5 | 38.6 | 43.2 | 43.1 | 43.3 |
| 2 | 43.5 | 43.3 | 43.4 | 56.2 | 48.9 | 43.7 | 43.2 |
| 3 | 43.6 | 48.3 | 43.3 | 48.9 | 38.7 | 43.9 | 43.5 |
| 4 | 48.3 | 56.3 | 48.2 | 66.3 | 43.5 | 56.0 | 48.3 |
| 5 | 48.3 | 43.8 | 43.2 | 43.0 | 43.4 | 55.6 | 48.3 |
| 6 | 43.8 | 43.6 | 43.6 | 43.5 | 43.1 | 48.8 | 43.0 |
| 7 | 43.0 | 44.0 | 43.4 | 43.9 | 43.8 | 43.4 | 43.5 |
| 8 | 43.8 | 43.9 | 43.4 | 43.4 | 43.5 | 43.5 | 43.2 |
| 9 | 38.8 | 43.2 | 43.7 | 43.7 | 43.8 | 43.5 | 43.9 |
| 10 | 43.7 | 43.7 | 43.3 | 43.5 | 43.6 | 43.2 | 43.6 |
| 11 | 43.0 | 44.0 | 43.6 | 43.5 | 43.8 | 43.3 | 43.8 |
| 12 | 43.4 | 43.2 | 43.2 | 43.5 | 43.0 | 43.1 | 43.9 |
| 13 | 43.9 | 43.5 | - | 43.4 | 43.2 | 43.6 | 43.3 |
| 14 | 43.8 | 43.1 | 43.2 | 43.4 | 43.1 | 43.2 | 43.5 |
| 15 | 43.4 | 44.0 | 43.6 | 43.3 | 43.1 | 43.9 | 43.1 |
| 16 | 44.0 | 43.7 | 43.1 | 43.1 | 43.3 | 43.1 | 43.6 |
| 17 | 43.9 | 43.0 | 43.5 | 43.2 | 43.1 | 43.4 | 43.4 |
| 18 | 43.1 | 43.6 | 43.9 | 44.0 | 43.0 | 43.3 | 44.0 |
| 19 | 43.9 | 43.1 | 43.3 | 43.1 | 43.5 | 43.9 | 43.1 |
| 20 | 43.4 | 43.1 | 43.8 | 43.4 | 43.7 | 43.8 | 43.9 |
| 21 | 43.5 | 43.8 | 43.4 | 43.4 | 43.5 | 43.3 | 43.6 |
| 22 | 43.8 | 43.3 | 43.3 | 43.5 | 43.8 | 43.7 | 43.3 |
| 23 | 43.1 | 43.0 | 43.9 | 43.2 | 43.1 | 43.3 | 43.1 |
| 24 | 48.6 | 48.3 | 43.6 | 43.5 | 43.2 | 48.1 | 43.5 |


| $10-12$ | 43.5 | 43.4 | 43.6 | 43.3 | 43.7 | 43.0 | 43.2 |
| :---: | :--- | :--- | :--- | :--- | :--- | :--- | :--- |
| $14-16$ | 43.3 | 43.3 | 43.4 | 43.6 | 43.5 | 43.3 | 44.0 |
| $0-24$ | 43.6 | 43.9 | 43.1 | 43.5 | 43.4 | 43.8 | 43.1 |

## Skelmersdale ATC 1, Neverswitch Road

Produced by Streetwise Services Ltd.

## SStreetwise

Average Speed
Week 1

| Hr Ending | $\begin{gathered} \hline \text { 19/01/2020 } \\ \text { Sunday } \end{gathered}$ | 20/01/2020 | 21/01/2020 <br> Tuesday | 22/01/2020 <br> Wednesday | 23/01/2020 <br> Thursday | $\begin{gathered} \text { 24/01/2020 } \\ \text { Friday } \end{gathered}$ | 25/01/2020 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | 34.6 | 38.7 | 36.8 | 39.4 | 40.1 | 39.7 | 40.0 |
| 2 | 33.6 | 39.9 | 31.8 | 38.7 | 39.7 | 36.3 | 39.5 |
| 3 | - | 46.1 | 34.4 | 44.1 | 38.0 | 38.0 | 42.4 |
| 4 | - | 44.5 | 38.0 | 40.1 | 36.3 | 39.8 | 40.0 |
| 5 | - | 38.7 | 37.3 | 37.2 | 38.2 | 40.4 | 36.1 |
| 6 | - | 38.8 | 36.2 | 39.6 | 39.7 | 38.6 | 38.4 |
| 7 | - | 39.0 | 36.5 | 38.4 | 39.0 | 38.9 | 37.6 |
| 8 | - | 34.9 | 33.9 | 37.0 | 36.5 | 36.3 | 37.3 |
| 9 | - | 36.0 | 34.5 | 36.2 | 36.6 | 36.6 | 38.0 |
| 10 | - | 35.3 | 33.6 | 36.8 | 36.7 | 36.9 | 36.9 |
| 11 | 36.1 | 35.0 | 33.4 | 36.9 | 36.4 | 36.4 | 37.4 |
| 12 | 35.3 | 34.5 | 33.4 | 36.2 | 36.3 | 36.2 | 37.6 |
| 13 | 35.4 | 34.0 | - | 37.8 | 37.0 | 36.8 | 37.5 |
| 14 | 34.6 | 34.9 | 35.6 | 36.4 | 37.0 | 36.4 | 37.8 |
| 15 | 36.0 | 34.2 | 36.5 | 37.1 | 37.6 | 36.8 | 38.1 |
| 16 | 36.0 | 33.0 | 36.6 | 36.6 | 37.2 | 36.1 | 38.1 |
| 17 | 35.8 | 34.7 | 36.1 | 36.9 | 37.4 | 36.3 | 37.3 |
| 18 | 37.2 | 34.2 | 36.8 | 36.5 | 36.8 | 36.1 | 37.2 |
| 19 | 36.0 | 34.1 | 36.7 | 37.7 | 37.4 | 38.0 | 39.6 |
| 20 | 35.1 | 36.0 | 38.3 | 37.8 | 37.9 | 37.1 | 37.1 |
| 21 | 35.3 | 36.1 | 38.5 | 37.6 | 39.2 | 38.9 | 39.3 |
| 22 | 35.9 | 36.7 | 38.3 | 37.7 | 39.8 | 39.1 | 39.0 |
| 23 | 37.2 | 36.9 | 40.0 | 37.5 | 37.5 | 38.4 | 40.4 |
| 24 | 35.4 | 35.4 | 40.7 | 36.4 | 37.5 | 40.1 | 39.1 |


| $10-12$ | 35.6 | 34.8 | 33.4 | 36.5 | 36.4 | 36.3 | 37.5 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $14-16$ | 36.0 | 33.5 | 36.5 | 36.8 | 37.4 | 36.4 | 38.1 |
| $0-24$ | 35.6 | 35.0 | 35.7 | 37.0 | 37.2 | 36.9 | 37.9 |

85th Percentile

|  | 19/01/2020 | 20/01/2020 | 21/01/2020 | 22/01/2020 | 23/01/2020 | 24/01/2020 | 25/01/2020 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Hr Ending | Sunday | Monday | Tuesday | Wednesday | Thursday | Friday | Saturday |
| 1 | 38.1 | 44.0 | 43.2 | 43.9 | 43.5 | 43.3 | 48.2 |
| 2 | 38.9 | 43.3 | 38.7 | 38.7 | 49.0 | 44.0 | 43.4 |
| 3 | - | 56.0 | 48.7 | 55.6 | 38.2 | 43.8 | 48.2 |
| 4 | - | 55.9 | 43.6 | 56.1 | 38.4 | 43.7 | 48.9 |
| 5 | - | 43.7 | 43.8 | 43.7 | 48.4 | 48.9 | 38.6 |
| 6 | - | 43.2 | 38.2 | 43.0 | 43.3 | 43.9 | 43.4 |
| 7 | - | 43.4 | 43.8 | 43.4 | 43.5 | 43.4 | 43.4 |
| 8 | - | 38.5 | 38.2 | 43.4 | 43.1 | 43.1 | 43.9 |
| 9 | - | 38.8 | 39.0 | 43.3 | 38.5 | 44.0 | 43.6 |
| 10 | - | 38.5 | 38.1 | 44.0 | 44.0 | 43.8 | 43.9 |
| 11 | 38.6 | 38.4 | 38.1 | 43.8 | 43.6 | 38.7 | 43.5 |
| 12 | 38.3 | 38.5 | 38.8 | 43.7 | 43.9 | 43.4 | 43.3 |
| 13 | 38.1 | 38.2 | - | 43.4 | 43.7 | 43.0 | 43.9 |
| 14 | 38.5 | 38.6 | 38.4 | 43.7 | 43.4 | 43.2 | 43.3 |
| 15 | 38.2 | 38.5 | 43.5 | 43.3 | 43.7 | 43.2 | 43.3 |
| 16 | 44.0 | 38.7 | 43.1 | 43.4 | 43.1 | 43.5 | 43.2 |
| 17 | 38.1 | 38.9 | 43.1 | 43.4 | 43.8 | 43.4 | 43.3 |
| 18 | 38.0 | 38.4 | 43.2 | 43.9 | 43.7 | 38.1 | 43.0 |
| 19 | 43.3 | 38.3 | 43.0 | 43.1 | 43.5 | 43.3 | 43.7 |
| 20 | 38.5 | 38.3 | 43.7 | 43.6 | 43.2 | 43.6 | 43.8 |
| 21 | 38.9 | 43.2 | 43.5 | 43.3 | 43.2 | 48.8 | 43.3 |
| 22 | 38.5 | 43.5 | 43.6 | 43.1 | 43.3 | 48.5 | 43.7 |
| 23 | 43.4 | 38.2 | 43.2 | 43.2 | 43.8 | 43.2 | 48.4 |
| 24 | 38.8 | 43.6 | 48.5 | 43.1 | 43.1 | 43.9 | 43.8 |


| $10-12$ | 38.8 | 38.4 | 38.7 | 43.4 | 43.5 | 43.4 | 43.7 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $14-16$ | 43.7 | 38.9 | 43.8 | 44.0 | 43.8 | 43.3 | 43.4 |
| $0-24$ | 38.7 | 38.5 | 43.4 | 43.5 | 43.8 | 43.8 | 43.1 |

Appendix B：Proposed Site Layou $\dagger$
ロワロ
TRANSPORT


Appendix C: CBO Drawings



## TRIP RATE CALCULATI ON SELECTI ON PARAMETERS:

Land Use : 03-RESIDENTIAL
Category : A - HOUSES PRIVATELY OWNED
MULTI-MODAL VEHICLES
Selected regions and areas:

| 02 | SOUTH EAST |  |
| :---: | :---: | :---: |
|  | ES EAST SUSSEX | 1 days |
|  | HF HERTFORDSHIRE | 1 days |
|  | KC KENT | 3 days |
|  | SC SURREY | 1 days |
|  | WS WEST SUSSEX | 3 days |
| 04 | EAST ANGLIA |  |
|  | NF NORFOLK | 1 days |
| 06 | WEST MIDLANDS |  |
|  | SH SHROPSHIRE | 1 days |
|  | ST STAFFORDSHIRE | 1 days |

This section displays the number of survey days per TRICS® sub-region in the selected set

## Secondary Filtering selection:

This data displays the chosen trip rate parameter and its selected range. Only sites that fall within the parameter range are included in the trip rate calculation.

| Parameter: | Number of dwellings |
| :--- | :--- |
| Actual Range: | 108 to 288 (units:) |
| Range Selected by User: | 100 to 300 (units:) |
| Parking Spaces Range: | All Surveys Included |

Bedrooms per Dwelling Range: All Surveys Included
Percentage of dwellings privately owned: All Surveys Included
Public Transport Provision:
Selection by: Include all surveys
Date Range: $\quad 01 / 01 / 09$ to $23 / 09 / 19$
This data displays the range of survey dates selected. Only surveys that were conducted within this date range are included in the trip rate calculation.

Selected survey days:

| Monday | 4 days |
| :--- | :--- |
| Tuesday | 1 days |
| Wednesday | 2 days |
| Thursday | 4 days |
| Friday | 1 days |

This data displays the number of selected surveys by day of the week.
Selected survey types:
Manual count $\quad 12$ days
This data displays the number of manual classified surveys and the number of unclassified ATC surveys, the total adding up to the overall number of surveys in the selected set. Manual surveys are undertaken using staff, whilst ATC surveys are undertaking using machines.

Selected Locations:
Suburban Area (PPS6 Out of Centre) 1
Edge of Town 10
Neighbourhood Centre (PPS6 Local Centre) 1
This data displays the number of surveys per main location category within the selected set. The main location categories consist of Free Standing, Edge of Town, Suburban Area, Neighbourhood Centre, Edge of Town Centre, Town Centre and Not Known.

Selected Location Sub Categories:
Residential Zone 10
Village
No Sub Category

## Secondary Filtering selection:

Use Class:
C3
12 days

This data displays the number of surveys per Use Class classification within the selected set. The Use Classes Order 2005 has been used for this purpose, which can be found within the Library module of TRICS ${ }^{\circledR}$.

Population within 1 mile:

| 1,001 to 5,000 | 1 days |
| :--- | :--- |
| 5,001 to 10,000 | 4 days |
| 10,001 to 15,000 | 4 days |
| 15,001 to 20,000 | 1 days |
| 20,001 to 25,000 | 2 days |

This data displays the number of selected surveys within stated 1-mile radii of population.
Population within 5 miles:

| 5,001 to 25,000 | 1 days |
| :--- | :--- |
| 25,001 to 50,000 | 1 days |
| 50,001 to 75,000 | 1 days |
| 75,001 to 100,000 | 4 days |
| 125,001 to 250,000 | 5 days |

This data displays the number of selected surveys within stated 5 -mile radii of population.
Car ownership within 5 miles:

| 0.6 to 1.0 | 2 days |
| :--- | :--- |
| 1.1 to 1.5 | 8 days |
| 1.6 to 2.0 | 2 days |

This data displays the number of selected surveys within stated ranges of average cars owned per residential dwelling, within a radius of 5 -miles of selected survey sites.

| Travel Plan: | 6 days |
| :--- | :--- |
| Yes | 6 days |

This data displays the number of surveys within the selected set that were undertaken at sites with Travel Plans in place, and the number of surveys that were undertaken at sites without Travel Plans.

## PTAL Rating:

```
No PTAL Present
12 days
```

This data displays the number of selected surveys with PTAL Ratings.

1 ES-03-A-03
MI XED HOUSES \& FLATS
SHEPHAM LANE
POLEGATE
Edge of Town
Residential Zone
Total Number of dwellings:
212
Survey date: MONDAY 11/07/16
2 HF-03-A-03
MI XED HOUSES
HARE STREET ROAD
BUNTINGFORD
Edge of Town
Residential Zone
Total Number of dwellings: 160
Survey date: MONDAY 08/07/19
3 KC-03-A-04
SEMI -DETACHED \& TERRACED
KILN BARN ROAD
AYLESFORD
DITTON
Edge of Town
Residential Zone
Total Number of dwellings:
110
Survey date: FRIDAY 22/09/17
4 KC-03-A-07
MI XED HOUSES
RECULVER ROAD
HERNE BAY
Edge of Town
Residential Zone
Total Number of dwellings:
Survey date: WEDNESDAY 27/09/17
5 KC-03-A-08
MI XED HOUSES
MAIDSTONE ROAD
CHARING
Neighbourhood Centre (PPS6 Local Centre)
Village
Total Number of dwellings:
159
22/05/18
6 NF-03-A-06 MIXED HOUSES
BEAUFORT WAY
GREAT YARMOUTH
BRADWELL
Edge of Town
Residential Zone
Total Number of dwellings:
Survey date: MONDAY
275
23/09/19
7 SC-03-A-05 MI XED HOUSES
REIGATE ROAD
HORLEY
Edge of Town
Residential Zone
Total Number of dwellings:
207
Survey date: MONDAY
01/04/19
8 SH-03-A-04 TERRACED
ST MICHAEL'S STREET
SHREWSBURY
Suburban Area (PPS6 Out of Centre)
No Sub Category
Total Number of dwellings: $\begin{array}{ll}\text { Survey date: THURSDAY } & 11 / 06 / 09\end{array}$

## EAST SUSSEX

Survey Type: MANUAL HERTFORDSHI RE

Survey Type: MANUAL

## KENT

Survey Type: MANUAL KENT

Survey Type: MANUAL KENT

Survey Type: MANUAL

## NORFOLK

Survey Type: MANUAL SURREY

Survey Type: MANUAL SHROPSHIRE

Survey Type: MANUAL

LIST OF SITES relevant to selection parameters (Cont.)

