

FLOOD RISK AND SURFACE WATER DRAINAGE ASSESSMENT

OF

TREETOPS HOLIDAY PARK, WEEK ST MARY, CORNWALL, EX22 4UH

FOR

COUNTRYWIDE PARK HOMES LTD

09 February 2022

d Contract

Job No. 17310/ R1

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Report Status:	R1					
Report Reference:	17310					
	Engineer	Signature	Date			
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Reviewed by:	T. White		09.02.2022			
This report is issued to Countrywide Park Homes Ltd and does not confer or purport to confer on any Third Party, any benefit or any right pursuant to the Contracts (Rights of Third Parties) Act 1999.						



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	1.0 IN TRO DUC TIO N
1.1 Terms of Reference	Acting on instructions received from Clare Anscombe, on behalf of Countrywide
	Park Homes Ltd, John Grimes Partnership Ltd has carried out a Flood Risk and
	Surface Water Drainage Assessment of land at Treetops, Week St Mary, EX22 6UH.
1.2 Scope of Work	The site currently comprises a disused holiday chalet comple
	of Week St Mary. It is proposed to construct 22 new park homes for residential
	purposes with ancillary infrastructure.
	The purpose of this report is to:
	- identify potential sources of flooding,
	- Identify probability of hooding,
	- novide a surface water drainage assessment of the site
1.3 Third Party Pights and	This report is issued to Countrywide Park Homes Ltd and does not confer or purpo
Other limitations	to confer on any third party any benefit or any right pursuant to the Contracts
	(Rights of Third Parties) Act 1999.
	The assessment provided within this report is based partly on data acquired from
	the Environment Agency, where relevant, which in turn is supplied by third parties.
	Data has been interpreted in accordance with guidance notes and limitations
	provided by these third parties.
1.4 Limitations	1. This report has been produced in compliance with the
	between John Grimes Partnership Ltd. (JGP) and Countrywide Park Homes
	Ltd. 2. This was at here been an and far the here fit of Osmatrix ide Dark Here establish
	2. This report has been prepared for the benefit of Countrywide Park Homes Lid
	report shall not be relied upon for any other situation; neither shall it be
	transferred to any other party without the written agreement of IGP_IGP
	accepts no responsibility or liability for the use of this report for any purpose or
	any project except for that for which it was specifically prepared.
	3. The conclusions and advice provided in this report are based on:
	Current best practice and legislation [JGP accepts no responsibility or
	liability for any change in best practice advice or statute. In the event of
	additional information becoming available, improved p
	changes in legislation, amendment or re-interpretation of the assessment
	or report (in whole or in part) may be necessary];
	Sound engineering judgement and assessment of observations
	take interaceount the percentions of other involved and interacted
	narties
	4 Any information and data supplied by third parties has been interpreted in
	accordance with guidance notes and limitations provided by these third
	parties. Although this information has been reviewed and is considered
	relevant, no guarantee can be given to its accuracy and JGP can accept
	no responsibility for inaccuracies within the data supplied by other parties. In
	addition, interpretation of historic data should only be co
	indicative.
	5. JGP believes that providing information about limitations is essential to help
	the client identify and thereby manage risks.



