DRAINAGE REPORT



Project: New Stone Barn, Ramsden

Made by:	Date:	Project No:	Checked by:	Approved by:	Pages	Rev
M Taylor	07/05/21	2132		MT	3	

Item	Details and Design Rationale	References and output
Method of surface water discharge	Surface water flows from new developments should be dealt with in the following order of preference: 1. Soakaways (infiltration drainage), 2. Discharge to a water course, 3. Discharge to a sewer.	Hierarchy
	In line with this protocol an infiltration test was carried out at the site to BRE365 on 7 th April 2021 to establish if soakaways would function. Infiltration rates where found to be good, indicating that infiltration will work well.	Infiltration Rate: 5.12x10 ⁻⁵ m/s
	The infiltration test pit was 1300mmx500mmx510mm deep to represent the infiltration blanket structure which is proposed.	
	The site previously served as a farm yard.	
	Currently all surface water infiltrates within the site. Surface water discharge from the site will be at greenfield rate.	
	The new roof area created by the proposed dwelling will discharge to a new infiltration blanket below the porous driveway, designed to accommodate the 100 year storm plus a 40% allowance for future climate change.	
	All surface water run-off will be dealt with within the site and the area will continue to discharge at greenfield rates.	

Calculations	Soakaway:			
	Soakaway Contributing	Impermeable area= 614m2		
	New Roof = Driveway =		100yr + 40% Utilisation Factor =0.44 <1 – OK	
	TOTAL = Soakaway Dimensions	614m2 s = 20.3m x 16.9m x 0.35m	Half Drain Time =0.46 hrs < 24 - OK	
	Storage Void Ratio = 3 (Granular sub-base m			
	Infiltration Rates = 5.12×10^{-5} m/s			
		ed by 50% to account for silting		
		infiltration rate calculation and soakaway to BRE Digest 365, are attached to this		
Groundwater clearance		Iso be designed to ensure a minimum of round is provided between the formation er level.		
	investigations record	led 300m east of the site for well ed no groundwater to a depth of 366 feet ed 1km to the west struck groundwater at		
	It is therefore conclu 100m below ground le	uded that ground water sits around 90 - evel at the site.		

Exceedance	In any surface water design scenario it is possible that the peak design storm could be exceeded or the system could fail through damage or blockage. The exceedance flow route indicates how water will behave in this event, and must ensure that it is controlled and dispersed safely, avoiding risk to property or persons.	
	The minimum deth of sub-base required for the structural integrity of the driveway is deeper that that required for the storage of the design storm (utilisation factor 0.44). This means that storms far in excess of the design event will be easily accommodated by the system. Any exceedance during very high order rainfall events will discharge safely along the driveway to the public highway.	
	Given the constraints of the site and adjacent land this is the only available safe exceedance path.	

Enclosures:

BRE 365 Design Spreadsheet

Borehole Record

Maintenance Schedule

Drainage Drawings – 2132/01, Drainage Layout 2132/02, Drainage Details

Flow Drainage Design

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New Slo	ne barn	-	
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Prepared By:	МТ	Date:	07/05/2021

SUMMARY OF CALCULATIONS				
critical design rainfall duration t_{crit} =	15	min		
required storage volume 'V _{req} ' =	13.46	m³		
provided storage volume 'V _{prov} ' =	30.88	m³		
utilisation factor =	0.44	.ОК		
required time to discharge 50% $'t_{50}' =$	0.46	hours		
utilisation factor =	0.02	.ОК		

SOAKAWAY DATA	
soakaway width 'W' [m] =	16.90
soakaway length 'L' [m] =	20.30
total depth from ground level ' D_{b} ' [m] =	0.30
depth to drain invert level 'D _d ' [<i>m</i>] =	0.00
soakaway effective depth 'D _{eff} ' [m] =	0.30
free volume in infill aggregate [%] =	30

SOAKAGE TRIAL PIT DATA	
soakage trial pit width ' W_t ' [m] =	0.50
soakage trial pit length 'L _t ' $[m] =$	1.30
total depth from ground level 'D _{tb} ' [m] =	0.51
depth to pipe invert level 'D _{tp} ' [m] =	0.28
soakage trial pit effective depth 'D _{teff} ' [m] =	0.23
free volume in infill aggregate [%] =	100
NOTE: faces of excavation assumed to	o be vertical