9 ST-03-A-07
DETACHED \& SEMI-DETACHED
BEACONSIDE
STAFFORD
MARSTON GATE
Edge of Town
Residential Zone
Total Number of dwellings: Survey date: WEDNESDAY 22/11/17
10
WS-03-A-04
MI XED HOUSES
HILLS FARM LANE
HORSHAM
BROADBRIDGE HEATH
Edge of Town
Residential Zone
Total Number of dwellings: 151
Survey date: THURSDAY 11/12/14
11 WS-03-A-08 MI XED HOUSES
ROUNDSTONE LANE
ANGMERING
Edge of Town
Residential Zone
Total Number of dwellings:
180
Survey date: THURSDAY 19/04/18
12 WS-03-A-09 MI XED HOUSES \& FLATS
LITTLEHAMPTON ROAD
WORTHING
WEST DURRINGTON
Edge of Town
Residential Zone
Total Number of dwellings
Survey date: THURSDAY 05/07/18 Survey Type: MANUAL

This section provides a list of all survey sites and days in the selected set. For each individual survey site, it displays a unique site reference code and site address, the selected trip rate calculation parameter and its value, the day of the week and date of each survey, and whether the survey was a manual classified count or an ATC count.

## MANUALLY DESELECTED SITES

| Site Ref |  |
| :--- | :--- |
| DH-03-A-02 | Low rates |
| DV-03-A-02 | Bungalows |
| ES-03-A-04 | Low rates |
| FA-03-A-02 | Low ave beds |
| NY-03-A-06 | Bungalows |

TRIP RATE for Land Use 03 - RESIDENTIAL/A - HOUSES PRIVATELY OWNED
MULTI-MODAL VEHICLES
Calculation factor: 1 DWELLS
BOLD print indicates peak (busiest) period

|  | ARRIVALS |  |  | DEPARTURES |  |  | TOTALS |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Time Range | No. Days | Ave. DWELLS | Trip Rate | No. Days | Ave. DWELLS | Trip Rate | No. Days | Ave. DWELLS | Trip Rate |
| 00:00-01:00 |  |  |  |  |  |  |  |  |  |
| 01:00-02:00 |  |  |  |  |  |  |  |  |  |
| 02:00-03:00 |  |  |  |  |  |  |  |  |  |
| 03:00-04:00 |  |  |  |  |  |  |  |  |  |
| 04:00-05:00 |  |  |  |  |  |  |  |  |  |
| 05:00-06:00 |  |  |  |  |  |  |  |  |  |
| 06:00-07:00 |  |  |  |  |  |  |  |  |  |
| 07:00-08:00 | 12 | 191 | 0.088 | 12 | 191 | 0.302 | 12 | 191 | 0.390 |
| 08:00-09:00 | 12 | 191 | 0.152 | 12 | 191 | 0.387 | 12 | 191 | 0.539 |
| 09:00-10:00 | 12 | 191 | 0.149 | 12 | 191 | 0.182 | 12 | 191 | 0.331 |
| 10:00-11:00 | 12 | 191 | 0.125 | 12 | 191 | 0.151 | 12 | 191 | 0.276 |
| 11:00-12:00 | 12 | 191 | 0.131 | 12 | 191 | 0.153 | 12 | 191 | 0.284 |
| 12:00-13:00 | 12 | 191 | 0.156 | 12 | 191 | 0.148 | 12 | 191 | 0.304 |
| 13:00-14:00 | 12 | 191 | 0.165 | 12 | 191 | 0.148 | 12 | 191 | 0.313 |
| 14:00-15:00 | 12 | 191 | 0.174 | 12 | 191 | 0.201 | 12 | 191 | 0.375 |
| 15:00-16:00 | 12 | 191 | 0.271 | 12 | 191 | 0.174 | 12 | 191 | 0.445 |
| 16:00-17:00 | 12 | 191 | 0.273 | 12 | 191 | 0.167 | 12 | 191 | 0.440 |
| 17:00-18:00 | 12 | 191 | 0.352 | 12 | 191 | 0.154 | 12 | 191 | 0.506 |
| 18:00-19:00 | 12 | 191 | 0.301 | 12 | 191 | 0.179 | 12 | 191 | 0.480 |
| 19:00-20:00 |  |  |  |  |  |  |  |  |  |
| 20:00-21:00 |  |  |  |  |  |  |  |  |  |
| 21:00-22:00 |  |  |  |  |  |  |  |  |  |
| 22:00-23:00 |  |  |  |  |  |  |  |  |  |
| 23:00-24:00 |  |  |  |  |  |  |  |  |  |
| Total Rates: |  |  | 2.337 |  |  | 2.346 |  |  | 4.683 |

This section displays the trip rate results based on the selected set of surveys and the selected count type (shown just above the table). It is split by three main columns, representing arrivals trips, departures trips, and total trips (arrivals plus departures). Within each of these main columns are three sub-columns. These display the number of survey days where count data is included (per time period), the average value of the selected trip rate calculation parameter (per time period), and the trip rate result (per time period). Total trip rates (the sum of the column) are also displayed at the foot of the table.

To obtain a trip rate, the average (mean) trip rate parameter value (TRP) is first calculated for all selected survey days that have count data available for the stated time period. The average (mean) number of arrivals, departures or totals (whichever applies) is also calculated (COUNT) for all selected survey days that have count data available for the stated time period. Then, the average count is divided by the average trip rate parameter value, and multiplied by the stated calculation factor (shown just above the table and abbreviated here as FACT). So, the method is: COUNT/TRP*FACT. Trip rates are then rounded to 3 decimal places.

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## Parameter summary

Trip rate parameter range selected: Survey date date range:
Number of weekdays (Monday-Friday):
Number of Saturdays:
Number of Sundays:
Surveys automatically removed from selection:
Surveys manually removed from selection:

108-288 (units:)
01/01/09-23/09/19
12
0
0
0
5

This section displays a quick summary of some of the data filtering selections made by the TRICS ${ }^{\circledR}$ user. The trip rate calculation parameter range of all selected surveys is displayed first, followed by the range of minimum and maximum survey dates selected by the user. Then, the total number of selected weekdays and weekend days in the selected set of surveys are show. Finally, the number of survey days that have been manually removed from the selected set outside of the standard filtering procedure are displayed.

TRIP RATE for Land Use 03 - RESIDENTIAL/A - HOUSES PRIVATELY OWNED
MULTI-MODAL TAXIS

## Calculation factor: 1 DWELLS

BOLD print indicates peak (busiest) period

|  | ARRIVALS |  |  | DEPARTURES |  |  | TOTALS |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Time Range | No. Days | Ave. DWELLS | Trip Rate | No. Days | Ave. DWELLS | Trip Rate | No. Days | Ave. DWELLS | Trip Rate |
| 00:00-01:00 |  |  |  |  |  |  |  |  |  |
| 01:00-02:00 |  |  |  |  |  |  |  |  |  |
| 02:00-03:00 |  |  |  |  |  |  |  |  |  |
| 03:00-04:00 |  |  |  |  |  |  |  |  |  |
| 04:00-05:00 |  |  |  |  |  |  |  |  |  |
| 05:00-06:00 |  |  |  |  |  |  |  |  |  |
| 06:00-07:00 |  |  |  |  |  |  |  |  |  |
| 07:00-08:00 | 12 | 191 | 0.003 | 12 | 191 | 0.003 | 12 | 191 | 0.006 |
| 08:00-09:00 | 12 | 191 | 0.004 | 12 | 191 | 0.003 | 12 | 191 | 0.007 |
| 09:00-10:00 | 12 | 191 | 0.002 | 12 | 191 | 0.002 | 12 | 191 | 0.004 |
| 10:00-11:00 | 12 | 191 | 0.001 | 12 | 191 | 0.001 | 12 | 191 | 0.002 |
| 11:00-12:00 | 12 | 191 | 0.001 | 12 | 191 | 0.001 | 12 | 191 | 0.002 |
| 12:00-13:00 | 12 | 191 | 0.001 | 12 | 191 | 0.001 | 12 | 191 | 0.002 |
| 13:00-14:00 | 12 | 191 | 0.002 | 12 | 191 | 0.002 | 12 | 191 | 0.004 |
| 14:00-15:00 | 12 | 191 | 0.003 | 12 | 191 | 0.002 | 12 | 191 | 0.005 |
| 15:00-16:00 | 12 | 191 | 0.006 | 12 | 191 | 0.007 | 12 | 191 | 0.013 |
| 16:00-17:00 | 12 | 191 | 0.004 | 12 | 191 | 0.005 | 12 | 191 | 0.009 |
| 17:00-18:00 | 12 | 191 | 0.003 | 12 | 191 | 0.002 | 12 | 191 | 0.005 |
| 18:00-19:00 | 12 | 191 | 0.002 | 12 | 191 | 0.002 | 12 | 191 | 0.004 |
| 19:00-20:00 |  |  |  |  |  |  |  |  |  |
| 20:00-21:00 |  |  |  |  |  |  |  |  |  |
| 21:00-22:00 |  |  |  |  |  |  |  |  |  |
| 22:00-23:00 |  |  |  |  |  |  |  |  |  |
| 23:00-24:00 |  |  |  |  |  |  |  |  |  |
| Total Rates: |  |  | 0.032 |  |  | 0.031 |  |  | 0.063 |

This section displays the trip rate results based on the selected set of surveys and the selected count type (shown just above the table). It is split by three main columns, representing arrivals trips, departures trips, and total trips (arrivals plus departures). Within each of these main columns are three sub-columns. These display the number of survey days where count data is included (per time period), the average value of the selected trip rate calculation parameter (per time period), and the trip rate result (per time period). Total trip rates (the sum of the column) are also displayed at the foot of the table.

To obtain a trip rate, the average (mean) trip rate parameter value (TRP) is first calculated for all selected survey days that have count data available for the stated time period. The average (mean) number of arrivals, departures or totals (whichever applies) is also calculated (COUNT) for all selected survey days that have count data available for the stated time period. Then, the average count is divided by the average trip rate parameter value, and multiplied by the stated calculation factor (shown just above the table and abbreviated here as FACT). So, the method is: COUNT/TRP*FACT. Trip rates are then rounded to 3 decimal places.

TRIP RATE for Land Use 03 - RESIDENTIAL/A - HOUSES PRIVATELY OWNED
MULTI-MODAL OGVS

## Calculation factor: 1 DWELLS

BOLD print indicates peak (busiest) period

|  | ARRIVALS |  |  | DEPARTURES |  |  | TOTALS |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Time Range | No. Days | Ave. DWELLS | Trip Rate | No. Days | Ave. DWELLS | Trip Rate | No. Days | Ave. DWELLS | Trip Rate |
| 00:00-01:00 |  |  |  |  |  |  |  |  |  |
| 01:00-02:00 |  |  |  |  |  |  |  |  |  |
| 02:00-03:00 |  |  |  |  |  |  |  |  |  |
| 03:00-04:00 |  |  |  |  |  |  |  |  |  |
| 04:00-05:00 |  |  |  |  |  |  |  |  |  |
| 05:00-06:00 |  |  |  |  |  |  |  |  |  |
| 06:00-07:00 |  |  |  |  |  |  |  |  |  |
| 07:00-08:00 | 12 | 191 | 0.002 | 12 | 191 | 0.001 | 12 | 191 | 0.003 |
| 08:00-09:00 | 12 | 191 | 0.002 | 12 | 191 | 0.002 | 12 | 191 | 0.004 |
| 09:00-10:00 | 12 | 191 | 0.004 | 12 | 191 | 0.002 | 12 | 191 | 0.006 |
| 10:00-11:00 | 12 | 191 | 0.003 | 12 | 191 | 0.003 | 12 | 191 | 0.006 |
| 11:00-12:00 | 12 | 191 | 0.001 | 12 | 191 | 0.001 | 12 | 191 | 0.002 |
| 12:00-13:00 | 12 | 191 | 0.005 | 12 | 191 | 0.004 | 12 | 191 | 0.009 |
| 13:00-14:00 | 12 | 191 | 0.003 | 12 | 191 | 0.002 | 12 | 191 | 0.005 |
| 14:00-15:00 | 12 | 191 | 0.002 | 12 | 191 | 0.003 | 12 | 191 | 0.005 |
| 15:00-16:00 | 12 | 191 | 0.002 | 12 | 191 | 0.003 | 12 | 191 | 0.005 |
| 16:00-17:00 | 12 | 191 | 0.002 | 12 | 191 | 0.002 | 12 | 191 | 0.004 |
| 17:00-18:00 | 12 | 191 | 0.002 | 12 | 191 | 0.000 | 12 | 191 | 0.002 |
| 18:00-19:00 | 12 | 191 | 0.001 | 12 | 191 | 0.002 | 12 | 191 | 0.003 |
| 19:00-20:00 |  |  |  |  |  |  |  |  |  |
| 20:00-21:00 |  |  |  |  |  |  |  |  |  |
| 21:00-22:00 |  |  |  |  |  |  |  |  |  |
| 22:00-23:00 |  |  |  |  |  |  |  |  |  |
| 23:00-24:00 |  |  |  |  |  |  |  |  |  |
| Total Rates: |  |  | 0.029 |  |  | 0.025 |  |  | 0.054 |

This section displays the trip rate results based on the selected set of surveys and the selected count type (shown just above the table). It is split by three main columns, representing arrivals trips, departures trips, and total trips (arrivals plus departures). Within each of these main columns are three sub-columns. These display the number of survey days where count data is included (per time period), the average value of the selected trip rate calculation parameter (per time period), and the trip rate result (per time period). Total trip rates (the sum of the column) are also displayed at the foot of the table.

To obtain a trip rate, the average (mean) trip rate parameter value (TRP) is first calculated for all selected survey days that have count data available for the stated time period. The average (mean) number of arrivals, departures or totals (whichever applies) is also calculated (COUNT) for all selected survey days that have count data available for the stated time period. Then, the average count is divided by the average trip rate parameter value, and multiplied by the stated calculation factor (shown just above the table and abbreviated here as FACT). So, the method is: COUNT/TRP*FACT. Trip rates are then rounded to 3 decimal places.

