



WIDE AND 1.5m DEEP INTO THE CHALK GEOLOGY (FINAL DEPTH TO BE DETERMINED ON SITE AT CONSTRUCTION STAGE). AN ASSUMED 5X10⁻⁴m/s (0.18m/hr) INFILTRATION RATE WAS APPLIED AT THE BASE/SIDES OF THE FILTER DRAIN TO DETERMINE APPROXIMATELY THE ATTENUATION VOLUME REQUIREMENTS.

> THE FILTER DRAIN SHOULD BE BUILT WITH 6-20mm CRUSHED STONE AGGREGATE (0.3 VOID RATIO).

SEE SWALES - SOUTH EAST CALCULATIONS RESULTS. FINAL CL/IL VARIES BASED ON LOCAL CONDITIONS. CURRENT TOTAL CONSTRUCTION DEPTH: 1.75m.

f	A IBAA STORAGE SITE WITH CONCRETE PAD FOUNDATION AND LAGOON FOR WATER STORAGE AND REUSE FOR DUST		b	CONFIRMED ON SITE. b. THIS DRAWING IS TO BE READ IN CONJUNCTION WITH ALL RELEVANT ARCHITECTURAL DRAWINGS DETAILED SPECIFICATIONS WHERE
/	THE REDLINE APPLICATION AREA IS APPROXIMATELY 21480m ² .	/	с	APPLICABLE AND ALL ASSOCIATED DRAWINGS IN THIS SERIES. c. ANY DISCREPANCY ON THIS DRAWING IS TO BE REPORTED IMMEDIATELY TO THE DAPTNERSHID FOR CLAPIEICATION
	THE RUNOFF GENERATED ON THE PROPOSED SITE IS DEEMED HAZARDOUS TO THE ENVIRONMENT AND DISCHARGES TO THE	3	c	 d. THE CONTRACTOR IS RESPONSIBLE FOR ALL TEMPORARY WORKS AND FOR THE STABILITY OF THE WORKS IN PROGRESS.
	GROUND OR WATERCOURSE ARE NOT FEASIBLE. ANY RUNOFF GENERATED WITHIN THE CONFINES OF THE EARTH BUNDS		e	e. CDM REGULATIONS 2015. ALL CURRENT DRAWINGS AND SPECIFICATIONS MUST BE READ IN CONJUNCTION WITH THE DESIGNER'S HAZARD RISK AND
	WOULD BE REUSED ON-SITE AND IN A WORST-CASE SCENARIO TANKERED TO A SPECIALIST TREATMENT FACILITY. (1/5)	/		ON INFORMATION PROVIDED BY THE CLIENT/PRINCIPLE DESIGNER AVAILABLE AT TIME OF ISSUE. CONTRACTOR TO REVIEW DRAWING AND
	THE PRINCIPAL CATCHMENT AREA OF THE SITE IS 18210m ² (1.821ha) AND IS COMPRISED BY THE CONCRETE			SPECIFICATION IN CONTEXT WITH THE WIDER SITE AND SPECIFIC SITE INVESTIGATION, CONTAMINATION ASSESSMENT, ASBESTOS SURVEY,
	AREA AND LAGOON, AND INTERNAL BUND SLOPE AREA.			INFORMATION AND MANAGE RISKS RELATING TO THE WORKS OUTLINED IN THE DRAWINGS AND SPECIFICATION. PRINCIPLE CONTRACTOR TO MAKE
	THE REMAINING EXTERNAL BUND AREAS (LIGHT RED HATCH) WOULD CONSTITUTE A SECONDARY CATCHMENT OF 2710m ² .		f	DESIGNER AND CLIENT AWARE OF SITE SPECIFIC RISKS THAT MAY AFFECT THE DRAWING AND SPECIFICATION. f CDM REGULATIONS 2015 FOR GENERIC MAINTENANCE AND MANAGEMENT
	DUE TO THE STEEP SLOPE OF THE BUNDS, THE RESPECTIVE AREAS WOULD BE CONSIDERED AS IMPERMEABLE IN			RISKS REFER TO CHAPTER 36 OF CIRIA 752 SUDS MANUAL. FOR PROPRIETARY SYSTEMS SEE MANUFACTURER'S MANAGEMENT AND
	CALCULATIONS. THE TOTAL STORAGE REQUIREMENT FOR THE PROPOSED			MAINTENANCE DETAILS AND RISK ASSESSMENT WITH REGARDS TO MAINTENANCE OF PROPRIETARY SYSTEMS.
	IMPERMEABLE AREA OF 18210m ² (INC. LAGOON AND BUNDS SLOPES) DURING THE 7 DAYS, 1 IN 100 YEAR + 40%CC EVENT IS		2 a	 CONSTRUCTION NOTE THE MAIN CONTRACTOR IS RESPONSIBLE FOR THE DESIGN OF ALL TEMPORARY WORKS AND IS ALSO RESPONSIBLE FOR THE CASE
Ly .	2861m°. THE PROPOSED LAGOON WOULD HOLD APPROXIMATELY		k	MAINTENANCE AND STABILITY OF EXISTING BUILDINGS AT ALL TIMES. b. THE MAIN CONTRACTOR IS RESPONSIBLE FOR ALL OCCURRENCES OF GROUND
