## **Structural Engineers**

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T M Ventham
Practice

## Construction of New Residential Apartments at Air Balloon Road

## Design for the Disposal of Surface Water

For Mooshead Ltd

Ref 7451

February 2021

7451 Air Balloon Road February 2021

The proposed development will occupy a site that was once occupied by several terraced houses. The terraced houses were demolished some considerable time ago, and the site has been recently used as a location for an advertising hoarding.

The approved development occupies most of the site, with a landscaped area at one end. Most of the site is occupied by the building and some small courtyard areas. Drawing 7451-SK00 (copy appended) shows the impermeable catchment area of the site, which has been calculated as  $257\text{m}^2$ .

Some boreholes and trial holes have been sunk on the site, and the underlying subsoil has been found to be stiff red-brown clay over layers of sandstone and mudstone, i.e. all of low permeability.

With soil of low permeability and with the development occupying a high percentage of the site, soakaway drainage is not a realistic possibility. Any attempt at soakaway would not be able to comply with the building regulation requirement to be 5m from a building or boundary.

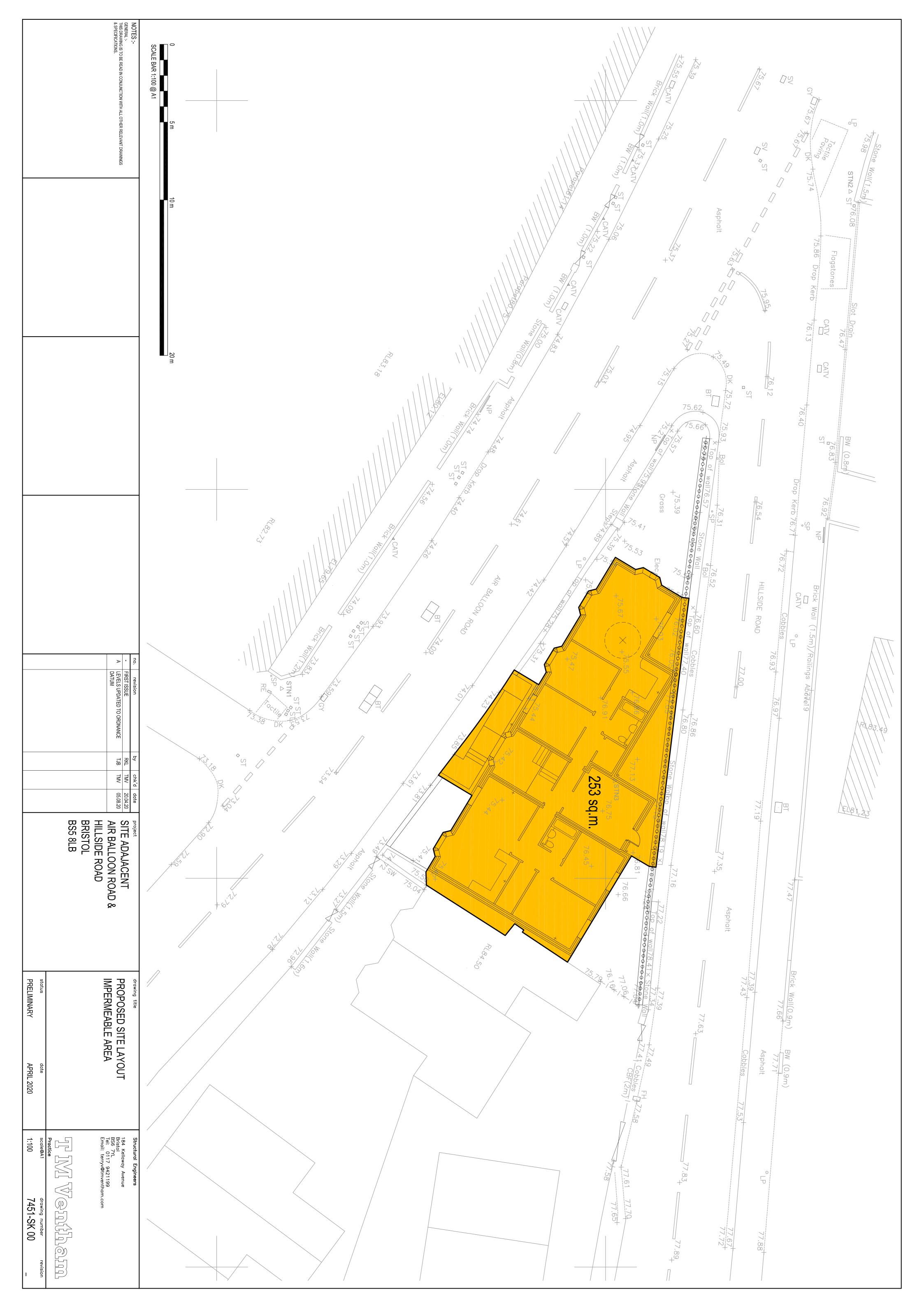
The project has been discussed with Wessex Water, who have agreed to allow the surface water to be taken to a nearby combined sewer, provided that the flow rate is attenuated to 2L/s.