TRIP RATE for Land Use 03 - RESIDENTIAL/A - HOUSES PRIVATELY OWNED
MULTI-MODAL PSVS

## Calculation factor: 1 DWELLS

BOLD print indicates peak (busiest) period

|  | ARRIVALS |  |  | DEPARTURES |  |  | TOTALS |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Time Range | No. Days | Ave. DWELLS | Trip Rate | No. Days | Ave. DWELLS | Trip Rate | No. Days | Ave. DWELLS | Trip Rate |
| 00:00-01:00 |  |  |  |  |  |  |  |  |  |
| 01:00-02:00 |  |  |  |  |  |  |  |  |  |
| 02:00-03:00 |  |  |  |  |  |  |  |  |  |
| 03:00-04:00 |  |  |  |  |  |  |  |  |  |
| 04:00-05:00 |  |  |  |  |  |  |  |  |  |
| 05:00-06:00 |  |  |  |  |  |  |  |  |  |
| 06:00-07:00 |  |  |  |  |  |  |  |  |  |
| 07:00-08:00 | 12 | 191 | 0.001 | 12 | 191 | 0.001 | 12 | 191 | 0.002 |
| 08:00-09:00 | 12 | 191 | 0.000 | 12 | 191 | 0.000 | 12 | 191 | 0.000 |
| 09:00-10:00 | 12 | 191 | 0.001 | 12 | 191 | 0.001 | 12 | 191 | 0.002 |
| 10:00-11:00 | 12 | 191 | 0.001 | 12 | 191 | 0.001 | 12 | 191 | 0.002 |
| 11:00-12:00 | 12 | 191 | 0.001 | 12 | 191 | 0.001 | 12 | 191 | 0.002 |
| 12:00-13:00 | 12 | 191 | 0.001 | 12 | 191 | 0.001 | 12 | 191 | 0.002 |
| 13:00-14:00 | 12 | 191 | 0.001 | 12 | 191 | 0.001 | 12 | 191 | 0.002 |
| 14:00-15:00 | 12 | 191 | 0.001 | 12 | 191 | 0.001 | 12 | 191 | 0.002 |
| 15:00-16:00 | 12 | 191 | 0.001 | 12 | 191 | 0.001 | 12 | 191 | 0.002 |
| 16:00-17:00 | 12 | 191 | 0.001 | 12 | 191 | 0.001 | 12 | 191 | 0.002 |
| 17:00-18:00 | 12 | 191 | 0.001 | 12 | 191 | 0.001 | 12 | 191 | 0.002 |
| 18:00-19:00 | 12 | 191 | 0.000 | 12 | 191 | 0.000 | 12 | 191 | 0.000 |
| 19:00-20:00 |  |  |  |  |  |  |  |  |  |
| 20:00-21:00 |  |  |  |  |  |  |  |  |  |
| 21:00-22:00 |  |  |  |  |  |  |  |  |  |
| 22:00-23:00 |  |  |  |  |  |  |  |  |  |
| 23:00-24:00 |  |  |  |  |  |  |  |  |  |
| Total Rates: |  |  | 0.010 |  |  | 0.010 |  |  | 0.020 |

This section displays the trip rate results based on the selected set of surveys and the selected count type (shown just above the table). It is split by three main columns, representing arrivals trips, departures trips, and total trips (arrivals plus departures). Within each of these main columns are three sub-columns. These display the number of survey days where count data is included (per time period), the average value of the selected trip rate calculation parameter (per time period), and the trip rate result (per time period). Total trip rates (the sum of the column) are also displayed at the foot of the table.

To obtain a trip rate, the average (mean) trip rate parameter value (TRP) is first calculated for all selected survey days that have count data available for the stated time period. The average (mean) number of arrivals, departures or totals (whichever applies) is also calculated (COUNT) for all selected survey days that have count data available for the stated time period. Then, the average count is divided by the average trip rate parameter value, and multiplied by the stated calculation factor (shown just above the table and abbreviated here as FACT). So, the method is: COUNT/TRP*FACT. Trip rates are then rounded to 3 decimal places.

TRIP RATE for Land Use 03 - RESIDENTIAL/A - HOUSES PRIVATELY OWNED
MULTI-MODAL CYCLISTS

## Calculation factor: 1 DWELLS

BOLD print indicates peak (busiest) period

|  | ARRIVALS |  |  | DEPARTURES |  |  | TOTALS |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Time Range | No. Days | Ave. DWELLS | Trip Rate | No. Days | Ave. DWELLS | Trip Rate | No. Days | Ave. DWELLS | Trip Rate |
| 00:00-01:00 |  |  |  |  |  |  |  |  |  |
| 01:00-02:00 |  |  |  |  |  |  |  |  |  |
| 02:00-03:00 |  |  |  |  |  |  |  |  |  |
| 03:00-04:00 |  |  |  |  |  |  |  |  |  |
| 04:00-05:00 |  |  |  |  |  |  |  |  |  |
| 05:00-06:00 |  |  |  |  |  |  |  |  |  |
| 06:00-07:00 |  |  |  |  |  |  |  |  |  |
| 07:00-08:00 | 12 | 191 | 0.006 | 12 | 191 | 0.008 | 12 | 191 | 0.014 |
| 08:00-09:00 | 12 | 191 | 0.011 | 12 | 191 | 0.018 | 12 | 191 | 0.029 |
| 09:00-10:00 | 12 | 191 | 0.000 | 12 | 191 | 0.003 | 12 | 191 | 0.003 |
| 10:00-11:00 | 12 | 191 | 0.003 | 12 | 191 | 0.004 | 12 | 191 | 0.007 |
| 11:00-12:00 | 12 | 191 | 0.003 | 12 | 191 | 0.005 | 12 | 191 | 0.008 |
| 12:00-13:00 | 12 | 191 | 0.005 | 12 | 191 | 0.005 | 12 | 191 | 0.010 |
| 13:00-14:00 | 12 | 191 | 0.001 | 12 | 191 | 0.001 | 12 | 191 | 0.002 |
| 14:00-15:00 | 12 | 191 | 0.004 | 12 | 191 | 0.004 | 12 | 191 | 0.008 |
| 15:00-16:00 | 12 | 191 | 0.007 | 12 | 191 | 0.007 | 12 | 191 | 0.014 |
| 16:00-17:00 | 12 | 191 | 0.014 | 12 | 191 | 0.012 | 12 | 191 | 0.026 |
| 17:00-18:00 | 12 | 191 | 0.016 | 12 | 191 | 0.008 | 12 | 191 | 0.024 |
| 18:00-19:00 | 12 | 191 | 0.011 | 12 | 191 | 0.010 | 12 | 191 | 0.021 |
| 19:00-20:00 |  |  |  |  |  |  |  |  |  |
| 20:00-21:00 |  |  |  |  |  |  |  |  |  |
| 21:00-22:00 |  |  |  |  |  |  |  |  |  |
| 22:00-23:00 |  |  |  |  |  |  |  |  |  |
| 23:00-24:00 |  |  |  |  |  |  |  |  |  |
| Total Rates: |  |  | 0.081 |  |  | 0.085 |  |  | 0.166 |

This section displays the trip rate results based on the selected set of surveys and the selected count type (shown just above the table). It is split by three main columns, representing arrivals trips, departures trips, and total trips (arrivals plus departures). Within each of these main columns are three sub-columns. These display the number of survey days where count data is included (per time period), the average value of the selected trip rate calculation parameter (per time period), and the trip rate result (per time period). Total trip rates (the sum of the column) are also displayed at the foot of the table.

To obtain a trip rate, the average (mean) trip rate parameter value (TRP) is first calculated for all selected survey days that have count data available for the stated time period. The average (mean) number of arrivals, departures or totals (whichever applies) is also calculated (COUNT) for all selected survey days that have count data available for the stated time period. Then, the average count is divided by the average trip rate parameter value, and multiplied by the stated calculation factor (shown just above the table and abbreviated here as FACT). So, the method is: COUNT/TRP*FACT. Trip rates are then rounded to 3 decimal places.

## TRIP RATE for Land Use 03 - RESIDENTIAL/A - HOUSES PRIVATELY OWNED

MULTI-MODAL VEHICLE OCCUPANTS

## Calculation factor: 1 DWELLS

BOLD print indicates peak (busiest) period


This section displays the trip rate results based on the selected set of surveys and the selected count type (shown just above the table). It is split by three main columns, representing arrivals trips, departures trips, and total trips (arrivals plus departures). Within each of these main columns are three sub-columns. These display the number of survey days where count data is included (per time period), the average value of the selected trip rate calculation parameter (per time period), and the trip rate result (per time period). Total trip rates (the sum of the column) are also displayed at the foot of the table.

To obtain a trip rate, the average (mean) trip rate parameter value (TRP) is first calculated for all selected survey days that have count data available for the stated time period. The average (mean) number of arrivals, departures or totals (whichever applies) is also calculated (COUNT) for all selected survey days that have count data available for the stated time period. Then, the average count is divided by the average trip rate parameter value, and multiplied by the stated calculation factor (shown just above the table and abbreviated here as FACT). So, the method is: COUNT/TRP*FACT. Trip rates are then rounded to 3 decimal places.

TRIP RATE for Land Use 03 - RESIDENTIAL/A - HOUSES PRIVATELY OWNED
MULTI-MODAL PEDESTRIANS

## Calculation factor: 1 DWELLS

BOLD print indicates peak (busiest) period


This section displays the trip rate results based on the selected set of surveys and the selected count type (shown just above the table). It is split by three main columns, representing arrivals trips, departures trips, and total trips (arrivals plus departures). Within each of these main columns are three sub-columns. These display the number of survey days where count data is included (per time period), the average value of the selected trip rate calculation parameter (per time period), and the trip rate result (per time period). Total trip rates (the sum of the column) are also displayed at the foot of the table.

To obtain a trip rate, the average (mean) trip rate parameter value (TRP) is first calculated for all selected survey days that have count data available for the stated time period. The average (mean) number of arrivals, departures or totals (whichever applies) is also calculated (COUNT) for all selected survey days that have count data available for the stated time period. Then, the average count is divided by the average trip rate parameter value, and multiplied by the stated calculation factor (shown just above the table and abbreviated here as FACT). So, the method is: COUNT/TRP*FACT. Trip rates are then rounded to 3 decimal places.

TRIP RATE for Land Use 03 - RESIDENTIAL/A - HOUSES PRIVATELY OWNED
MULTI-MODAL BUS/ TRAM PASSENGERS

## Calculation factor: 1 DWELLS

## BOLD print indicates peak (busiest) period

|  | ARRIVALS |  |  | DEPARTURES |  |  | TOTALS |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Time Range | No. Days | Ave. DWELLS | Trip Rate | No. Days | Ave. DWELLS | Trip Rate | No. Days | Ave. DWELLS | Trip Rate |
| 00:00-01:00 |  |  |  |  |  |  |  |  |  |
| 01:00-02:00 |  |  |  |  |  |  |  |  |  |
| 02:00-03:00 |  |  |  |  |  |  |  |  |  |
| 03:00-04:00 |  |  |  |  |  |  |  |  |  |
| 04:00-05:00 |  |  |  |  |  |  |  |  |  |
| 05:00-06:00 |  |  |  |  |  |  |  |  |  |
| 06:00-07:00 |  |  |  |  |  |  |  |  |  |
| 07:00-08:00 | 12 | 191 | 0.000 | 12 | 191 | 0.013 | 12 | 191 | 0.013 |
| 08:00-09:00 | 12 | 191 | 0.002 | 12 | 191 | 0.025 | 12 | 191 | 0.027 |
| 09:00-10:00 | 12 | 191 | 0.002 | 12 | 191 | 0.009 | 12 | 191 | 0.011 |
| 10:00-11:00 | 12 | 191 | 0.005 | 12 | 191 | 0.004 | 12 | 191 | 0.009 |
| 11:00-12:00 | 12 | 191 | 0.003 | 12 | 191 | 0.003 | 12 | 191 | 0.006 |
| 12:00-13:00 | 12 | 191 | 0.003 | 12 | 191 | 0.003 | 12 | 191 | 0.006 |
| 13:00-14:00 | 12 | 191 | 0.003 | 12 | 191 | 0.004 | 12 | 191 | 0.007 |
| 14:00-15:00 | 12 | 191 | 0.004 | 12 | 191 | 0.003 | 12 | 191 | 0.007 |
| 15:00-16:00 | 12 | 191 | 0.020 | 12 | 191 | 0.009 | 12 | 191 | 0.029 |
| 16:00-17:00 | 12 | 191 | 0.012 | 12 | 191 | 0.006 | 12 | 191 | 0.018 |
| 17:00-18:00 | 12 | 191 | 0.012 | 12 | 191 | 0.003 | 12 | 191 | 0.015 |
| 18:00-19:00 | 12 | 191 | 0.018 | 12 | 191 | 0.006 | 12 | 191 | 0.024 |
| 19:00-20:00 |  |  |  |  |  |  |  |  |  |
| 20:00-21:00 |  |  |  |  |  |  |  |  |  |
| 21:00-22:00 |  |  |  |  |  |  |  |  |  |
| 22:00-23:00 |  |  |  |  |  |  |  |  |  |
| 23:00-24:00 |  |  |  |  |  |  |  |  |  |
| Total Rates: |  |  | 0.084 |  |  | 0.088 |  |  | 0.172 |

This section displays the trip rate results based on the selected set of surveys and the selected count type (shown just above the table). It is split by three main columns, representing arrivals trips, departures trips, and total trips (arrivals plus departures). Within each of these main columns are three sub-columns. These display the number of survey days where count data is included (per time period), the average value of the selected trip rate calculation parameter (per time period), and the trip rate result (per time period). Total trip rates (the sum of the column) are also displayed at the foot of the table.

To obtain a trip rate, the average (mean) trip rate parameter value (TRP) is first calculated for all selected survey days that have count data available for the stated time period. The average (mean) number of arrivals, departures or totals (whichever applies) is also calculated (COUNT) for all selected survey days that have count data available for the stated time period. Then, the average count is divided by the average trip rate parameter value, and multiplied by the stated calculation factor (shown just above the table and abbreviated here as FACT). So, the method is: COUNT/TRP*FACT. Trip rates are then rounded to 3 decimal places.

TRIP RATE for Land Use 03 - RESIDENTIAL/A - HOUSES PRIVATELY OWNED
MULTI-MODAL TOTAL RAIL PASSENGERS

## Calculation factor: 1 DWELLS

## BOLD print indicates peak (busiest) period



This section displays the trip rate results based on the selected set of surveys and the selected count type (shown just above the table). It is split by three main columns, representing arrivals trips, departures trips, and total trips (arrivals plus departures). Within each of these main columns are three sub-columns. These display the number of survey days where count data is included (per time period), the average value of the selected trip rate calculation parameter (per time period), and the trip rate result (per time period). Total trip rates (the sum of the column) are also displayed at the foot of the table.

To obtain a trip rate, the average (mean) trip rate parameter value (TRP) is first calculated for all selected survey days that have count data available for the stated time period. The average (mean) number of arrivals, departures or totals (whichever applies) is also calculated (COUNT) for all selected survey days that have count data available for the stated time period. Then, the average count is divided by the average trip rate parameter value, and multiplied by the stated calculation factor (shown just above the table and abbreviated here as FACT). So, the method is: COUNT/TRP*FACT. Trip rates are then rounded to 3 decimal places.