	2.0 EX ISTIN G	SITE INFOR	RMATION		
2.1 Site Details					
2.1 Site Details Name Address (including postcode) NGR Development Type Local Planning Authority Lead Local Flood Authority Environment Agency Area 2.2 2.2 Location Plan Reproduced behalf of The Controller of Her Majesty's Stationery Office. © from Ordnance Survey map with the permission of Ordnance Survey® on Crown copyright. All rights reserved. Licence No.AL 100002364	2.0 EXISTING	STTE INFOR	Tota s of Scilly Po Po Fo Ki	Stewart	400 ha
2.3 Site Elevation (mAOD) and Grade (1:X m)	Highest Length Notes: General gra	pple odd <u>144.75</u> 147.60 de based on h	Lowest General (ighest and lo	Reeve House Grade west elevatio	<u>127.97</u> 1in9 ns
	between them.				
2.4 Type of Site	Greenfield 🗆 Bro	wnfield 🗆 Mix	ked 🛛 Agric	ultural 🗆	
2.5 Existing Surfacing	If not greenfield, indi Impermeable - Drain Engineered permeab Granular/gravel surfa Notes: The existing s chalet foundations) p The car park is access of the site. Bare ear noted across the wes covered with hedg boundaries.	cate the types of ed \Box ; Undraine ble surfaces, i.e. acing \boxtimes ite to be develop resent in the eas seed by a gravel th and evidence tern part of the si erows and occ.	f surfacing present ed ⊠ permeable pay ed comprises s t part of the sit track which pase e of recent veg ite. The remain asional matur	sent on the site ving several concret e along with a g sses through th getation / tree der of the site is re trees prese	e slabs (former gravel car park. le southern part clearance was s typically grass
2.6 Existing Drainage		Present	Private	Public	Unknown
Present on or Local to Site	Surface Water Foul Combined Land drainage Highways Soa kawa ys Attenuation Tanks Watercourse				
	Swales Ponds				\boxtimes



	Notes: The site is understood to have been a holiday static caravan site accessivia a gravel track. The existing house drains surface and foul water southward off the site. The eastern region of the site comprises several concrete slabs, where upstands of land drains, foul water and surface water runs can be located. The surface water network from here runs north-west to an outfall in the northern watercourse. The foul drainage continues to the north-west region into a suspected drainage field. The western region of the site comprises surface water drainage that outfalls north to the bottom of a bank, which also conveys spring water west watercourse. Foul networks are unknown in the western region.					
2.7 Surface Water Drainage	Surface water sewers \square ; Combined sewers \square ; Watercourse \square ; Sea \square ;					
Discharge from Existing Site	Unknown ⊠; No existing drainage □					
	Other 🛛 As exceedance flow to low ground					

	3.0 PROPOSED DEVELOPMENT
3.1 Description of Proposed Development	It is proposed to construct 22 new park homes for residential purposes with ancillar infrastructure.
3.2 Flood Risk Vulnerability Classification	According to Table 2: Flood Risk vulnerability classification in the PPG-FRCC the proposed development is classed as Highly Vulnerable .





4.2 Risk of Flooding From	Does the site currently discharge surface water to the sea? Yes \Box No \boxtimes
Coastal Sources	Does the site elevation place it at risk of coastal flooding? Yes
Is further analysis of coastal processes required to address flood risk at this site?	Yes □ No ⊠ Not at this time □ Further analysis is provided on wave setup □; overtopping □; wave run-up □; Other: □
 4.3 Risk of Pluvial Flooding (Rainfall/Surface Water) Based on flood risk information accessed on gov.uk on 22/08/2019. High AEP greater than 	Are there surface water flow routes on site? Yes □ No ⊠ If yes, flood risk is High □; Medium □; Low □ (tick all that apply) at High Risk peak depths are estimated at 0.0m and peak velocities at 0m/s. at Medium Risk peak depths are estimated 0.0m and peak velocities at 0.0m/s. at Low Risk peak depths are estimated at 0.0m and peak velocities at 0.0m/s.
3.3% Medium AEP between 1%-	Flood risk
Low AEP between 0.1%-1%	
Very Low AEP less than 0.1%	Image: Second secon
Does the proposed development have the potential to alter surface flow routes?	Yes \Box No \boxtimes If yes, describe how the development could alter surface water flow routes



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4.9 Site Flood Risk Summary	This table indicates if the proposed development is at risk of flooding or influences flooding elsewhere for the sources indicated.						
	On-site Flood Fu Risk to/from and development req			ther Iysis uired	Section	Comment/ Additional Analysis	
		Yes	No	Yes	No		Provided
	Fluvial		\boxtimes		\boxtimes	4.1	
	Tid a I/Sea		\boxtimes		\boxtimes	4.2	
	Pluvial		\boxtimes		\boxtimes	4.3	
	Sewers					4.4	
	Reservoirs		\boxtimes		\boxtimes	4.5	
	Canals				\boxtimes	4.6	
	Groundwater	\boxtimes		\boxtimes		4.7	Retain existing flow paths