	1654m ³ OF THE ATTENUATION VOLUME REQUIREMENT AND THE REMAINING 1207m ³ WOULD BE ACCOMMODATED IN THE		c	WATER DURING THE CONSTRUCTION PERIOD. c. ANY INFORMATION GIVEN REGARDING EXISTING UNDERGROUND SERVICES IS GIVEN. IN GOOD FAITH AFTER CONSULTATION, WITH THE RELEVANT
	CALCULATIONS (A) SHEETS. (2/5)			AUTHORITY, HOWEVER ACCURACY IS NOT CERTAIN. THE MAIN CONTRACTOR IS RESPONSIBLE FOR CHECKING ALL INFORMATION ON SITE
	ANALYSIS OF THE MET OFFICE MORECS CLIMATE DATA AT THE LOCATION OF THE SITE, INDICATES THAT IN ANY GIVEN		c	 PRIOR TO WORK COMMENCING AND TAKING DUE CARE AND ATTENTION WHILST UNDERTAKING THE WORKS. d. THE CONTRACTOR MUST COMPLY WITH ALL CURRENT LEGISLATION RELATING
88	AVERAGE TEAK THE COMPARISON BETWEEN TOTAL KAINFALL AND POTENTIAL EVAPORATION RESULTS IN A TOTAL WATER SURPLUS OF 255mm.		e	TO HEALTH & SAFETY. e. ALL PRODUCTS SPECIFIED SHALL BE INSTALLED IN STRICT ACCORDANCE WITH
32	THE CALCULATION USES POTENTIAL EVAPORATION RATE WAS			ARE DISCREPANCIES BETWEEN THAT INFORMATION AND THE DETAILS ON ANY AMBIENTAL DRAWINGS, THE MANUFACTURERS INSTRUCTIONS MUST
K	WOULD SPRAY THE IBAA STOCKPILE ON A REGULAR BASIS FOR DUST SUPPRESSION. ALSO, THE IBAA'S POROSITY WOULD		3	BE USED 3. BELOW GROUND DRAINAGE
	INCREASE THE OVERALL EXPOSED SURFACE AREA WHICH INCREASES THE REAL EVAPORATION RATE OF THE SITE. FOR CONSERVATIVE REASONS THIS ELEMENT SHOULD NOT BE		a	a. PIPEWORK TO BE UPVC-U PIPES TO BS 4660 : 2000 AND INSPECTION CHAMBERS TO BS 7158 : 2001.
.95	ACCOUNTED FOR IN CALCULATIONS GIVEN THE VARYING IBAA STOCKPILE LEVELS ACROSS THE YEAR.)Z 	b	 ALL ADOPTABLE DRAINAGE TO BE CONSTRUCTED IN ACCORDANCE WITH 'SEWERS FOR ADOPTION' 7TH EDITION AND THE RELEVANT COUNCIL DESIGN GUIDE.
	IT IS SUGGESTED TO PROVIDE ADDITIONAL STORAGE FOR 255mm WATER DEPTH, OR 4644m ³ (18210m ² x 0.255m). AS	-	c	c. ALL PRIVATE SURFACE WATER SEWERS TO BE LAID AT 1 IN 100 UNLESS OTHERWISE STATED ON THE DRAWING.
	SUCH, IN THE CIRCUMSTANCE THAT IN A PARTICULAR YEAR THERE WOULD BE NO REUSE OF WATER, THE SITE TO HAVE CAPACITY TO ACCOMMODATE A 1 IN 100 YEAR + 40% STORM		e	 a. ALL PRIVATE FOOL WATER SEWERS TO BE LAID AT I IN 40 AT THE HEAD OF PIPE RUNS AND 1 IN 80 ELSEWHERE UNLESS OTHERWISE STATED. e. ALL PRIVATE FOUL SEWER PIPES TO BE 150MM DIAMETER UNLESS
	EVENT.	7	^	OTHERWISE STATED ON THE DRAWING. ALL PRIVATE SURFACE WATER SEWER PIPES TO BE 100MM DIAMETER FROM DOWNPIPES AND 150MM DIAMETER ELSEWHERE LINESS OTHERWISE STATED ON THE DRAWING
	THE DESIGN RUNOFF ATTENUATION VOLUME IS COMPRISED OF THE 7 DAY, 1 IN 100 YEAR + 40%CC EVENT PLUS THE AVERAGE YEARLY WATER SURPLUS, TOTALING		f	f. ALLOW FOR RODDING ACCESS ABOVE GROUND WHERE RAINWATER DOWNPIPES DO NOT HAVE A DIRECT CONNECTION TO AN INSPECTION
	APPROXIMATELY 7505m ³ .			CHAMBER. EXISTING SEWER PIPE TO BE RE-USED TO BE SURVEYED AND LEVELLED PRIOR TO COMMENCEMENT OF THE DRAINAGE WORKS AND REFURBISHED IF NECESSARY.
	THE 61.5m CONTOUR. (3/5)	33	ę	g. CONNECTIONS TO AN ADOPTED SEWER ONLY TO BE MADE FOLLOWING APPROVAL FROM THE RELEVANT ADOPTING AUTHORITY.