TRIP RATE for Land Use 03-RESIDENTIAL/A - HOUSES PRIVATELY OWNED
MULTI-MODAL PUBLIC TRANSPORT USERS

## Calculation factor: 1 DWELLS

BOLD print indicates peak (busiest) period

|  | ARRIVALS |  |  | DEPARTURES |  |  | TOTALS |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Time Range | No. Days | Ave. DWELLS | Trip Rate | No. Days | Ave. DWELLS | Trip Rate | No. Days | Ave. DWELLS | Trip Rate |
| 00:00-01:00 |  |  |  |  |  |  |  |  |  |
| 01:00-02:00 |  |  |  |  |  |  |  |  |  |
| 02:00-03:00 |  |  |  |  |  |  |  |  |  |
| 03:00-04:00 |  |  |  |  |  |  |  |  |  |
| 04:00-05:00 |  |  |  |  |  |  |  |  |  |
| 05:00-06:00 |  |  |  |  |  |  |  |  |  |
| 06:00-07:00 |  |  |  |  |  |  |  |  |  |
| 07:00-08:00 | 12 | 191 | 0.003 | 12 | 191 | 0.020 | 12 | 191 | 0.023 |
| 08:00-09:00 | 12 | 191 | 0.002 | 12 | 191 | 0.036 | 12 | 191 | 0.038 |
| 09:00-10:00 | 12 | 191 | 0.003 | 12 | 191 | 0.014 | 12 | 191 | 0.017 |
| 10:00-11:00 | 12 | 191 | 0.005 | 12 | 191 | 0.007 | 12 | 191 | 0.012 |
| 11:00-12:00 | 12 | 191 | 0.003 | 12 | 191 | 0.005 | 12 | 191 | 0.008 |
| 12:00-13:00 | 12 | 191 | 0.003 | 12 | 191 | 0.005 | 12 | 191 | 0.008 |
| 13:00-14:00 | 12 | 191 | 0.003 | 12 | 191 | 0.005 | 12 | 191 | 0.008 |
| 14:00-15:00 | 12 | 191 | 0.005 | 12 | 191 | 0.003 | 12 | 191 | 0.008 |
| 15:00-16:00 | 12 | 191 | 0.025 | 12 | 191 | 0.012 | 12 | 191 | 0.037 |
| 16:00-17:00 | 12 | 191 | 0.015 | 12 | 191 | 0.007 | 12 | 191 | 0.022 |
| 17:00-18:00 | 12 | 191 | 0.017 | 12 | 191 | 0.005 | 12 | 191 | 0.022 |
| 18:00-19:00 | 12 | 191 | 0.026 | 12 | 191 | 0.007 | 12 | 191 | 0.033 |
| 19:00-20:00 |  |  |  |  |  |  |  |  |  |
| 20:00-21:00 |  |  |  |  |  |  |  |  |  |
| 21:00-22:00 |  |  |  |  |  |  |  |  |  |
| 22:00-23:00 |  |  |  |  |  |  |  |  |  |
| 23:00-24:00 |  |  |  |  |  |  |  |  |  |
| Total Rates: |  |  | 0.110 |  |  | 0.126 |  |  | 0.236 |

This section displays the trip rate results based on the selected set of surveys and the selected count type (shown just above the table). It is split by three main columns, representing arrivals trips, departures trips, and total trips (arrivals plus departures). Within each of these main columns are three sub-columns. These display the number of survey days where count data is included (per time period), the average value of the selected trip rate calculation parameter (per time period), and the trip rate result (per time period). Total trip rates (the sum of the column) are also displayed at the foot of the table.

To obtain a trip rate, the average (mean) trip rate parameter value (TRP) is first calculated for all selected survey days that have count data available for the stated time period. The average (mean) number of arrivals, departures or totals (whichever applies) is also calculated (COUNT) for all selected survey days that have count data available for the stated time period. Then, the average count is divided by the average trip rate parameter value, and multiplied by the stated calculation factor (shown just above the table and abbreviated here as FACT). So, the method is: COUNT/TRP*FACT. Trip rates are then rounded to 3 decimal places.

TRIP RATE for Land Use 03 - RESIDENTIAL/A - HOUSES PRIVATELY OWNED
MULTI-MODAL TOTAL PEOPLE

## Calculation factor: 1 DWELLS

BOLD print indicates peak (busiest) period

|  | ARRIVALS |  |  | DEPARTURES |  |  | TOTALS |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Time Range | No. Days | Ave. DWELLS | Trip Rate | No. Days | Ave. DWELLS | Trip Rate | No. Days | Ave. DWELLS | Trip Rate |
| 00:00-01:00 |  |  |  |  |  |  |  |  |  |
| 01:00-02:00 |  |  |  |  |  |  |  |  |  |
| 02:00-03:00 |  |  |  |  |  |  |  |  |  |
| 03:00-04:00 |  |  |  |  |  |  |  |  |  |
| 04:00-05:00 |  |  |  |  |  |  |  |  |  |
| 05:00-06:00 |  |  |  |  |  |  |  |  |  |
| 06:00-07:00 |  |  |  |  |  |  |  |  |  |
| 07:00-08:00 | 12 | 191 | 0.132 | 12 | 191 | 0.552 | 12 | 191 | 0.684 |
| 08:00-09:00 | 12 | 191 | 0.249 | 12 | 191 | 0.864 | 12 | 191 | 1.113 |
| 09:00-10:00 | 12 | 191 | 0.240 | 12 | 191 | 0.337 | 12 | 191 | 0.577 |
| 10:00-11:00 | 12 | 191 | 0.202 | 12 | 191 | 0.281 | 12 | 191 | 0.483 |
| 11:00-12:00 | 12 | 191 | 0.207 | 12 | 191 | 0.273 | 12 | 191 | 0.480 |
| 12:00-13:00 | 12 | 191 | 0.272 | 12 | 191 | 0.251 | 12 | 191 | 0.523 |
| 13:00-14:00 | 12 | 191 | 0.282 | 12 | 191 | 0.256 | 12 | 191 | 0.538 |
| 14:00-15:00 | 12 | 191 | 0.293 | 12 | 191 | 0.332 | 12 | 191 | 0.625 |
| 15:00-16:00 | 12 | 191 | 0.604 | 12 | 191 | 0.329 | 12 | 191 | 0.933 |
| 16:00-17:00 | 12 | 191 | 0.574 | 12 | 191 | 0.316 | 12 | 191 | 0.890 |
| 17:00-18:00 | 12 | 191 | 0.681 | 12 | 191 | 0.261 | 12 | 191 | 0.942 |
| 18:00-19:00 | 12 | 191 | 0.586 | 12 | 191 | 0.359 | 12 | 191 | 0.945 |
| 19:00-20:00 |  |  |  |  |  |  |  |  |  |
| 20:00-21:00 |  |  |  |  |  |  |  |  |  |
| 21:00-22:00 |  |  |  |  |  |  |  |  |  |
| 22:00-23:00 |  |  |  |  |  |  |  |  |  |
| 23:00-24:00 |  |  |  |  |  |  |  |  |  |
| Total Rates: |  |  | 4.322 |  |  | 4.411 |  |  | 8.733 |

This section displays the trip rate results based on the selected set of surveys and the selected count type (shown just above the table). It is split by three main columns, representing arrivals trips, departures trips, and total trips (arrivals plus departures). Within each of these main columns are three sub-columns. These display the number of survey days where count data is included (per time period), the average value of the selected trip rate calculation parameter (per time period), and the trip rate result (per time period). Total trip rates (the sum of the column) are also displayed at the foot of the table.

To obtain a trip rate, the average (mean) trip rate parameter value (TRP) is first calculated for all selected survey days that have count data available for the stated time period. The average (mean) number of arrivals, departures or totals (whichever applies) is also calculated (COUNT) for all selected survey days that have count data available for the stated time period. Then, the average count is divided by the average trip rate parameter value, and multiplied by the stated calculation factor (shown just above the table and abbreviated here as FACT). So, the method is: COUNT/TRP*FACT. Trip rates are then rounded to 3 decimal places.

Appendix E: Traffic Distribution Data

## Selection of areas




## Appendix F: Junctions 9 Outputs:

 Proposed A577 Neverstitch Road Site Access
## Junctions 9

## PICADY 9 - Priority Intersection Module

Version: 9.5.1.7462
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Filename: Site access TA.j9
Path: L:\Projects\CBO-0603 Lathom Pastures Phase 2\Modelling
Report generation date: 16/11/2020 11:23:25

## „Proposed Access - 2025 Base Plus Dev, AM <br> „Proposed Access - 2025 Base Plus Dev, PM

## Summary of junction performance

|  | AM |  |  |  |  |  |  | PM |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Set <br> ID | $\begin{gathered} \text { Q } \\ \text { (Veh) } \end{gathered}$ | Delay <br> (s) | RFC | LOS | Junction Delay (s) | Res Cap | Set <br> ID | $\begin{gathered} \text { Q } \\ \text { (Veh) } \end{gathered}$ | Delay <br> (s) | RFC | LOS | Junction Delay (s) | Res Cap |
|  | Proposed Access - 2025 Base Plus Dev |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Stream B-C | D1 | 0.1 | 7.43 | 0.06 | A | 0.97 | $59 \text { \% }$ <br> [Stream BA] | D2 | 0.0 | 7.50 | 0.03 | A | 0.65 | $63 \text { \% }$ <br> [Stream BA] |
| Stream B-A |  | 0.3 | 14.14 | 0.20 | B |  |  |  | 0.1 | 13.75 | 0.12 | B |  |  |
| Stream C-B |  | 0.0 | 6.11 | 0.02 | A |  |  |  | 0.1 | 6.79 | 0.05 | A |  |  |

Values shown are the highest values encountered over all time segments. Delay is the maximum value of $A v$. delay per arriving vehicle. Junction LOS and Junction Delay are demand-weighted Av.s. Res Cap indicates the amount by which network flow could be increased before a userdefinable threshold (see Analysis Options) is met.

## File summary

File Description

| Title | Proposed Site access |
| :--- | :--- |
| Location |  |
| Site number |  |
| Date | $13 / 11 / 2020$ |
| Version |  |
| Status |  |
| Identifier |  |
| Client |  |
| Jobnumber |  |
| Enumerator | CBO |
| Description |  |

Units

| Distance | Speed | Traffic units | Traffic units | Flow | Av. delay | Total delay | Rate of delay |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |


| units | units | input | results | units | units | units | units |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| m | mph | Veh | Veh | perMin | s | -Min |  |

Analysis Options

| Calculate Q <br> Percentiles | Calculate residual <br> capacity | Residual capacity criteria <br> type | RFC <br> Threshold | Av. Delay threshold <br> (s) | Q threshold <br> (PCU) |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  | $\checkmark$ | Delay | 0.85 | 36.00 | 20.00 |

Demand Set Summary

| ID | Scenario name | Time Period <br> name | Traffic profile <br> type | Start time <br> (HH:mm) | Finish time <br> (HH:mm) | Time segment length <br> (min) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| D1 | 2025 Base Plus Dev | AM | ONE HOUR | $07: 45$ | $09: 15$ | 15 |
| D2 | 2025 Base Plus Dev | PM | ONE HOUR | $16: 45$ | $18: 15$ | 15 |

## Analysis Set Details

| ID | Name | Network flow scaling factor (\%) |
| :---: | :---: | :---: |
| A1 | Proposed Access | 100.000 |

## Proposed Access - 2025 Base Plus Dev, AM

## Data Errors and Warnings

No errors or warnings

## Junction Network

## Junctions

| Junction | Name | Junction type | Major road direction | Use circulating lanes | Junction Delay (s) | Junction LOS |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\mathbf{1}$ | untitled | T-Junction | Two-way |  | 0.97 | A |

## Junction Network Options

| Driving side | Lighting | Res Cap (\%) | First arm reaching threshold |
| :---: | :---: | :---: | :---: |
| Left | Normal/unknown | 59 | Stream B-A |

## Arms

## Arms

| Arm | Name | Description | Arm type |
| :---: | :--- | :--- | :--- |
| A | Neverstitch Rd (SW) |  | Major |
| B | Site Access |  | Minor |
| C | Neverstitch Rd (NE) |  | Major |

## Major Arm Geometry

| Arm | Width of <br> carriageway (m) | Has kerbed central <br> reserve | Has right <br> turn bay | Width for right <br> turn $(\mathbf{m})$ | Visibility for right <br> turn $(\mathbf{m})$ | Blocks? | Blocking queue <br> (PCU) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| C | 7.30 |  | $\checkmark$ | 3.50 | 100.0 |  | - |

Geometries for Arm C are measured opposite Arm B. Geometries for Arm A (if relevant) are measured opposite Arm D.

| Arm | Minor arm <br> type | Width at <br> give-way <br> $(\mathbf{m})$ | Width at <br> $\mathbf{5 m}(\mathbf{m})$ | Width at <br> $\mathbf{1 0 m}(\mathbf{m})$ | Width at <br> $\mathbf{1 5 m}(\mathbf{m})$ | Width at <br> $\mathbf{2 0 m}(\mathbf{m})$ | Estimate <br> flare length | Flare <br> length <br> $(\mathbf{P C U})$ | Visibility to <br> left $(\mathbf{m})$ | Visibility to <br> right $(\mathbf{m})$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| B | One lane <br> plus flare | 10.00 | 4.90 | 3.10 | 2.75 | 2.75 | $\checkmark$ | 1.00 | 26 | 20 |

## Slope / Intercept / Capacity

Priority Intersection Slopes and Intercepts

| Stream | Intercept <br> (Veh/min) | Slope <br> for <br> A-B | Slope <br> for <br> A-C | Slope <br> for <br> C-A | Slope <br> for <br> C-B |
| :---: | :---: | :---: | :---: | :---: | :---: |
| B-A | 8.595 | 0.089 | 0.224 | 0.141 | 0.320 |
| B-C | 10.874 | 0.094 | 0.238 | - | - |
| C-B | 12.021 | 0.264 | 0.264 | - | - |

The slopes and intercepts shown above do NOT include any corrections or adjustments
Streams may be combined, in which case capacity will be adjusted.
Values are shown for the first time segment only; they may differ for subsequent time segments.

## Traffic Demand

Demand Set Details

| ID | Scenario name | Time Period <br> name | Traffic profile <br> type | Start time <br> (HH:mm) | Finish time <br> (HH:mm) | Time segment length <br> (min) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| D1 | 2025 Base Plus Dev | AM | ONE HOUR | $07: 45$ | $09: 15$ | 15 |


| Vehicle mix source | PCU Factor for a HV (PCU) |
| :---: | :---: |
| HV Percentages | 2.00 |

## Demand overview (Traffic)

| Arm | Linked arm | Use O-D data | Av. Demand (Veh/min) | Scaling Factor (\%) |
| :---: | :---: | :---: | :---: | :---: |
| A |  | $\checkmark$ | 6.63 | 100.000 |
| B |  | $\checkmark$ | 1.42 | 100.000 |
| C |  | $\checkmark$ | 9.10 | 100.000 |

## Origin-Destination Data

Demand (Veh/min)

|  | To |  |  |  |
| :---: | :---: | :---: | :---: | :---: |
| From |  | A | B | C |
|  | A | 0.00 | 0.37 | 6.27 |
|  | B | 0.98 | 0.00 | 0.43 |
|  | C | 8.95 | 0.15 | 0.00 |

## Vehicle Mix

|  | To |  |  |  |
| :---: | :---: | :---: | :---: | :---: |
| From |  | A | B | C |
|  | A | 0 | 0 | 6 |
|  | B | 0 | 0 | 0 |
|  | C | 11 | 0 | 0 |

## Detailed Demand Data

Demand for each time segment

| Time Segment | Arm | Demand (Veh/min) | Demand in PCU (PCU/min) |
| :---: | :---: | :---: | :---: |
| $\mathbf{0 7 : 4 5 - 0 8 : 0 0}$ | A | 4.99 | 5.28 |
|  | B | 1.07 | 1.07 |
|  | C | 6.85 | 7.59 |
| $\mathbf{0 8 : 0 0 - 0 8 : 1 5 ~}$ | A | 5.96 | 6.30 |
|  | B | 1.27 | 1.27 |
|  | C | 8.18 | 9.07 |
| $\mathbf{0 8 : 1 5 - 0 8 : 3 0 ~}$ | A | 7.30 | 7.72 |
|  | B | 1.56 | 1.56 |
|  | C | 10.02 | 11.10 |
| $\mathbf{0 8 : 3 0 - 0 8 : 4 5}$ | A | 7.30 | 7.72 |
|  | B | 1.56 | 1.56 |
|  | C | 10.02 | 11.10 |
| $\mathbf{0 8 : 4 5 - 0 9 : 0 0 ~}$ | A | 5.96 | 6.30 |
|  | B | 1.27 | 1.27 |
|  | C | 8.18 | 9.07 |
| $\mathbf{0 9 : 0 0 - 0 9 : 1 5 ~}$ | A | 4.99 | 5.28 |
|  | B | 1.07 | 1.07 |
|  | C | 6.85 | 7.59 |