	5.0 FLOOD RISK MITIGATION
5.1 Introduction	The purpose of this section is to provide guidance on mea mitigate the identified sources of flood risk to and from the proposed development. It is organised on the basis of sources of flooding.
5.2 Fluvial	Mitigation required? Yes □ No ⊠ If yes, provide further details below.
5.3 Tid a I/Se a	Mitigation required? Yes □ No ⊠ If yes, provide further details below.
5.4 Pluvial	Mitigation required? Yes □ No ⊠ If yes, provide further details below.
5.5 Sewer	Mitigation required? Yes ⊠ No □. If yes, provide further details below. See Section 7.0
5.6 Reservoirs	Mitigation required? Yes □ No ⊠ If yes, provide further details below.
5.7 Canals	Mitigation required? Yes □ No ⊠ If yes, provide further details below.
5.8 Groundwater	Mitigation required? Yes ⊠ No □ If yes, provide further details below. Retain existing flow paths
5.9 Residual Uncertainty and Freeboard	It is recommended that the finished floor level of the new buildings have a minimum freeboard of 0.15m above external finished ground levels to provide protection from minor flooding events / surface water ponding and exceedance events. Park homes are generally elevated above the founding pad and therefore finished floor levels are likely to be considerably more than 0.15m.
5.10 Access / Egress	N/A
5.11 Operational Measures	A maintenance scheme should be put in place to ensure the resilient measures installed remain effective. It is also important the drainage features are maintained in good working condition and free of blockages



6.0 PLANNING POLICY TE	STS							
6.1 Sequential Test Planning Policy Guidance on Flood Risk and Coastal Change (PPG-FRCC)	Since the site is in Flood Zone 1 ; according to guidance in the PPG-FRCC on the Sequential test, the proposed development is appropriate and does not need to pass the Sequential Test.							
6.2 Exception Test	Since the site is in Flood Zone 1 ; according to Table 3 of the Floor							
(Note: this is usually carried out by the Local Authority).	Vulnerability Classification in the PPG-FRCC, the proposed development is appropriate and does not need to pass the Exception Test							
	Exception Test Notes: In accordance with paragraph 160 of NF application of the Exception Test should be informed by a strategic or site-specific flood risk assessment, depending on whether it is being applied during plan production or at the application stage. For the Exception Test to be passed it should be demonstrated that:							
	a) the development would provide wider sustainability be community that outweigh flood risk; and							
	b) the development will be safe for its lifetime taking ac vulnerability of its users, without increasing flood risk elsewhere, and, where possible, will reduce flood risk overall.							
	If required, Section 5.0 of this Flood Risk Assessment addresses Part B of the Exception Test.							
	Is further analysis required for the Exception Test? Yes \Box No \boxtimes							
	7.0 DRAINAGE STRATEGY							
7.1 Rationale of Drainage Choice	Has a Phase II site investigation been carried out?Yes □No ⊠Have soakaway tests been carried out on the site?Yes ⊠No □							
	Soakaway tests to BRE 365 have been undertaken and the infiltration rates did not meet the required rates for soakaways to be an option. A tertiary river is located 2m west of the site, therefore it is proposed to drain a private hardstandings and roofs to the stream via attenuati controls.							