 ALTERNATIVE SOAKAWAY SIZES

 trench soakaways

 width of trench [mm]:
 450
 600
 900

 required trench length [m]:
 338.16
 267.90
 189.25

 tring soakaways
 tring soakaways
 tring soakaways

 diameter of ring [mm]:
 1050
 1350
 1500

 required pit diameter [m]:
 3.55
 3.41
 3.32

required trench length [m]:	338.16	267.90	189.25		
	ring soakaways				
diameter of ring [mm]:	1050	1350	1500		
required pit diameter [m]:	3.55	3.41	3.32		
* Based on effective depth and number of pits as in Soakaway Data table					
GENERAL DATA					
site leasting. England and Wales					

site location: England and Wales soakaway type: geocellular units

impermeable area drained to soakaway 'A' $[m^2] =$	614
60 min rainfall depth of 5 year return period 'R' [mm] =	20
M5-60 to M5-2d rainfall ratio 'r' =	0.50
allowance for climate change:	40%

a	allowance for infiltration through soakaway base:	50%
	available on-site infiltration test results: 🔘 Ye	s 🔘 No
	use soakage trial pit table below	
	internal surface area of trial pit a_{p50} [m ²] =	1.06
	storage volume between 75-25% $V_{p}' [m^{3}] =$	0.07
	time for water to fall from 75-25% t_p' [min] =	22.88
	soil infiltration rate 'f' [m/s] =	5.12E-05

	REQUIRED STORAGE CAPACITY PER RAINFALL DURATION												
rainfall		M5-D	M10-D			M50-D				M100-I	D	outflow from	required
duration <i>[min]</i>	rainfall factor Z1	rainfalls <i>[mm]</i>	Z2	rainfalls <i>[mm]</i>	inflow [m ³]	Z2 rainfalls inflow [mm] [m ³]		Z2	rainfalls <i>[mm]</i>	inflow [m ³]	soakaway [m³]	storage [m ³]	
5	0.39	7.80	1.21	13.18	8.09	1.61	17.59	10.80	1.86	20.28	12.45	2.81	9.65
10	0.54	10.80	1.22	18.49	11.36	1.66	25.07	15.39	1.92	29.07	17.85	5.61	12.24
15	0.65	13.00	1.23	22.42	13.77	1.68	30.58	18.77	1.96	35.64	21.88	8.42	13.46
30	0.82	16.40	1.24	28.47	17.48	1.71	39.22	24.08	2.00	45.95	28.21	16.83	11.38
60	1.00	20.00	1.24	34.72	21.32	1.73	48.44	29.74	2.03	56.84	34.90	33.67	1.23
120	1.19	23.80	1.24	41.32	25.37	1.72	57.39	35.24	2.01	67.13	41.22	67.33	0.00
240	1.38	27.60	1.23	47.51	29.17	1.71	66.06	40.56	1.99	76.86	47.19	134.66	0.00
360	1.51	30.20	1.22	51.56	31.66	1.70	71.83	44.10	1.97	83.22	51.10	201.99	0.00
600	1.68	33.60	1.21	56.88	34.92	1.68	78.95	48.48	1.94	91.31	56.07	336.66	0.00
1440	2.03	40.60	1.19	67.57	41.49	1.64	93.01	57.11	1.89	107.15	65.79	807.97	0.00

* Z2 is growth factor from M5 rainfalls

	SOAKAGE TRIAL PIT INFILTRATION TEST RESULTS																			
water	level measurement N°:	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19
Soakage	time [min] =	0	2	5	10	14	20													
Trial 1	depth to water [m] =	0.28	0.34	0.40	0.44	0.48	0.51													
Soakage	time [min] =	0	2	6	10	14	19	30												
Trial 2	depth to water [m] =	0.28	0.32	0.36	0.39	0.41	0.44	0.49												
Soakage	time [min] =	0	3	10	25	32	36													
Trial 3	depth to water [m] =	0.28	0.32	0.36	0.43	0.47	0.49													

Spreadsheet provided by: www.YourSpreadsheets.co.uk

calculations are based on BRE Guidelines (Digest 365)