## Results

Results Summary for whole modelled period

| Stream | Max RFC | Max Delay (s) | Max Q (Veh) | Max LOS |
| :---: | :---: | :---: | :---: | :---: |
| B-C | 0.06 | 7.43 | 0.1 | A |
| B-A | 0.20 | 14.14 | 0.3 | B |
| C-A |  |  |  |  |
| C-B | 0.02 | 6.11 | 0.0 | A |
| A-B |  |  |  |  |
| A-C |  |  |  |  |

07:45-08:00

| Stream | Total Demand <br> (Veh/min) | Capacity <br> (Veh/min) | RFC | Throughput <br> (Veh/min) | End queue <br> (Veh) | Delay (s) | Unsignalised <br> level of service |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| B-C | 0.33 | 9.36 | 0.035 | 0.32 | 0.0 | 6.637 | A |
| B-A | 0.74 | 6.36 | 0.116 | 0.73 | 0.1 | 10.645 | B |
| C-A | 6.74 |  |  | 6.74 |  |  |  |
| C-B | 0.11 | 10.63 | 0.011 | 0.11 | 0.0 | 5.704 | A |
| A-B | 0.28 |  |  | 0.28 |  |  |  |
| A-C | 4.72 |  |  | 4.72 |  |  |  |

08:00-08:15

| Stream | Total Demand <br> (Veh/min) | Capacity <br> (Veh/min) | RFC | Throughput <br> (Veh/min) | End queue <br> (Veh) | Delay (s) | Unsignalised <br> level of service |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| B-C | 0.39 | 9.03 | 0.043 | 0.39 | 0.0 | 6.939 | A |
| B-A | 0.88 | 5.93 | 0.149 | 0.88 | 0.2 | 11.889 | B |
| C-A | 8.05 |  |  | 8.05 |  |  |  |
| C-B | 0.13 | 10.36 | 0.013 | 0.13 | 0.0 | 5.867 | A |
| A-B | 0.33 |  |  | 0.33 |  |  |  |
| A-C | 5.63 |  |  | 5.63 |  |  |  |

08:15-08:30

| Stream | Total Demand <br> (Veh/min) | Capacity <br> (Veh/min) | RFC | Throughput <br> (Veh/min) | End queue <br> (Veh) | Delay (s) | Unsignalised <br> level of service |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| B-C | 0.48 | 8.56 | 0.056 | 0.48 | 0.1 | 7.425 | A |
| B-A | 1.08 | 5.32 | 0.203 | 1.08 | 0.3 | 14.111 | B |
| C-A | 9.85 |  |  | 9.85 |  |  |  |
| C-B | 0.17 | 9.99 | 0.017 | 0.16 | 0.0 | 6.108 | A |
| A-B | 0.40 |  |  | 0.40 |  |  |  |
| A-C | 6.90 |  |  | 6.90 |  |  |  |

08:30-08:45

| Stream | Total Demand <br> (Veh/min) | Capacity <br> (Veh/min) | RFC | Throughput <br> (Veh/min) | End queue <br> (Veh) | Delay (s) | Unsignalised <br> level of service |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| B-C | 0.48 | 8.55 | 0.056 | 0.48 | 0.1 | 7.428 | A |
| B-A | 1.08 | 5.32 | 0.203 | 1.08 | 0.3 | 14.143 | B |
| C-A | 9.85 |  |  | 9.85 |  |  |  |
| C-B | 0.17 | 9.99 | 0.017 | 0.17 | 0.0 | 6.108 | A |
| A-B | 0.40 |  |  | 0.40 |  |  |  |
| A-C | 6.90 |  |  | 6.90 |  |  |  |

08:45-09:00

| Stream | Total Demand <br> (Veh/min) | Capacity <br> (Veh/min) | RFC | Throughput <br> (Veh/min) | End queue <br> (Veh) | Delay (s) | Unsignalised <br> level of service |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| B-C | 0.39 | 9.03 | 0.043 | 0.39 | 0.0 | 6.945 | A |
| B-A | 0.88 | 5.93 | 0.149 | 0.89 | 0.2 | 11.925 | B |
| C-A | 8.05 |  |  | 8.05 |  |  |  |
| C-B | 0.13 | 10.36 | 0.013 | 0.14 | 0.0 | 5.867 | A |
| A-B | 0.33 |  |  | 0.33 |  |  |  |
| A-C | 5.63 |  |  | 5.63 |  |  |  |

09:00-09:15

| Stream | Total Demand <br> (Veh/min) | Capacity <br> (Veh/min) | RFC | Throughput <br> (Veh/min) | End queue <br> (Veh) | Delay (s) | Unsignalised <br> level of service |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| B-C | 0.33 | 9.35 | 0.035 | 0.33 | 0.0 | 6.649 | A |
| B-A | 0.74 | 6.36 | 0.116 | 0.74 | 0.1 | 10.689 | B |
| C-A | 6.74 |  |  | 6.74 |  |  |  |
| C-B | 0.11 | 10.63 | 0.011 | 0.11 | 0.0 | 5.706 | A |
| A-B | 0.28 |  |  | 0.28 |  |  |  |
| A-C | 4.72 |  |  | 4.72 |  |  |  |

## Proposed Access - 2025 Base Plus Dev, PM

## Data Errors and Warnings

No errors or warnings

## Junction Network

## Junctions

| Junction | Name | Junction type | Major road direction | Use circulating lanes | Junction Delay (s) | Junction LOS |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\mathbf{1}$ | untitled | T-Junction | Two-way |  | 0.65 | A |

## Junction Network Options

| Driving side | Lighting | Res Cap (\%) | First arm reaching threshold |
| :---: | :---: | :---: | :---: |
| Left | Normal/unknown | 63 | Stream B-A |

## Traffic Demand

## Demand Set Details

| ID | Scenario name | Time Period <br> name | Traffic profile <br> type | Start time <br> (HH:mm) | Finish time <br> (HH:mm) | Time segment length <br> (min) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| D2 | 2025 Base Plus Dev | PM | ONE HOUR | $16: 45$ | $18: 15$ | 15 |


| Vehicle mix source | PCU Factor for a HV (PCU) |
| :---: | :---: |
| HV Percentages | 2.00 |

Demand overview (Traffic)

| Arm | Linked arm | Use O-D data | Av. Demand (Veh/min) | Scaling Factor (\%) |
| :---: | :---: | :---: | :---: | :---: |
| A |  | $\checkmark$ | 8.78 | 100.000 |
| B |  | $\checkmark$ | 0.80 | 100.000 |
| C |  | $\checkmark$ | 8.17 | 100.000 |

Demand (Veh/min)

|  | To |  |  |  |
| :---: | :---: | :---: | :---: | :---: |
|  |  | A | B | C |
|  | A | 0.00 | 0.90 | 7.88 |
|  | B | 0.55 | 0.00 | 0.25 |
|  | C | 7.75 | 0.42 | 0.00 |

## Vehicle Mix

HV \%s

|  | To |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | A | B | C |  |
|  | A | 0 | 0 | 8 |  |
|  | B | 0 | 0 | 0 |  |
|  | C | 7 | 0 | 0 |  |

## Detailed Demand Data

Demand for each time segment

| Time Segment | Arm | Demand (Veh/min) | Demand in PCU (PCU/min) |
| :---: | :---: | :---: | :---: |
| 16:45-17:00 | A | 6.61 | 7.09 |
|  | B | 0.60 | 0.60 |
|  | C | 6.15 | 6.56 |
| 17:00-17:15 | A | 7.90 | 8.46 |
|  | B | 0.72 | 0.72 |
|  | C | 7.34 | 7.83 |
| 17:15-17:30 | A | 9.67 | 10.37 |
|  | B | 0.88 | 0.88 |
|  | C | 8.99 | 9.59 |
| 17:30-17:45 | A | 9.67 | 10.37 |
|  | B | 0.88 | 0.88 |
|  | C | 8.99 | 9.59 |
| 17:45-18:00 | A | 7.90 | 8.46 |
|  | B | 0.72 | 0.72 |
|  | C | 7.34 | 7.83 |
| 18:00-18:15 | A | 6.61 | 7.09 |
|  | B | 0.60 | 0.60 |
|  | C | 6.15 | 6.56 |

## Results

Results Summary for whole modelled period

| Stream | Max RFC | Max Delay (s) | Max Q (Veh) | Max LOS |
| :---: | :---: | :---: | :---: | :---: |
| B-C | 0.03 | 7.50 | 0.0 | A |


| B-A | 0.12 | 13.75 | 0.1 | B |
| :---: | :---: | :---: | :---: | :---: |
| C-A |  |  |  |  |
| C-B | 0.05 | 6.79 | 0.1 | A |
| A-B |  |  |  |  |
| A-C |  |  |  |  |

## Main Results for each time segment

16:45-17:00

| Stream | Total Demand <br> (Veh/min) | Capacity <br> (Veh/min) | RFC | Throughput <br> (Veh/min) | End queue <br> (Veh) | Delay (s) | Unsignalised <br> level of service |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| B-C | 0.19 | 9.14 | 0.021 | 0.19 | 0.0 | 6.704 | A |
| B-A | 0.41 | 6.12 | 0.068 | 0.41 | 0.1 | 10.506 | B |
| C-A | 5.83 |  |  | 5.83 |  |  |  |
| C-B | 0.31 | 10.15 | 0.031 | 0.31 | 0.0 | 6.095 | A |
| A-B | 0.68 |  |  | 0.68 |  |  |  |
| A-C | 5.93 |  |  | 5.93 |  |  |  |

17:00-17:15

| Stream | Total Demand <br> (Veh/min) | Capacity <br> (Veh/min) | RFC | Throughput <br> (Veh/min) | End queue <br> (Veh) | Delay (s) | Unsignalised <br> (evel of service |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| B-C | 0.22 | 8.78 | 0.026 | 0.22 | 0.0 | 7.012 | A |
| B-A | 0.49 | 5.63 | 0.088 | 0.49 | 0.1 | 11.665 | B |
| C-A | 6.97 |  |  | 6.97 |  |  |  |
| C-B | 0.37 | 9.79 | 0.038 | 0.37 | 0.0 | 6.371 | A |
| A-B | 0.81 |  |  | 0.81 |  |  |  |
| A-C | 7.09 |  |  | 7.09 |  |  |  |

17:15-17:30

| Stream | Total Demand <br> (Veh/min) | Capacity <br> (Veh/min) | RFC | Throughput <br> (Veh/min) | End queue <br> (Veh) | Delay (s) | Unsignalised <br> level of service |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| B-C | 0.28 | 8.28 | 0.033 | 0.27 | 0.0 | 7.495 | A |
| B-A | 0.61 | 4.97 | 0.122 | 0.60 | 0.1 | 13.729 | B |
| C-A | 8.53 |  |  | 8.53 |  |  |  |
| C-B | 0.46 | 9.29 | 0.049 | 0.46 | 0.1 | 6.794 | A |
| A-B | 0.99 |  |  | 0.99 |  |  |  |
| A-C | 8.68 |  |  | 8.68 |  |  |  |

17:30-17:45

| Stream | Total Demand <br> (Veh/min) | Capacity <br> (Veh/min) | RFC | Throughput <br> (Veh/min) | End queue <br> (Veh) | Delay (s) | Unsignalised <br> level of service |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| B-C | 0.28 | 8.28 | 0.033 | 0.28 | 0.0 | 7.497 | A |
| B-A | 0.61 | 4.97 | 0.122 | 0.61 | 0.1 | 13.745 | B |
| C-A | 8.53 |  |  | 8.53 |  |  |  |
| C-B | 0.46 | 9.29 | 0.049 | 0.46 | 0.1 | 6.794 | A |


| A-B | 0.99 |  | 0.99 |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| A-C | 8.68 |  |  | 8.68 |  |  |


| Stream | Total Demand (Veh/min) | Capacity <br> (Veh/min) | RFC | Throughput (Veh/min) | End queue (Veh) | Delay (s) | Unsignalised level of service |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| B-C | 0.22 | 8.78 | 0.026 | 0.23 | 0.0 | 7.019 | A |
| B-A | 0.49 | 5.63 | 0.088 | 0.50 | 0.1 | 11.683 | B |
| C-A | 6.97 |  |  | 6.97 |  |  |  |
| C-B | 0.37 | 9.79 | 0.038 | 0.38 | 0.0 | 6.375 | A |
| A-B | 0.81 |  |  | 0.81 |  |  |  |
| A-C | 7.09 |  |  | 7.09 |  |  |  |

18:00-18:15

| Stream | Total Demand <br> (Veh/min) | Capacity <br> (Veh/min) | RFC | Throughput <br> (Veh/min) | End queue <br> (Veh) | Delay (s) | Unsignalised <br> level of service |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| B-C | 0.19 | 9.13 | 0.021 | 0.19 | 0.0 | 6.709 | A |
| B-A | 0.41 | 6.12 | 0.068 | 0.42 | 0.1 | 10.531 | B |
| C-A | 5.83 |  |  | 5.83 |  |  |  |
| C-B | 0.31 | 10.15 | 0.031 | 0.31 | 0.0 | 6.100 | A |
| A-B | 0.68 |  |  | 0.68 |  |  |  |
| A-C | 5.93 |  |  | 5.93 |  |  |  |

# Appendix G: Junctions 9 Outputs: A577 Neverstitch Road / A577 Ormskirk Road / B5312 Railway Road / Witham Road / Ormskirk Road 

## Junctions 9

## ARCADY 9-Roundabout Module

Version: 9.5.1.7462
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For sales and distribution information, program advice and maintenance, contact TRL: +44 (0)1344379777 software@trl.co.uk www.trlsoftware.co.uk
The users of this computer program for the solution of an engineering problem are in no way relieved of their responsibility for the correctness of the solution

Filename: A577 Neverstitch Rd_A577 Ormskirk Rd_B5312 Railway Rd.j9
Path: L:\Projects\CBO-0603 Lathom Pastures Phase 2\Modelling
Report generation date: 16/11/2020 11:39:31
»Existing Layout - 2020 Adjusted, AM
„Existing Layout - 2020 Adjusted, PM
„Existing Layout - 2025 Base, AM
„Existing Layout - 2025 Base, PM
»Existing Layout - 2025 Base Plus Dev, AM
»Existing Layout - 2025 Base Plus Dev, PM

Summary of junction performance

|  | AM |  |  |  |  |  |  | PM |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Set ID | Q (Veh) | Delay (s) | RFC | LOS | Junction Delay (s) | Res Cap | Set ID | Q (Veh) | Delay (s) | RFC | LOS | Junction Delay (s) | Res Cap |
|  | Existing Layout - 2020 Adjusted |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Arm A | D1 | 0.6 | 3.88 | 0.37 | A | 3.90 | $98 \text { \% }$ <br> [Arm E] | D2 | 0.4 | 3.34 | 0.28 | A | 4.15 | $72 \text { \% }$ <br> [Arm E] |
| Arm C |  | 0.4 | 4.71 | 0.30 | A |  |  |  | 0.5 | 4.77 | 0.35 | A |  |  |
| Arm D |  | 0.4 | 2.51 | 0.29 | A |  |  |  | 0.3 | 2.33 | 0.25 | A |  |  |
| Arm E |  | 0.7 | 5.21 | 0.40 | A |  |  |  | 0.9 | 6.17 | 0.46 | A |  |  |
|  | Existing Layout - 2025 Base |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Arm A | D3 | 0.7 | 4.21 | 0.41 | A | 4.33 | 72 \% <br> [Arm E] | D4 | 0.4 | 3.52 | 0.31 | A | 4.64 | 55 \% <br> [Arm E] |
| Arm C |  | 0.5 | 5.06 | 0.33 | A |  |  |  | 0.7 | 5.32 | 0.40 | A |  |  |
| Arm D |  | 0.5 | 2.62 | 0.32 | A |  |  |  | 0.4 | 2.48 | 0.28 | A |  |  |
| Arm E |  | 0.9 | 6.03 | 0.47 | A |  |  |  | 1.1 | 7.14 | 0.53 | A |  |  |
|  | Existing Layout - 2025 Base Plus Dev |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Arm A | D5 | 0.8 | 4.51 | 0.45 | A | 4.49 | $68 \text { \% }$ <br> [Arm E] | D6 | 0.5 | 3.63 | 0.33 | A | 4.81 | $48 \text { \% }$ <br> [Arm E] |
| Arm C |  | 0.5 | 5.26 | 0.34 | A |  |  |  | 0.7 | 5.44 | 0.41 | A |  |  |
| Arm D |  | 0.5 | 2.67 | 0.33 | A |  |  |  | 0.4 | 2.56 | 0.30 | A |  |  |
| Arm E |  | 0.9 | 6.16 | 0.48 | A |  |  |  | 1.2 | 7.64 | 0.56 | A |  |  |

There are warnings associated with one or more model runs - see the 'Data Errors and Warnings' tables for each Analysis or Demand Set.
Values shown are the highest values encountered over all time segments. Delay is the maximum value of Av. delay per arriving vehicle. Junction LOS and Junction Delay are demand-weighted Av.s. Res Cap indicates the amount by which network flow could be increased before a userdefinable threshold (see Analysis Options) is met.