7.2 Summary of Surface Water	Separate systems will be provided for surface and foul water drainage.					
Drainage Strategy	The surface water drainage design has considered the recommended 40%					
	precautionary sensitivity from Table 5 in The Technical Guida National Planning Policy Framework for climate change.					
	Existing greenfield runoff rate: Greenfield Method	e: ReFH2 a 0.680 ba (batched areas in Appendix				
	SAAR BFIHost Region	0.880 1075 0.403 8	mm			
	1 in 2 years 1 in 100 years	5.5 18.6	l/s l/s	(control for morphology) (control for extreme events)		
	Discharge Rates Hydro-brakes at node S1.19: CTL-SHE-0107-4100-0200-410	0				
	Invert Level	132.00	00 m			
	Design Flow Design Depth	4.1	l/S m			
	CTL-SHE-0121-6000-0600-600	0.20				
	Invert Level	132.20	00 m			
	Design Flow Design Depth	6.0 0.6	I/S m			
	CTL-SHE-0067-1400-0200-140	0				
	Invert Level	136.95	55 m			
	Design Flow	1.4	l/s			
	CTI -SHF-0073-2000-0600-200	0.20	m			
	Invert Level	137.15	55 m			
	Design Flow	2.0	l/s			
	Design Depth	0.0	m			
	Initial analysis indicates that a required. This is proposed to b a bioretention rainwater garde to meet interception requirer structure will need to be confir	hat approximately 260 m ³ (240-350 m ³) of storage is to be primarily attenuation tanks complemented with arden (of dimensions $1.2x3.43x0.8$ m) for each roo uirements. The configuration and type of storage onfirmed as part of the final design.				
	The surface water infrastructuaccess to implement the drain	cture will be designed with silt traps and means c rainage maintenance schedule.				
	The surface water infrastructur is maintained in good condition of soakaway/storage should b Initial frequent inspections will will reduce to annual inspection storm events as required.	cture should be inspected regularly to ensure that i ition and free of blockages. The silt traps upstream d be inspected and cleared out at regular intervals will be required as the system settles. Typically, this actions with additional inspections following majo				
7.3 Exceedance	An exceedance flow plan is p	rovided	CONS	ideration of the exceedance flow		
	An exceedance flow plan is provided, consideration of the exceedance flow also indicates the arrangements for the redirection of existing flows around the buildings in the developed state. Finished floor levels should be at least 150mm above final external ground levels.					
7.4 Summary of Foul Water Drainage Strategy	Foul water is to fall to a pumpin Adopt a package pumping sta	ng station ation wit	on. h at le	east 13,200 litres capacity such as		
	(Appendix 2). The package pumping station rising main to a SWW connect	ing Sta discharg ion east	ges up t of the	o an 80mm HPPE (PE100) SDR 17 e site (see Appendix 3).		



8.0 CONCLUSIONS & RECOMMENDATIONS

- 1. The report describes the site and presents information on flood risk and surface w development.
- 2. The site is in Flood Zone 1 and the Flood Risk Assessment concludes that the site is not at risk from flooding from Fluvial, Tidal, Groundwater, Sewers, Pluvial, or Reservoirs, Canals, and other Artificial Sources.
- 3. The site passes the Sequential Test and does not need to pass the Exception Test before planning permission can be granted.
- 4. Discharge into the receiving watercourse may be subject to obtaining permission from Cornwall Council.
- 5. Exceedance pathways and maintenance regime are addressed.

APPENDICES

APPENDIX 1



APPENDIX 2

Ref: 17310; Date: 04th Febuary 2022; Site : Treetops Holiday Park; Client: Countrywide Park Homes Ltd; Author: RL; Checker: TW; Company: JGP							
Scenario 1	No. (P)	Flow (L)	BOD (g)	NH ₃ (g)	Comments		
Mobile home type caravans with full services	88	13200	5280	704	22 caravans 4 people each		
Total (per day) for Scenario 1	88	13200	5280	704			
Worst Case Totals (per day)	88	13200	5280	704			
Adopt suggested type of plant (or similar): Klargester Vertical Pumping Station Tank Size 2600 Diameter. Capacity (Ltrs) Up to 22,000.							
Surface water: all to be excluded from foul sewer.							
Consent to dischage: to be obtained from the Regulator.							
Waste Disposal Units: assumed that none are fitted.							

Flows and Loads taken from British Water Flows and Loads 4

Scenario 1: 22 caravans

APPENDIX 3