	THE WETTEST YEAR SINCE 1855 (ACCORDING TO MET OFFICE HISTORICAL DATA) WAS IN 1960 WHEN 1166mm OF RAINFALL WAS RECORDED FOR THIS YEAR THE NET SUBPLUS OF RUNOFF		r	n. ALL DRAINS, SEWER PIPES AND MANHOLES TO BE CLEANED AND TESTED FOR WATER TIGHTNESS ON COMPLETION OF CONSTRUCTION.
D 67	AFTER EVAPORATION WOULD HAVE BEEN APPROXIMATELY 640mm - OR 11655m ³ FOR THE PROPOSED SITE.		a	 MANHOLE COVERS AND FRAMES MANHOLE COVERS TO BE CLASS D400 IN HIGHWAYS, CLASS B125 IN FOOTWAYS AND VERGES, CLASS A15 IN NON-TRAFFICKED AREAS.
	ACCORING TO THE WATER BALANCE REPORT (Tier 3 SSA - Front field A303 Environpark IBAA storage facility - RA 1197 - issue		Ł	 MANHOLE COVER AND FRAME TO BE BEDDED AND SURROUNDED IN 1:3 MORTAR.
m	3.docx / Issue 3 / January 2021) THE MAXIMUM WATER STORAGE CAPACITY OF THE IBAA STOCKPILE (HIGH STOCK			
	4650m ³ - APPROXIMATELY 0.1m ³ WATER STORAGE PER m ³ OF IBAA.			
m	ACCOUNTING FOR IBAA VOLUME UNDER THE 61.5m CONTOUR			
	IBAA VOLUME (39013m ³) ABOVE THE 61.5m CONTOUR, WOULD HOLD APPROXIMATELY 3900m ³ OF WATER WITHIN			
L S	THE STOCKPILE, ACHIEVED BY SPRAY PUMPING. AS SUCH. ASSUMING THE SITE WOULD BE FULLY STOCKED			
	(ANTICIPATED DURING WINTER MONTHS), THE TOTAL WATER STORAGE CAPACITY OF THE SITE WOULD BE APPROXIMATELY		2	
	GENERATED BY THE LARGEST RAINFALL ON RECORD. (4/5)	В		
	AS PART OF THE PROPOSED WORKS, A BENTONITE LINER WOULD BE BUILT BENEATH THE LAGOON TO PROTECT AGAINST LEACHING OF HAZARDOUS CHEMICALS FROM THE	Ć	XX	
	IBAA TO THE GROUNDWATER.			LECEND
	LAGOON COULD BE DISPOSED OF TO A SPECIALIST TREATMENT FACILITY IF NECESSARY. THE PROPOSED SITE			STORAGE AREA BELOW 60.5m CONTOUR
	SHOULD AT ALL TIMES HAVE CAPACITY TO ACCOMMODATE A 1 IN 100 YEAR + 40% STORM EVENT, THEREFORE IT IS SUGGESTED TO USE THE 61m CONTOUR LEVEL AS THE		X	STORAGE AREA BELOW 61m CONTOUR
	CRITICAL WATER LEVEL TO START DISPOSING OF RUNOFF UNTIL THE WATER LEVEL RETURNS TO THE 60.5m CONTOUR.		9	STORAGE AREA BELOW 61.5m CONTOUR
	IBAA STOCKPILE SHOULD BE SPRAYED CONTINUOUSLY AND KEPT FULLY SATURATED TO INCREASE THE OVERALL			SWALE CATCHMENT
]	EVAPORATION RATE OF THE SITE.	<u> </u>		OVERLAND FLOW DIRECTION
	MEASURED EVAPORATION RATE AT A NEARBY IBAA STORAGE SITE WOULD BE 8.5 mm/day (EXCLUDING WINTER MONTHS).			
	THE ANTICIPATED IDAA STORAGE AREA IS 125600° (90% OF THE CONCRETE PAD AREA), ACHIEVING AN EVAPORATION RATE OF MORE THAN 100m ³ /day ON AVERAGE DURING EIGHT			
	MONTHS OF THE YEAR. AS SUCH, THE RISK OF WATER LEVELS FXCFFDING THE 61m			
	CONTOUR LEVEL AND TANKERING TO BE REQUIRED IS DEEMED LOW. (5/5)		A Re	A 02.02.2021 SD AMENDED TO SUIT CLIENT'S REQUIREMENTS REV DATE BY CKD APPDDESCRIPTION
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				a company of Royal HaskoningDHV
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				SURFACE WATER DRAINAGE SCHEME
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GENERA

THE PROPOSED DEVELOPMENT IS FOR THE CONSTRUCTION OF a.

THIS DRAWING IS NOT TO BE SCALED, WORK TO FIGURED DIMENSIONS ONLY,