## File summary

File Description

| Title | A577 Neverstitch Rd / A577 Ormskirk Rd / B5312 Railway Rd / Witham Rd / Ormskirk Rd |
| :--- | :--- |
| Location |  |
| Site number |  |
| Date | $16 / 11 / 2020$ |
| Version |  |
| Status |  |
| Identifier |  |
| Client |  |
| Jobnumber | $\mathrm{CBO}-0603$ |
| Enumerator | CBO |
| Description |  |

Units

| Distance <br> units | Speed <br> units | Traffic units <br> input | Traffic units <br> results | Flow <br> units | Av. delay <br> units | Total delay <br> units | Rate of delay <br> units |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| m | kph | Veh | Veh | perMin | s | - Min | perMin |

## Analysis Options

| Calculate Q <br> Percentiles | Calculate residual <br> capacity | Residual capacity criteria <br> type | RFC <br> Threshold | Av. Delay threshold <br> (s) | Q threshold <br> (PCU) |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  | $\checkmark$ | Delay | 0.85 | 36.00 | 20.00 |

## Demand Set Summary

| ID | Scenario name | Time Period <br> name | Traffic profile <br> type | Start time <br> (HH:mm) | Finish time <br> (HH:mm) | Time period <br> length (min) | Time segment <br> length (min) |
| :--- | :--- | :---: | :---: | :---: | :---: | :---: | :---: |
| D1 | 2020 Adjusted | AM | DIRECT | $08: 00$ | $09: 00$ | 60 | 15 |
| D2 | 2020 Adjusted | PM | DIRECT | $16: 15$ | $17: 15$ | 60 | 15 |
| D3 | 2025 Base | AM | DIRECT | $08: 00$ | $09: 00$ | 60 | 15 |
| D4 | 2025 Base | PM | DIRECT | $16: 15$ | $17: 15$ | 60 | 15 |
| D5 | 2025 Base Plus Dev | AM | DIRECT | $08: 00$ | $09: 00$ | 60 | 15 |
| D6 | 2025 Base Plus Dev | PM | DIRECT | $16: 15$ | $17: 15$ | 60 | 15 |

## Analysis Set Details

| ID | Name | Network flow scaling factor (\%) |
| :---: | :---: | :---: |
| A1 | Existing Layout | 100.000 |

## Existing Layout - 2020 Adjusted, AM

Data Errors and Warnings

| Severity | Area | Item | Description |
| :--- | :--- | :--- | :--- |
| Warning | Geometry | Arm E- <br> Roundabout <br> Geometry | Effective flare length is over 30 m, which is outside the normal range. Treat capacities with <br> increasing caution. |
| Warning | Profile Type | D1-2020 <br> Adjusted, AM | The DIRECT profile type is intended to be used for demand that varies over time. You are <br> using it with the 'Use O-D data' option, but your O-D data does not vary over time. Are you <br> sure this is correct? |

Junction Network

## Junctions

| Junction | Name | Junction type | Use circulating lanes | Arm order | Junction Delay (s) | Junction LOS |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\mathbf{1}$ | untitled | Standard Roundabout |  | A, B, C, D, E | 3.90 | A |

Junction Network Options

| Driving side | Lighting | Res Cap (\%) | First arm reaching threshold |
| :---: | :---: | :---: | :---: |
| Left | Normal/unknown | 98 | Arm E |

## Arms

## Arms

| Arm | Name | Description |
| :---: | :--- | :--- |
| A | A577 Neverstitch Rd |  |
| B | Clayton St |  |
| C | Witham Rd |  |
| D | B5312 Railway Rd |  |
| E | A577 Ormskirk Rd |  |

Roundabout Geometry

| Arm | $\mathbf{V}(\mathbf{m})$ | $\mathbf{E}(\mathbf{m})$ | $\mathbf{l}(\mathbf{m})$ | $\mathbf{R ( m )}$ | $\mathbf{D}(\mathbf{m})$ | PHI (deg) | Exit only |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| A | 3.65 | 7.40 | 19.0 | 40.0 | 70.0 | 38.0 |  |
| B |  |  |  |  |  |  | $\checkmark$ |
| C | 3.95 | 4.90 | 12.0 | 28.0 | 70.0 | 25.0 |  |
| D | 6.85 | 7.40 | 26.0 | 70.0 | 70.0 | 22.0 |  |
| E | 3.21 | 4.55 | 40.0 | 24.0 | 70.0 | 26.0 |  |

## Slope / Intercept / Capacity

Roundabout Slope and Intercept used in model

| Arm | Final slope | Final intercept (PCU/min) |
| :---: | :---: | :---: |
| A | 0.520 | 29.940 |
| B |  |  |
| C | 0.477 | 24.520 |
| D | 0.626 | 39.526 |
| E | 0.459 | 22.814 |

The slope and intercept shown above include any corrections and adjustments.

## Traffic Demand

## Demand Set Details

| ID | Scenario <br> name | Time Period <br> name | Traffic profile <br> type | Start time <br> $(H H: m m)$ | Finish time <br> (HH:mm) | Time period length <br> $(\mathbf{m i n})$ | Time segment length <br> $(\mathbf{m i n})$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| D1 | 2020 Adjusted | AM | DIRECT | $08: 00$ | $09: 00$ | 60 | 15 |


| Vehicle mix source | PCU Factor for a HV (PCU) |
| :---: | :---: |
| HV Percentages | 2.00 |

## Demand overview (Traffic)

| Arm | Linked arm | Use O-D data | Scaling Factor (\%) |
| :---: | :---: | :---: | :---: |
| A |  |  | 100.000 |
| B |  |  |  |


| C |  |  | 100.000 |
| :--- | :--- | :--- | :--- |
| D |  |  | 100.000 |
| E |  |  | 100.000 |

## Origin-Destination Data

Demand (Veh/min)

|  | To |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| From |  | $\mathbf{A}$ | $\mathbf{B}$ | $\mathbf{C}$ | $\mathbf{D}$ | $\mathbf{E}$ |  |
|  | A | 0.03 | 0.33 | 2.50 | 1.77 | 3.68 |  |
|  | B | Exit-only | Exit-only | Exit-only | Exit-only | Exit-only |  |
|  | C | 1.65 | 0.08 | 0.00 | 1.75 | 1.13 |  |
|  | D | 1.93 | 0.40 | 2.50 | 0.07 | 4.15 |  |
|  | E | 2.00 | 0.20 | 1.23 | 3.30 | 0.02 |  |

## Vehicle Mix

HV \%s

|  | To |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| From |  | A | B | C | D | E |  |
|  | A | 0 | 32 | 3 | 4 | 3 |  |
|  | B | Exit-only | Exit-only | Exit-only | Exit-only | Exit-only |  |
|  | C | 9 | 20 | 0 | 6 | 11 |  |
|  | D | 6 | 9 | 4 | 0 | 2 |  |
|  | E | 3 | 55 | 0 | 4 | 0 |  |

## Results

Results Summary for whole modelled period

| Arm | Max RFC | Max Delay (s) | Max Q (Veh) | Max LOS |
| :---: | :---: | :---: | :---: | :---: |
| A | 0.37 | 3.88 | 0.6 | A |
| B |  |  |  |  |
| C | 0.30 | 4.71 | 0.4 | A |
| D | 0.29 | 2.51 | 0.4 | A |
| E | 0.40 | 5.21 | 0.7 | A |

Main Results for each time segment
08:00-08:15

| Arm | Total Demand <br> (Veh/min) | Circulating <br> flow (Veh/min) | Capacity <br> (Veh/min) | RFC | Throughput <br> (Veh/min) | End queue <br> (Veh) | Delay (s) | Unsignalised <br> level of service |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| A | 7.35 | 7.72 | 24.65 | 0.298 | 7.32 | 0.4 | 3.457 | A |
| B |  | 14.06 |  |  |  |  |  |  |
| C | 3.78 | 7.95 | 18.97 | 0.199 | 3.76 | 0.2 | 3.941 | A |
| D | 10.08 | 5.62 | 34.52 | 0.292 | 10.05 | 0.4 | 2.450 | A |


| E | 6.23 | 6.89 | 18.63 | 0.334 | 6.20 | 0.5 | 4.813 | A |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |

08:15-08:30

| Arm | Total Demand <br> $(\mathbf{V e h} / \mathbf{m i n})$ | Circulating <br> flow (Veh/min) | Capacity <br> $(\mathbf{V e h} / \mathbf{m i n})$ | RFC | Throughput <br> $(\mathbf{V e h} / \mathbf{m i n})$ | End queue <br> $($ Veh $)$ | Delay (s) | Unsignalised <br> level of service |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| A | 8.05 | 7.63 | 24.69 | 0.326 | 8.05 | 0.5 | 3.603 | A |
| B |  | 14.65 |  |  |  |  |  |  |
| C | 4.83 | 8.37 | 18.78 | 0.257 | 4.82 | 0.3 | 4.297 | A |
| D | 9.94 | 6.61 | 33.88 | 0.293 | 9.94 | 0.4 | 2.505 | A |
| E | 6.09 | 7.23 | 18.47 | 0.330 | 6.09 | 0.5 | 4.849 | A |

08:30-08:45

| Arm | Total Demand <br> (Veh/min) | Circulating <br> flow (Veh/min) | Capacity <br> (Veh/min) | RFC | Throughput <br> $($ Veh/min) | End queue <br> (Veh) | Delay (s) | Unsignalised <br> level of service |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| A | 8.68 | 8.04 | 24.48 | 0.355 | 8.68 | 0.5 | 3.795 | A |
| B |  | 15.67 |  |  |  |  |  |  |
| C | 4.48 | 9.26 | 18.38 | 0.244 | 4.48 | 0.3 | 4.320 | A |
| D | 9.10 | 6.68 | 33.84 | 0.269 | 9.10 | 0.4 | 2.426 | A |
| E | 7.07 | 6.65 | 18.74 | 0.377 | 7.06 | 0.6 | 5.136 | A |

08:45-09:00

| Arm | Total Demand <br> (Veh/min) | Circulating <br> flow (Veh/min) | Capacity <br> $($ Veh/min) | RFC | Throughput <br> $($ Veh/min) | End queue <br> (Veh) | Delay (s) | Unsignalised <br> level of service |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| A | 9.17 | 7.73 | 24.64 | 0.372 | 9.17 | 0.6 | 3.877 | A |
| B |  | 15.90 |  |  |  |  |  |  |
| C | 5.39 | 9.81 | 18.12 | 0.297 | 5.38 | 0.4 | 4.707 | A |
| D | 7.07 | 7.46 | 33.33 | 0.212 | 7.08 | 0.3 | 2.285 | A |
| E | 7.56 | 5.89 | 19.08 | 0.396 | 7.56 | 0.7 | 5.205 | A |

## Existing Layout - 2020 Adjusted, PM

Data Errors and Warnings

| Severity | Area | Item | Description |
| :--- | :--- | :--- | :--- |
| Warning | Geometry | Arm E - <br> Roundabout <br> Geometry | Effective flare length is over 30m, which is outside the normal range. Treat capacities with <br> increasing caution. |
| Warning | Profile Type | D2-2020 <br> Adjusted, PM | The DIRECT profile type is intended to be used for demand that varies over time. You are <br> using it with the 'Use O-D data' option, but your O-D data does not vary over time. Are you <br> sure this is correct? |

## Junction Network

## Junctions

| Junction | Name | Junction type | Use circulating lanes | Arm order | Junction Delay (s) | Junction LOS |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\mathbf{1}$ | untitled | Standard Roundabout |  | A, B, C, D, E | 4.15 | A |

## Junction Network Options

| Driving side | Lighting | Res Cap (\%) | First arm reaching threshold |
| :---: | :---: | :---: | :---: |
| Left | Normal/unknown | 72 | Arm E |

## Traffic Demand

Demand Set Details

| ID | Scenario <br> name | Time Period <br> name | Traffic profile <br> type | Start time <br> (HH:mm) | Finish time <br> (HH:mm) | Time period length <br> (min) | Time segment length <br> (min) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| D2 | 2020 Adjusted | PM | DIRECT | $16: 15$ | $17: 15$ | 60 | 15 |


| Vehicle mix source | PCU Factor for a HV (PCU) |
| :---: | :---: |
| HV Percentages | 2.00 |

Demand overview (Traffic)

| Arm | Linked arm | Use O-D data | Scaling Factor (\%) |
| :---: | :---: | :---: | :---: |
| A |  |  | 100.000 |
| B |  |  |  |
| C |  |  | 100.000 |
| D |  |  | 100.000 |
| E |  |  | 100.000 |

## Origin-Destination Data

Demand (Veh/min)

|  | To |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| From |  | A | B | C | D | $\mathbf{E}$ |  |
|  | A | 0.02 | 0.53 | 1.77 | 2.23 | 2.32 |  |
|  | B | Exit-only | Exit-only | Exit-only | Exit-only | Exit-only |  |
|  | C | 2.40 | 0.12 | 0.00 | 2.28 | 1.32 |  |
|  | D | 2.02 | 0.50 | 2.18 | 0.13 | 2.92 |  |
|  | E | 2.68 | 0.42 | 0.92 | 3.58 | 0.00 |  |

## Vehicle Mix

HV \%s

|  | To |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| From |  | A | B | C | D | E |  |
|  | A | 0 | 15 | 1 | 0 | 1 |  |
|  | B | Exit-only | Exit-only | Exit-only | Exit-only | Exit-only |  |
|  | C | 3 | 0 | 0 | 2 | 12 |  |
|  | D | 4 | 4 | 0 | 71 | 1 |  |
|  | E | 2 | 20 | 2 | 6 | 0 |  |

## Results

Results Summary for whole modelled period

| Arm | Max RFC | Max Delay (s) | Max Q (Veh) | Max LOS |
| :---: | :---: | :---: | :---: | :---: |
| A | 0.28 | 3.34 | 0.4 | A |
| B |  |  |  |  |
| C | 0.35 | 4.77 | 0.5 | A |
| D | 0.25 | 2.33 | 0.3 | A |
| E | 0.46 | 6.17 | 0.9 | A |