Section: Soakaway

WELL RECORD		SHEET 1	· · · · · ·		. /	-
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WELL DESCRIPTION	British Geologic	al Survey			ish Geological Su	110)
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est level of water	^m . <u>above</u> * <u>0.D.</u> *	on when he			Date	
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est level of water	m. <u>abeve</u> <u>-0.p.</u> below well t		91	. m. deep.	Date	1976
ore	below well t	op when bo	re 			
sthod of drilling						

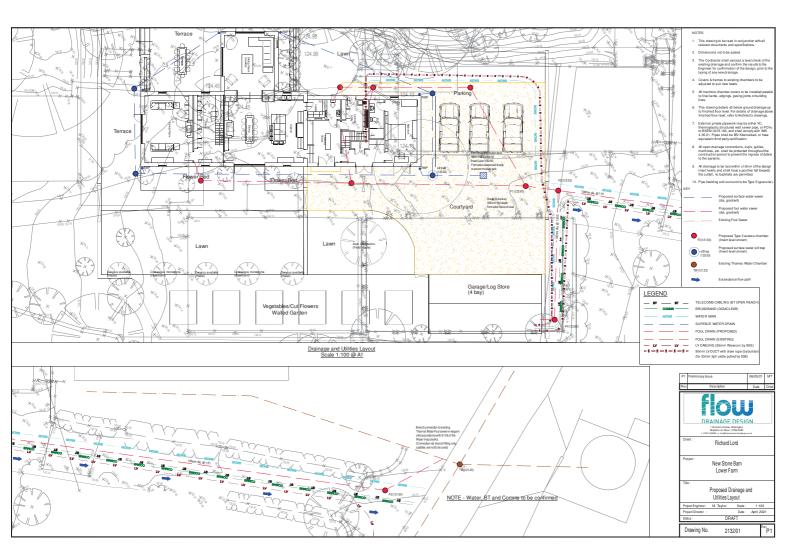
Surface Water Drainage System Maintenance Schedule

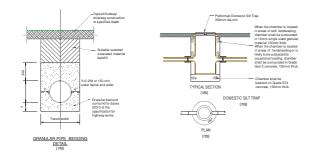


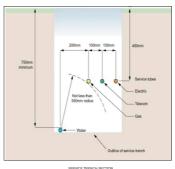
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	Frequency
CCTV camera survey, flush, descale, repair as necessary	5 years (or when evidence of failure is witnessed)
Remove silt and debris from sump Inspect chamber and repair as necessary	Annual (at the end of autumn to ensure leaf fall is dealt with as soon as possible)
Camera from silt trap to distribution tank to check integrity.	
Inspect ground over soakaway for signs of subsidence. Lift and repair/replace driveway sub-base as necessary.	5 years (or when evidence of failure is witnessed)
Inspect for areas of silt build up or subsidence within the pavement. Lift, clean and relay surface materials as necessary.	Annual
	Remove silt and debris from sump nspect chamber and repair as necessary Camera from silt trap to distribution tank to check integrity. nspect ground over soakaway for signs of subsidence. ift and repair/replace driveway sub-base as necessary.







SERVICE TRENCH SECTION

DRAINAGE NOTES

- This drawing to be read in conjunction with all relevant documents and specifications.
- 2. Dimensions not to be scaled.
- The Contractor shall carryout a level check of the existing drainage and confirm the results to the Engineer for confirmation of the design, prior to the laying of any new drainage.
- Covers & frames to existing chambers to be adjusted to suit new levels.
- All manhole chamber covers to be installed parallel to final kerbs, edgings, paving joints or building lines.
- This drawing details all below ground drainage up to finished floor level. For details of drainage above finished floor level, refer to Architect's drawings.
- External private pipework may be either VC, thermoplastic structured wall sever pipe, or PCVu to BSEN19476 160, and shall comply with WIS 4-35-01. Pipes shall be BSi Kitemarked, or have equivalent third party certification.
- All open drainage connections, svpls, guilles, manholes, etc. shall be protected throughout the construction period to prevent the ingress of debris to the systems.
- All drainage to be laid within ±10mm of the design invert levels and shall have a positive fall towards the cutfall, no baddalls are permitted.
- 10. Pipe bedding and surround to be Type S (granular).