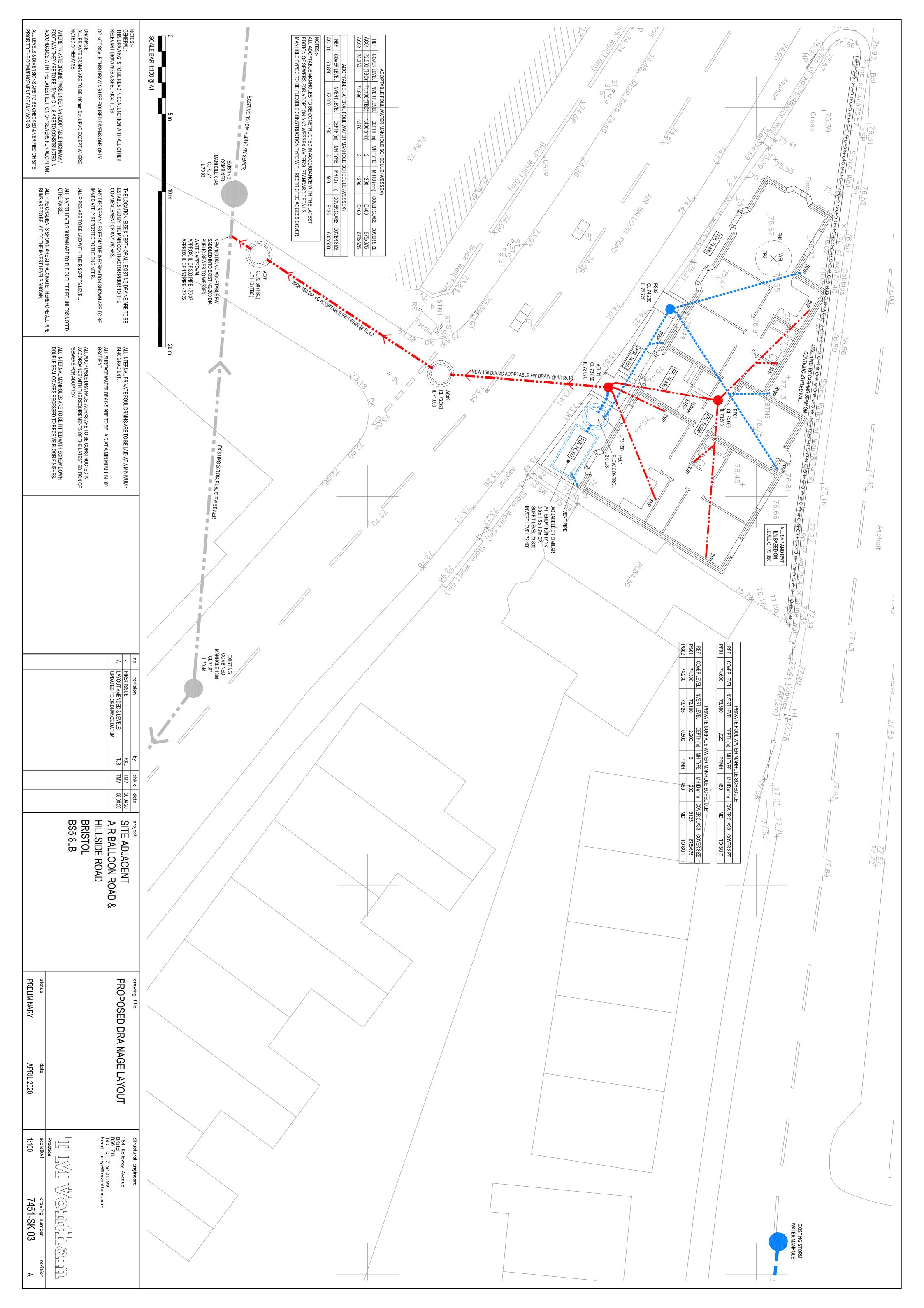
An attenuation tank has been designed based on a 100 years storm plus 40% with the agreed discharge rate of 2L/s. See attenuation tank design sheet attached. Also see drawing 7451-SK03. This drawing shows the drainage layout. This shows the location of a 7.2m<sup>3</sup> attenuation tank, and a flow control manhole with a discharge to the combined sewer at a rate of 2L/s.