## Main Results for each time segment

| 16:15-16:30 |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Arm | Total Demand <br> $($ Veh $/ \mathbf{m i n})$ | Circulating <br> flow (Veh/min) | Capacity <br> $(\mathbf{V e h} / \mathbf{m i n})$ | RFC | Throughput <br> $($ Veh $/ \mathbf{m i n})$ | End queue <br> $($ Veh $)$ | Delay (s) | Unsignalised <br> level of service |
| A | 7.12 | 7.71 | 25.26 | 0.282 | 7.09 | 0.4 | 3.298 | A |
| B |  | 13.28 |  |  |  |  |  |  |
| C | 5.44 | 8.62 | 19.38 | 0.281 | 5.41 | 0.4 | 4.289 | A |
| D | 6.64 | 5.80 | 34.74 | 0.191 | 6.62 | 0.2 | 2.133 | A |
| E | 8.08 | 6.38 | 18.86 | 0.428 | 8.03 | 0.7 | 5.513 | A |

16:30-16:45

| Arm | Total Demand <br> $($ Veh/min) | Circulating <br> flow (Veh/min) | Capacity <br> $(\mathbf{V e h} / \mathbf{m i n})$ | RFC | Throughput <br> $(\mathbf{V e h} / \mathbf{m i n})$ | End queue <br> $($ Veh $)$ | Delay (s) | Unsignalised <br> level of service |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| A | 6.96 | 7.64 | 25.31 | 0.275 | 6.96 | 0.4 | 3.269 | A |
| B |  | 13.03 |  |  |  |  |  |  |
| C | 6.40 | 8.13 | 19.61 | 0.326 | 6.39 | 0.5 | 4.539 | A |
| D | 8.00 | 6.37 | 34.37 | 0.233 | 8.00 | 0.3 | 2.274 | A |
| E | 7.12 | 7.63 | 18.29 | 0.389 | 7.13 | 0.6 | 5.377 | A |

16:45-17:00

| Arm | Total Demand <br> $($ Veh/min) | Circulating <br> flow (Veh/min) | Capacity <br> $(\mathbf{V e h} / \mathbf{m i n})$ | RFC | Throughput <br> $($ Veh/min) | End queue <br> $($ Veh $)$ | Delay (s) | Unsignalised <br> level of service |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| A | 6.64 | 7.39 | 25.44 | 0.261 | 6.64 | 0.4 | 3.191 | A |
| B |  | 12.52 |  |  |  |  |  |  |
| C | 5.84 | 7.76 | 19.78 | 0.295 | 5.84 | 0.4 | 4.308 | A |
| D | 7.92 | 5.92 | 34.66 | 0.228 | 7.92 | 0.3 | 2.244 | A |
| E | 6.80 | 7.36 | 18.42 | 0.369 | 6.80 | 0.6 | 5.167 | A |

17:00 - 17:15

| Arm | Total Demand <br> $($ Veh/min) | Circulating <br> flow (Veh/min) | Capacity <br> $($ Veh/min) | RFC | Throughput <br> $($ Veh/min) | End queue <br> $($ Veh $)$ | Delay (s) | Unsignalised <br> level of service |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| A | 6.80 | 8.63 | 24.77 | 0.275 | 6.80 | 0.4 | 3.338 | A |
| B |  | 13.77 |  |  |  |  |  |  |
| C | 6.80 | 8.62 | 19.36 | 0.351 | 6.79 | 0.5 | 4.770 | A |
| D | 8.48 | 6.57 | 34.25 | 0.248 | 8.48 | 0.3 | 2.328 | A |
| E | 8.40 | 8.10 | 18.08 | 0.465 | 8.38 | 0.9 | 6.173 | A |

## Existing Layout - 2025 Base, AM

Data Errors and Warnings

| Severity | Area | Item | Description |
| :--- | :--- | :--- | :--- |
| Warning | Geometry | Arm E- <br> Roundabout <br> Geometry | Effective flare length is over 30m, which is outside the normal range. Treat capacities with <br> increasing caution. |
| Warning | Profile Type | D3-2025 Base, <br> AM | The DIRECT profile type is intended to be used for demand that varies over time. You are <br> using it with the 'Use O-D data' option, but your O-D data does not vary over time. Are you <br> sure this is correct? |

## Junctions

| Junction | Name | Junction type | Use circulating lanes | Arm order | Junction Delay (s) | Junction LOS |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\mathbf{1}$ | untitled | Standard Roundabout |  | A, B, C, D, E | 4.33 | A |

Junction Network Options

| Driving side | Lighting | Res Cap (\%) | First arm reaching threshold |
| :---: | :---: | :---: | :---: |
| Left | Normal/unknown | 72 | Arm E |

## Traffic Demand

## Demand Set Details

| ID | Scenario <br> name | Time Period <br> name | Traffic profile <br> type | Start time <br> (HH:mm) | Finish time <br> (HH:mm) | Time period length <br> (min) | Time segment length <br> $(\mathbf{m i n})$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| D3 | 2025 Base | AM | DIRECT | $08: 00$ | $09: 00$ | 60 | 15 |


| Vehicle mix source | PCU Factor for a HV (PCU) |
| :---: | :---: |
| HV Percentages | 2.00 |

Demand overview (Traffic)

| Arm | Linked arm | Use O-D data | Scaling Factor (\%) |
| :---: | :---: | :---: | :---: |
| A |  |  | 100.000 |
| B |  |  |  |
| C |  |  | 100.000 |
| D |  |  | 100.000 |
| E |  |  | 100.000 |

## Origin-Destination Data

Demand (Veh/min)

|  | To |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| From |  | $\mathbf{A}$ | $\mathbf{B}$ | $\mathbf{C}$ | $\mathbf{D}$ | $\mathbf{E}$ |  |
|  | A | 0.03 | 0.35 | 2.63 | 1.87 | 3.98 |  |
|  | B | Exit-only | Exit-only | Exit-only | Exit-only | Exit-only |  |
|  | C | 1.73 | 0.08 | 0.00 | 1.85 | 1.28 |  |
|  | D | 2.03 | 0.42 | 2.63 | 0.07 | 4.52 |  |
|  | E | 2.37 | 0.23 | 1.55 | 3.98 | 0.02 |  |

## Vehicle Mix

HV \%s

|  | To |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| From |  | A | B | C | D | E |  |
|  | A | 0 | 32 | 3 | 4 | 3 |  |
|  | B | Exit-only | Exit-only | Exit-only | Exit-only | Exit-only |  |
|  | C | 9 | 20 | 0 | 6 | 11 |  |
|  | D | 6 | 9 | 4 | 0 | 2 |  |
|  | E | 3 | 55 | 0 | 4 | 0 |  |

## Results

Results Summary for whole modelled period

| Arm | Max RFC | Max Delay (s) | Max Q (Veh) | Max LOS |
| :---: | :---: | :---: | :---: | :---: |
| A | 0.41 | 4.21 | 0.7 | A |
| B |  |  |  |  |
| C | 0.33 | 5.06 | 0.5 | A |
| D | 0.32 | 2.62 | 0.5 | A |
| E | 0.47 | 6.03 | 0.9 | A |

## Main Results for each time segment

08:00-08:15

| Arm | Total Demand <br> $($ Veh/min) | Circulating <br> flow (Veh/min) | Capacity <br> $(\mathbf{V e h} / \mathbf{m i n})$ | RFC | Throughput <br> $($ Veh/min) | End queue <br> $($ Veh $)$ | Delay (s) | Unsignalised <br> level of service |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| A | 7.85 | 8.89 | 24.04 | 0.326 | 7.82 | 0.5 | 3.689 | A |
| B |  | 15.65 |  |  |  |  |  |  |
| C | 4.06 | 8.97 | 18.50 | 0.219 | 4.04 | 0.3 | 4.144 | A |
| D | 10.77 | 6.09 | 34.23 | 0.315 | 10.74 | 0.5 | 2.551 | A |
| E | 7.60 | 7.23 | 18.48 | 0.411 | 7.55 | 0.7 | 5.467 | A |

08:15-08:30

| Arm | Total Demand <br> (Veh/min) | Circulating <br> flow (Veh/min) | Capacity <br> $(\mathbf{V e h} / \mathbf{m i n})$ | RFC | Throughput <br> $($ Veh/min) | End queue <br> (Veh) | Delay (s) | Unsignalised <br> level of service |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| A | 8.58 | 8.80 | 24.09 | 0.356 | 8.58 | 0.5 | 3.866 | A |
| B |  | 16.28 |  |  |  |  |  |  |
| C | 5.17 | 9.42 | 18.30 | 0.283 | 5.16 | 0.4 | 4.566 | A |
| D | 10.63 | 7.13 | 33.55 | 0.317 | 10.63 | 0.5 | 2.617 | A |
| E | 7.45 | 7.59 | 18.31 | 0.407 | 7.45 | 0.7 | 5.524 | A |

08:30-08:45

| Arm | Total Demand <br> $(\mathbf{V e h} / \mathbf{m i n})$ | Circulating <br> flow (Veh/min) | Capacity <br> $(\mathbf{V e h} / \mathbf{m i n})$ | RFC | Throughput <br> $(\mathbf{V e h} / \mathbf{m i n})$ | End queue <br> $($ Veh $)$ | Delay (s) | Unsignalised <br> level of service |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| A | 9.25 | 9.23 | 23.86 | 0.388 | 9.24 | 0.6 | 4.103 | A |
| B |  | 17.37 |  |  |  |  |  |  |
| C | 4.80 | 10.36 | 17.87 | 0.269 | 4.80 | 0.4 | 4.593 | A |
| D | 9.74 | 7.21 | 33.51 | 0.291 | 9.74 | 0.4 | 2.524 | A |
| E | 8.48 | 6.99 | 18.59 | 0.456 | 8.47 | 0.8 | 5.924 | A |

08:45-09:00

| Arm | Total Demand <br> (Veh/min) | Circulating <br> flow (Veh/min) | Capacity <br> $($ Veh/min) | RFC | Throughput <br> $($ Veh/min) | End queue <br> (Veh) | Delay (s) | Unsignalised <br> level of service |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| A | 9.76 | 8.93 | 24.02 | 0.406 | 9.76 | 0.7 | 4.206 | A |
| B |  | 17.62 |  |  |  |  |  |  |
| C | 5.76 | 10.94 | 17.60 | 0.327 | 5.75 | 0.5 | 5.059 | A |
| D | 7.60 | 8.04 | 32.97 | 0.231 | 7.61 | 0.3 | 2.366 | A |
| E | 9.00 | 6.20 | 18.95 | 0.475 | 9.00 | 0.9 | 6.025 | A |

## Existing Layout - 2025 Base, PM

Data Errors and Warnings

| Severity | Area | Item | Description |
| :--- | :--- | :--- | :--- |
| Warning | Geometry | Arm E- <br> Roundabout <br> Geometry | Effective flare length is over 30m, which is outside the normal range. Treat capacities with <br> increasing caution. |
| Warning | Profile Type | D4-2025 Base, <br> PM | The DIRECT profile type is intended to be used for demand that varies over time. You are <br> using it with the 'Use O-D data' option, but your O-D data does not vary over time. Are you <br> sure this is correct? |

## Junction Network

## Junctions

| Junction | Name | Junction type | Use circulating lanes | Arm order | Junction Delay (s) | Junction LOS |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\mathbf{1}$ | untitled | Standard Roundabout |  | A, B, C, D, E | 4.64 | A |

## Junction Network Options

| Driving side | Lighting | Res Cap (\%) | First arm reaching threshold |
| :---: | :---: | :---: | :---: |
| Left | Normal/unknown | 55 | Arm E |

## Traffic Demand

Demand Set Details

| ID | Scenario <br> name | Time Period <br> name | Traffic profile <br> type | Start time <br> $(\mathbf{H H}: \mathbf{m m})$ | Finish time <br> (HH:mm) | Time period length <br> (min) | Time segment length <br> (min) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| D4 | 2025 Base | PM | DIRECT | $16: 15$ | $17: 15$ | 60 | 15 |


| Vehicle mix source | PCU Factor for a HV (PCU) |
| :---: | :---: |
| HV Percentages | 2.00 |

Demand overview (Traffic)

| Arm | Linked arm | Use O-D data | Scaling Factor (\%) |
| :---: | :--- | :--- | :---: |
| A |  |  | 100.000 |
| B |  |  |  |
| C |  |  | 100.000 |
| D |  |  | 100.000 |
| E |  |  | 100.000 |

## Origin-Destination Data

Demand (Veh/min)

|  | To |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| From |  | A | B | C | D | $\mathbf{E}$ |  |
|  | A | 0.02 | 0.57 | 1.87 | 2.35 | 2.55 |  |
|  | B | Exit-only | Exit-only | Exit-only | Exit-only | Exit-only |  |
|  | C | 2.53 | 0.12 | 0.00 | 2.40 | 1.83 |  |
|  | D | 2.12 | 0.53 | 2.30 | 0.13 | 3.68 |  |
|  | E | 3.08 | 0.47 | 1.07 | 4.10 | 0.00 |  |

## Vehicle Mix

|  | To |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| From |  | A | $\mathbf{B}$ | $\mathbf{C}$ | $\mathbf{D}$ | $\mathbf{E}$ |  |
|  | A | 0 | 15 | 1 | 0 | 1 |  |
|  | B | Exit-only | Exit-only | Exit-only | Exit-only | Exit-only |  |
|  | C | 3 | 0 | 0 | 2 | 12 |  |
|  | D | 4 | 4 | 0 | 71 | 1 |  |
|  | E | 2 | 20 | 2 | 6 | 0 |  |

## Results

Results Summary for whole modelled period

| Arm | Max RFC | Max Delay (s) | Max Q (Veh) | Max LOS |
| :---: | :---: | :---: | :---: | :---: |
| A | 0.31 | 3.52 | 0.4 | A |
| B |  |  |  |  |
| C | 0.40 | 5.32 | 0.7 | A |
| D | 0.28 | 2.48 | 0.4 | A |
| E | 0.53 | 7.14 | 1.1 | A |

## Main Results for each time segment

16:15-16:30

| Arm | Total Demand <br> (Veh/min) | Circulating <br> flow (Veh/min) | Capacity <br> $(\mathbf{V e h} / \mathbf{m i n})$ | RFC | Throughput <br> $($ Veh/min) | End queue <br> (Veh) | Delay (s) | Unsignalised <br> level of service |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| A | 7.61 | 8.60 | 24.79 | 0.307 | 7.58 | 0.4 | 3.481 | A |
| B |  | 14.54 |  |  |  |  |  |  |
| C | 6.17 | 9.50 | 18.87 | 0.327 | 6.14 | 0.5 | 4.701 | A |
| D | 7.61 | 6.64 | 34.26 | 0.222 | 7.59 | 0.3 | 2.249 | A |
| E | 9.23 | 6.78 | 18.69 | 0.494 | 9.17 | 1.0 | 6.262 | A |

16:30-16:45

| Arm | Total Demand <br> (Veh/min) | Circulating <br> flow (Veh/min) | Capacity <br> $($ Veh/min) | RFC | Throughput <br> (Veh/min) | End queue <br> (Veh) | Delay (s) | Unsignalised <br> level of service |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| A | 7.44 | 8.50 | 24.85 | 0.299 | 7.44 | 0.4 | 3.446 | A |
| B |  | 14.25 |  |  |  |  |  |  |
| C | 7.18 | 8.99 | 19.11 | 0.376 | 7.17 | 0.6 | 5.023 | A |
| D | 9.04 | 7.27 | 33.85 | 0.267 | 9.03 | 0.4 | 2.418 | A |
| E | 8.22 | 8.02 | 18.13 | 0.453 | 8.23 | 0.8 | 6.068 | A |

