211053

Rhydolodog

Structural Calculations

at

Rhydoldog Estate Rhayader Powys LD6 5HB

Details prepared by:

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Project reference:

211053

Project title:

Structural Calculations

Design Code

Design for timber is based on:	Eurocode 5: Design of timber structures
Design for steel is based on:	Eurocode 3: Design of steel structures

Note:

The following calculations are for approximations of the actual cabin / tree arrangements. They were necessary in order to determine the impact of tree attachment bolts (TABs).

They demonstrate that a TAB has the ability to comfortably support a 50 - 60 kN load, which is sufficient for loads at the outer corners of the cabins.

Mid-span beam loads will be more than a TAB can support and it is proposed that these will be supported by a cut tree if one is conveniently located or by alternative arrangements.

A cut tree of the order of 300mm in diameter has the capacity to carry well in excess of 200 kN.

In the event that cut trees and TABs are not conveniently located there is still the option of constructing a support which will avoid excessive excavation using small diameter steel props fixed to the bedrock using a steel pin.

The design and construction of supports will be different for each cabin and final details will be worked out on site at each location, but we are confident that a structural solution can be found which minimises extensive excavation and disturbance to the natural habitats of animals and insect life.

Barratt Associates	Project	Rhydoldog Est	ate		Made by	Date	Job No
The Office. Berthlwyd. Llanarth. SA47 0PF	Client	Rhydolodog			DB	14-9-21	211053
Tel: 01545 580005					Checked	Revision	Page No
Email: barratt.associates@yahoo.co.uk	Description	Load breakdov	vn for structural	timber	DB	-	2
Loading							
Load breakdown forshallow pitched roof					Roof pitch =	20	0
Deed	Cadar ahinalaa	Characteristic	Factor	Design	Ich1/ma2		
Battens counter battens breatable memb	rane & sarking	0.20	1.35	0.27	KIN/III ⁻ kN/m ²		
Batteris, counter batteris, breatable memb	ers & insulation	0.30	1.35	0.27	kN/m ²		
Live	Snow / wind	0.75	1.5	1.13	kN/m²		
	w _{1,1} '=	1.45	w _{1,1} =	2.07	kN/m²		
Load on plan	w ₁ '=	1.54	w ₁ =	2.20	kN/m²		
l i har - balance for a to - a ba - it - ba - i a - f					Deefeitele -	70	•
Load breakdown for steeply pitched roof		Characteristic	Factor	Design	Roof pitch =	72	•
Dead	Cedar shingles	0.20	1.35	0.27	kN/m²		
Battens, counter battens, breatable memb	rane & sarking	0.2	1.35	0.27	kN/m²		
Rafte	ers & insulation	0.2	1.35	0.27	kN/m²		
Cei	ling & services	0.15	1.35	0.20	kN/m²		
Live	Snow	0.75	1.5	1.13	kN/m²		
	w _{2,1} '=	1.5	w _{2,1} =	2.14	kN/m²		
Load on plan	w ₂ '=	4.85	w ₂ =	6.92	kN/m²		
Load breakdown for internal timber floor							
		Characteristic	Factor	Design			
Dead	Flooring	0.15	1.35	0.20	kN/m²		
Ceilin	g and services	0.15	1.35	0.20	kN/m²		
250 x 75 flooring joists @600 c	rs & insulation	0.3	1.35	0.41	kN/m²		
Live	Domestic	1.5	1.5	2.25	kN/m ²		
Load on plan	w _{3,1} - w ₂ '=	2.1	w _{3,1} - w ₂ =	3.00	kN/m²		
	5	20	5	0.00			
Load breakdown for external balcony floor							
	-	Characteristic	Factor	Design			
Dead	Flooring	0.15	1.35	0.20	kN/m²		
Cellin 250 x 75 flooring i	g and services	0.15	1.35	0.20	KIN/M ²		
Live B	alcony loading	2.5	1.55	3 75	kN/m ²		
	w₂ ₁'=	3.02	W ₂ 1=	4 45	kN/m ²		
Load on plan	w ₃ '=	3.02	w ₃ =	4.45	kN/m²		
Load breakdown for stud wall		Characteristic	Factor	Design			
Dead	Cedar cladding		1 35	0.38	kN/m²		
bou	Sarking	0.175	1.35	0.24	kN/m²		
Stu	ds & insulation	0.24	1.35	0.32	kN/m²		
Live Plasterboared, s	kim & services	0.15	1.35	0.20	kN/m²		
	w _{4,1} '=	0.84	w _{4,1} =	1.14	kN/m²		
	Height of walls	4		4			
Load on plan	w ₄ '=	3.37	w ₄ =	4.54	kN/m		
Load breakdown for masonry					<u>^</u>		
	Density		γ =	2	0 kN/m ^³ -		
			t =	0.1	5 m 2 kN/m²		
Charac	Decign land		$w_5 = \gamma l =$	4.0	5 KN/111-		
	Design toad		w ₅ - 1.30W ₅ -	4.0			



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The Office. Berthlwyd. Llanarth. SA47 0PF Tel: 01545 580005 Email: barratt.associates@yahoo.co.uk		Client	Rhydolodog		DB	14-9-21	211053
			Loads on trees: Cut & Bolted		Checked	Revision	Page No
		Description			DB	-	4
Permissible timber stresses							
Assume 024							
Bending parallel to grain =		7.5	N/mm ²				
Tension parallel to grain =		4.5	N/mm ²				
Compression parallel to grain =		7.9	N/mm ²				
Compression perpendicular to grain	(min) =	1.9	N/mm ⁻				
Shear parallel to grain =		0.71	N/mm ⁻				
Modulous of elasticity (mean) =		10800	N/mm				
Assume trees with a minimum diam	eter of 300mm						
Loads bearing onto cut stump:							
Load =		105	kN				
Plan area =		70695	mm⁴				
Compressive stress =		1.49	N/mm [∠]				
Permissible =		7.9					
Ratio actual ÷ permissible =		0.19					
Factor of safety =		5.32					
Loads bearing onto tree attachment	bolt (TAB):			Face of tree			
Stem		Boss 75mm dia		Perch			
	30mm dia			30mm dia			
		•					
	150 mm	25 to 150	mm	minimum 150mm ma	ximum 300mm		_
							F
Load =		105	kN		52.5 kN		
Plan area =		11250	mm²	1	1250 mm ²		
Compressive stress =		9.33	N/mm [∠]		4.67 N/mm [∠]		

7.9

1.18

0.85

Fails

7.9

0.59

1.69

ΟК

Permissible =

Factor of safety =

Ratio actual ÷ permissible =