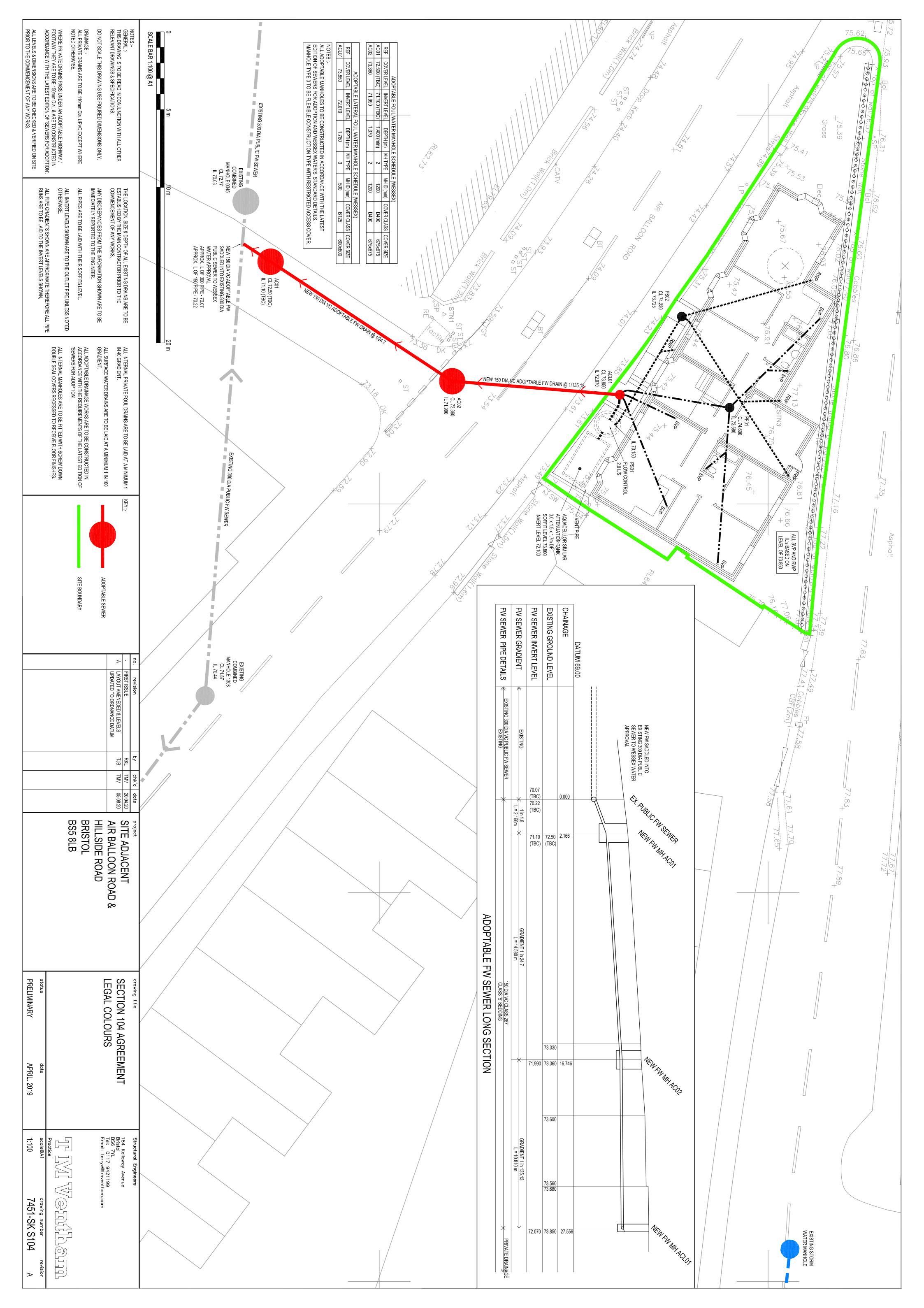
See also drawing 7451-SK104, which shows the details of the adoptable sewer which have been agreed with Wessex.



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Job: Structural Engineers

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Job number Sheet date

**7451** 02A 22/02/21 T M Ventham

**Practice** 

## **Attenuation Tank Design**

Return Period 100 Years

+ 40%

Location England and Wales

Ratio of 60 minute to 2 day rainfalls of 5 year return period (BRE digest 365-fig 1) r = 0.35

Impermeable area (sq.m.)257Outflow from tank in litres/second2Depth of tank (m)1.6

Duration	M5	Growth factor	Return Period	Inflow	Outflow	Storage
	rainfalls	Z2	rainfall	m3	m3	required m3
5mins	7.10	1.86	13.22	4.76	0.60	4.16
10mins	10.10	1.93	19.45	7.00	1.20	5.80
15mins	12.30	1.96	24.08	8.67	1.80	6.87
30mins	15.70	2.00	31.37	11.29	3.60	7.69
1 hour	20.00	2.03	40.60	14.61	7.20	7.41
2 hour	24.40	2.01	49.04	17.65	14.40	3.25
4 hour	29.90	1.97	58.90	21.19	28.80	-7.61
6 hour	33.80	1.94	65.50	23.57	43.20	-19.63
10 hour	38.90	1.90	73.83	26.56	72.00	-45.44
24 hour	49.60	1.81	89.78	32.30	172.80	-140.50

Note:- If Orifice plate used then only 50% of outflow rate used

Max storage required = 7.69 cubic metres

√(Max storage required/tank depth) 2.19 Square Tank required

Use Hydrobrake at 2 Litres / Second

Use  $3.0 \times 1.5 \times 1.6 \text{m deep} = 7.20 \text{ cubic metres}$ 

Note additional storage provided in the Flow Control manhole = 1.80 cubic metres

Total storrage provided 7.20 + 1.8 = 9.0 cubic metres.