16:45-17:00

| Arm | Total Demand <br> $(\mathbf{V e h} / \mathbf{m i n})$ | Circulating <br> flow (Veh/min) | Capacity <br> $(\mathbf{V e h} / \mathbf{m i n})$ | RFC | Throughput <br> $(\mathbf{V e h} / \mathbf{m i n})$ | End queue <br> $($ Veh $)$ | Delay (s) | Unsignalised <br> level of service |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| A | 7.11 | 8.24 | 24.99 | 0.285 | 7.11 | 0.4 | 3.356 | A |
| B |  | 13.72 |  |  |  |  |  |  |
| C | 6.59 | 8.60 | 19.29 | 0.342 | 6.59 | 0.5 | 4.728 | A |
| D | 8.95 | 6.78 | 34.16 | 0.262 | 8.95 | 0.4 | 2.379 | A |
| E | 7.88 | 7.75 | 18.25 | 0.432 | 7.88 | 0.8 | 5.791 | A |

17:00-17:15

| Arm | Total Demand <br> (Veh/min) | Circulating <br> flow (Veh/min) | Capacity <br> (Veh/min) | RFC | Throughput <br> (Veh/min) | End queue <br> (Veh) | Delay (s) | Unsignalised <br> level of service |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| A | 7.27 | 9.53 | 24.29 | 0.299 | 7.27 | 0.4 | 3.524 | A |
| B |  | 15.01 |  |  |  |  |  |  |
| C | 7.60 | 9.50 | 18.86 | 0.403 | 7.59 | 0.7 | 5.321 | A |
| D | 9.54 | 7.48 | 33.71 | 0.283 | 9.54 | 0.4 | 2.482 | A |
| E | 9.57 | 8.47 | 17.92 | 0.534 | 9.55 | 1.1 | 7.143 | A |

## Existing Layout - 2025 Base Plus Dev, AM

Data Errors and Warnings

| Severity | Area | Item | Description |
| :--- | :--- | :--- | :--- |
| Warning | Geometry | Arm E - <br> Roundabout <br> Geometry | Effective flare length is over 30 m, which is outside the normal range. Treat capacities with <br> increasing caution. |
| Warning | Profile Type | D5 -2025 Base <br> Plus Dev, AM | The DIRECT profile type is intended to be used for demand that varies over time. You are <br> using it with the 'Use O-D data' option, but your O-D data does not vary over time. Are you <br> sure this is correct? |

Junction Network

## Junctions

| Junction | Name | Junction type | Use circulating lanes | Arm order | Junction Delay (s) | Junction LOS |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\mathbf{1}$ | untitled | Standard Roundabout |  | A, B, C, D, E | 4.49 | A |

## Junction Network Options

| Driving side | Lighting | Res Cap (\%) | First arm reaching threshold |
| :---: | :---: | :---: | :---: |
| Left | Normal/unknown | 68 | Arm E |

## Traffic Demand

## Demand Set Details

| ID | Scenario name | Time Period <br> name | Traffic profile <br> type | Start time <br> (HH:mm) | Finish time <br> (HH:mm) | Time period <br> length (min) | Time segment <br> length (min) |
| :--- | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| D5 | 2025 Base Plus Dev | AM | DIRECT | $08: 00$ | $09: 00$ | 60 | 15 |


| Vehicle mix source | PCU Factor for a HV (PCU) |
| :---: | :---: |
| HV Percentages | 2.00 |

Demand overview (Traffic)

| Arm | Linked arm | Use O-D data | Scaling Factor (\%) |
| :---: | :---: | :---: | :---: |
| A |  |  | 100.000 |
| B |  |  |  |
| C |  |  | 100.000 |
| D |  |  | 100.000 |
| E |  |  | 100.000 |

## Origin-Destination Data

## Demand (Veh/min)

|  | To |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| From |  | $\mathbf{A}$ | $\mathbf{B}$ | $\mathbf{C}$ | $\mathbf{D}$ | $\mathbf{E}$ |  |
|  | A | 0.03 | 0.35 | 2.63 | 2.53 | 4.30 |  |
|  | B | Exit-only | Exit-only | Exit-only | Exit-only | Exit-only |  |
|  | C | 1.73 | 0.08 | 0.00 | 1.85 | 1.28 |  |
|  | D | 2.28 | 0.42 | 2.63 | 0.07 | 4.52 |  |
|  | E | 2.48 | 0.23 | 1.55 | 3.98 | 0.02 |  |

## Vehicle Mix

HV \%s

|  | To |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| From |  | A | $\mathbf{B}$ | $\mathbf{C}$ | $\mathbf{D}$ | $\mathbf{E}$ |  |
|  | A | 0 | 32 | 3 | 4 | 3 |  |
|  | B | Exit-only | Exit-only | Exit-only | Exit-only | Exit-only |  |
|  | C | 9 | 20 | 0 | 6 | 11 |  |
|  | D | 6 | 9 | 4 | 0 | 2 |  |
|  | E | 3 | 55 | 0 | 4 | 0 |  |

## Results

## Results Summary for whole modelled period

| Arm | Max RFC | Max Delay (s) | Max Q (Veh) | Max LOS |
| :---: | :---: | :---: | :---: | :---: |
| A | 0.45 | 4.51 | 0.8 | A |
| B |  |  |  |  |
| C | 0.34 | 5.26 | 0.5 | A |
| D | 0.33 | 2.67 | 0.5 | A |
| E | 0.48 | 6.16 | 0.9 | A |

## Main Results for each time segment

08:00 - 08:15

| Arm | Total Demand <br> $($ Veh/min) | Circulating <br> flow (Veh/min) | Capacity <br> $(\mathbf{V e h} / \mathbf{m i n})$ | RFC | Throughput <br> $(\mathbf{V e h} / \mathbf{m i n})$ | End queue <br> $($ Veh $)$ | Delay (s) | Unsignalised <br> level of service |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| A | 8.83 | 8.89 | 24.06 | 0.367 | 8.79 | 0.6 | 3.920 | A |
| B |  | 16.62 |  |  |  |  |  |  |
| C | 4.06 | 9.92 | 18.07 | 0.225 | 4.04 | 0.3 | 4.272 | A |
| D | 11.02 | 6.41 | 34.01 | 0.324 | 10.99 | 0.5 | 2.603 | A |
| E | 7.72 | 7.50 | 18.36 | 0.420 | 7.67 | 0.7 | 5.588 | A |

08:15-08:30

| Arm | Total Demand <br> $($ Veh $/ \mathrm{min})$ | Circulating <br> flow (Veh/min) | Capacity <br> $(\mathrm{Veh} / \mathrm{min})$ | RFC | Throughput <br> $($ Veh $/ \mathrm{min})$ | End queue <br> $($ Veh $)$ | Delay (s) | Unsignalised <br> level of service |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| A | 9.56 | 8.80 | 24.10 | 0.397 | 9.55 | 0.7 | 4.123 | A |


| B |  | 17.26 |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| C | 5.17 | 10.40 | 17.85 | 0.290 | 5.16 | 0.4 | 4.725 | A |
| D | 10.88 | 7.45 | 33.33 | 0.326 | 10.88 | 0.5 | 2.671 | A |
| E | 7.57 | 7.85 | 18.19 | 0.416 | 7.57 | 0.7 | 5.650 | A |

08:30 - 08:45

| Arm | Total Demand <br> $($ Veh $/ \mathbf{m i n})$ | Circulating <br> flow (Veh/min) | Capacity <br> $($ Veh $/ \mathbf{m i n})$ | RFC | Throughput <br> $($ Veh/min) | End queue <br> $($ Veh $)$ | Delay (s) | Unsignalised <br> level of service |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| A | 10.23 | 9.23 | 23.88 | 0.428 | 10.22 | 0.7 | 4.391 | A |
| B |  | 18.35 |  |  |  |  |  |  |
| C | 4.80 | 11.35 | 17.42 | 0.276 | 4.80 | 0.4 | 4.756 | A |
| D | 9.99 | 7.52 | 33.30 | 0.300 | 9.99 | 0.4 | 2.574 | A |
| E | 8.60 | 7.24 | 18.48 | 0.465 | 8.59 | 0.9 | 6.061 | A |

08:45-09:00

| Arm | Total Demand <br> $($ Veh/min) | Circulating <br> flow (Veh/min) | Capacity <br> $($ Veh/min) | RFC | Throughput <br> $($ Veh/min) | End queue <br> $($ Veh $)$ | Delay (s) | Unsignalised <br> level of service |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| A | 10.74 | 8.94 | 24.03 | 0.447 | 10.74 | 0.8 | 4.513 | A |
| B |  | 18.61 |  |  |  |  |  |  |
| C | 5.76 | 11.95 | 17.15 | 0.336 | 5.75 | 0.5 | 5.263 | A |
| D | 7.85 | 8.34 | 32.76 | 0.240 | 7.86 | 0.3 | 2.409 | A |
| E | 9.12 | 6.43 | 18.85 | 0.484 | 9.12 | 0.9 | 6.163 | A |

## Existing Layout - 2025 Base Plus Dev, PM

Data Errors and Warnings

| Severity | Area | Item | Description |
| :--- | :--- | :--- | :--- |
| Warning | Geometry | Arm E - <br> Roundabout <br> Geometry | Effective flare length is over 30m, which is outside the normal range. Treat capacities with <br> increasing caution. |
| Warning | Profile Type | D6-2025 Base <br> Plus Dev, PM | The DIRECT profile type is intended to be used for demand that varies over time. You are <br> using it with the 'Use O-D data' option, but your O-D data does not vary over time. Are you <br> sure this is correct? |

## Junction Network

## Junctions

| Junction | Name | Junction type | Use circulating lanes | Arm order | Junction Delay (s) | Junction LOS |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\mathbf{1}$ | untitled | Standard Roundabout |  | A, B, C, D, E | 4.81 | A |

## Junction Network Options

| Driving side | Lighting | Res Cap (\%) | First arm reaching threshold |
| :---: | :---: | :---: | :---: |
| Left | Normal/unknown | 48 | Arm E |

## Traffic Demand

Demand Set Details

| ID | Scenario name | Time Period <br> name | Traffic profile <br> type | Start time <br> $\mathbf{( H H : m m )}$ | Finish time <br> (HH:mm) | Time period <br> length $(\mathbf{m i n})$ | Time segment <br> length $(\mathbf{m i n})$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| D6 | 2025 Base Plus Dev | PM | DIRECT | $16: 15$ | $17: 15$ | 60 | 15 |


| Vehicle mix source | PCU Factor for a HV (PCU) |
| :---: | :---: |
| HV Percentages | 2.00 |

Demand overview (Traffic)

| Arm | Linked arm | Use O-D data | Scaling Factor (\%) |
| :---: | :---: | :---: | :---: |
| A |  |  | 100.000 |
| B |  |  |  |
| C |  |  | 100.000 |
| D |  |  | 100.000 |
| E |  |  | 100.000 |

## Origin-Destination Data

Demand (Veh/min)

|  | To |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| From |  | A | B | C | $\mathbf{D}$ | $\mathbf{E}$ |  |
|  | A | 0.02 | 0.57 | 1.87 | 2.72 | 2.73 |  |
|  | B | Exit-only | Exit-only | Exit-only | Exit-only | Exit-only |  |
|  | C | 2.53 | 0.12 | 0.00 | 2.40 | 1.83 |  |
|  | D | 2.73 | 0.53 | 2.30 | 0.13 | 3.68 |  |
|  | E | 3.37 | 0.47 | 1.07 | 4.10 | 0.00 |  |

## Vehicle Mix

HV \%s

|  | To |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| From |  | A | B | C | D | E |  |
|  | A | 0 | 15 | 1 | 0 | 1 |  |
|  | B | Exit-only | Exit-only | Exit-only | Exit-only | Exit-only |  |
|  | C | 3 | 0 | 0 | 2 | 12 |  |
|  | D | 4 | 4 | 0 | 71 | 1 |  |
|  | E | 2 | 20 | 2 | 6 | 0 |  |

## Results

Results Summary for whole modelled period

| Arm | Max RFC | Max Delay (s) | Max Q (Veh) | Max LOS |
| :---: | :---: | :---: | :---: | :---: |
| A | 0.33 | 3.63 | 0.5 | A |
| B |  |  |  |  |
| C | 0.41 | 5.44 | 0.7 | A |
| D | 0.30 | 2.56 | 0.4 | A |
| E | 0.56 | 7.64 | 1.2 | A |

16:15-16:30

| Arm | Total Demand <br> (Veh/min) | Circulating <br> flow (Veh/min) | Capacity <br> $($ Veh/min) | RFC | Throughput <br> $($ Veh/min) | End queue <br> (Veh) | Delay (s) | Unsignalised <br> level of service |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| A | 8.16 | 8.61 | 24.80 | 0.329 | 8.13 | 0.5 | 3.592 | A |
| B |  | 15.09 |  |  |  |  |  |  |
| C | 6.17 | 10.04 | 18.62 | 0.331 | 6.14 | 0.5 | 4.793 | A |
| D | 8.23 | 6.83 | 34.12 | 0.241 | 8.21 | 0.3 | 2.313 | A |
| E | 9.51 | 7.37 | 18.44 | 0.516 | 9.44 | 1.0 | 6.620 | A |

16:30-16:45

| Arm | Total Demand <br> $($ Veh/min) | Circulating <br> flow (Veh/min) | Capacity <br> $($ Veh/min) | RFC | Throughput <br> $($ Veh/min) | End queue <br> $($ Veh $)$ | Delay (s) | Unsignalised <br> level of service |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| A | 7.99 | 8.50 | 24.87 | 0.321 | 7.99 | 0.5 | 3.557 | A |
| B |  | 14.81 |  |  |  |  |  |  |
| C | 7.18 | 9.54 | 18.85 | 0.381 | 7.17 | 0.6 | 5.132 | A |
| D | 9.66 | 7.45 | 33.71 | 0.287 | 9.65 | 0.4 | 2.494 | A |
| E | 8.50 | 8.64 | 17.86 | 0.476 | 8.51 | 0.9 | 6.423 | A |

16:45-17:00

| Arm | Total Demand <br> $($ Veh/min) | Circulating <br> flow (Veh/min) | Capacity <br> $($ Veh/min) | RFC | Throughput <br> $($ Veh/min) | End queue <br> $($ Veh $)$ | Delay (s) | Unsignalised <br> level of service |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| A | 7.66 | 8.25 | 25.00 | 0.306 | 7.66 | 0.4 | 3.459 | A |
| B |  | 14.28 |  |  |  |  |  |  |
| C | 6.59 | 9.16 | 19.03 | 0.346 | 6.60 | 0.5 | 4.826 | A |
| D | 9.57 | 6.96 | 34.02 | 0.281 | 9.57 | 0.4 | 2.455 | A |
| E | 8.16 | 8.37 | 17.98 | 0.454 | 8.17 | 0.8 | 6.118 | A |

17:00-17:15

| Arm | Total Demand <br> $($ Veh/min) | Circulating <br> flow (Veh/min) | Capacity <br> $($ Veh/min) | RFC | Throughput <br> $($ Veh/min) | End queue <br> $($ Veh $)$ | Delay (s) | Unsignalised <br> level of service |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| A | 7.82 | 9.49 | 24.33 | 0.321 | 7.82 | 0.5 | 3.632 | A |
| B |  | 15.53 |  |  |  |  |  |  |
| C | 7.60 | 10.03 | 18.62 | 0.408 | 7.59 | 0.7 | 5.437 | A |
| D | 10.16 | 7.67 | 33.57 | 0.303 | 10.16 | 0.4 | 2.562 | A |
| E | 9.85 | 9.11 | 17.65 | 0.558 | 9.82 | 1.2 | 7.643 | A |

