

## Connecting Strength

## K2 Base Report

shed 4 DR Collin

Project address Company Author Issue date & version 39 Coldingham Rd, Eyemouth TD14 5AR, UK Maden Eco Conor Maden 27/11/2023 | K2 Base Version 3.1.106.0





### K2 Systems. Innovative mounting system from a strong team.

Since 2004 we have been developing pioneering and highly functional mounting system solutions for photovoltaic installations around the world. Our systems are designed in our own product development department where we continually optimize and adapt mounting systems to the ever-changing market.

#### A knowledgeable and friendly team

Just like a mountain climbing team, K2 Systems is built on mutual trust. This applies to our customer service as well as within the company itself, because we believe a trusting partnership leads to successful photovoltaic projects.

Our employees place total focus on the needs and wishes of our customer. This is true in all company departments.

#### Quality management and certificates

#### 10 locations and worldwide sales network

In our international team, everyone works together to provide customers with competent, comprehensive and entirely personalized service.

This is especially true in the constant training our employees undergo with regards to product optimization, quality assurance, or innovations in construction techniques.

K2 Systems stands for Connecting Strength, the highest quality, and precision-crafted and customized components. Our customers and business partners deeply appreciate all of these factors. Three independent authorities have tested, confirmed, and certified our skills and components. External authorities are not the only ones to have put K2 Systems to the test. Our internal quality control ensures that all our products are subject to a constant review process.

These measures all ensure the outstanding quality standards that exemplify products from K2 Systems, and which we maintain through largely exclusive "Made in Germany" or "Made in Europe" practices.



#### Product guarantee

K2 Systems offers a 12-year product warranty on all products in its integrated range. The use of high quality materials and a three-level quality inspection ensure these standards.

#### In a nutshell

As roof-top specialists, we offer effective and economical solutions for roofs all around the world and provide professional, fast and reliable support for our customers in the solar industry.



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### Project information

Name	shed 4 DR Collin
Address	39 Coldingham Rd, Eyemouth TD14 5AR, UK
Ground elevation	35.92 m
Author	Conor Maden

### Load settings

Design method	BS EN
Failure consequence class (CC)	CC1
Design working life	25 years
Terrain category	Sea
Environment	Normal area
Basic wind speed	24.5 m/s
Snow load zone	3
Snow load on ground level	0.50 kN/m <sup>2</sup>

### Roofs

Roof	System	Module	Power	Quantity	Total power
Roof 1	<u>K2 BasicRail</u>	TSM-440NEG9R.28 (Vertex S+)	440 Wp	124	54.56 kWp
Roof 2	<u>K2 BasicRail</u>	TSM-440NEG9R.28 (Vertex S+)	440 Wp	124	54.56 kWp

#### Total

#### 248 109.12 kWp



#### THE PROJECT IS VERIFIED.

The selected mounting system can be installed as planned Thank you for choosing a K2 mounting system.



## Roofs



## Project information

Name Address Ground elevation Author shed 4 DR Collin 39 Coldingham Rd, Eyemouth TD14 5AR, UK 35.92 m Conor Maden 1 ì





Roof	System	Module	Power	Quantity	Total power
Roof 1	<u>K2 BasicRail</u>	TSM-440NEG9R.28 (Vertex S+)	440 Wp	124	54.56 kWp

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# Connecting StrengthRoofs | Roof 1 | Assembly plan

### Base Rails

	Whole	e Rails		Rail cutting			
Туре	Total Rail Length	Quantity 4.40 m	Part of Rail	Length	Rest		
А	8.178	1	4.400	3.778	0.612		
В	9.332	2	4.400	0.700	3.690		
С	9.332	2	3.690	0.700	2.980		
D	9.332	2	2.980	0.700	2.270		
E	9.332	2	2.270	0.700	1.560		
F	9.332	2	1.560	0.700	0.850		
G	9.332	2	0.850	0.700	0.140		

## Module arrays

Module array	Width[m]	Length[m]	Width in modules	Length in modules
1	36.29	7.08	31	4

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## Connecting StrengthRoofs | Roof 1 | Module array 1



#### Roof (1) Module array (1)

Mounting System Module

Row spacing

#### <u>K2 BasicRail</u>

124(54.56 kWp) x TSM-440NEG9R.28 (Vertex S+) 1.77 m















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## I Connecting StrengthResults | Roof 1

Roof	System	Module	Power	Quantity	Total power
Roof 1	<u>K2 BasicRail</u>	TSM-440NEG9R.28 (Vertex S+)	440 Wp	124	54.56 kWp

### Module

Name	TSM-440NEG9R.28 (Vertex S+)
Manufacturer	Trina Solar Energy
Output power	440 Wp
Dimensions	1,762×1,134×30 mm
Weight	21.1 kg

### Components

Fastener	K2 BasicClip
Base rails	K2 BasicRail 22
Metal screw	Thread-forming metal screw 6.0×38

## Loads on modules (module dimensioning)

Array	Λ_TrΛ_	ultimate state [Pa]					Serviceability [Pa]			
Анау	[m <sup>2</sup> ]	Pressure ⊥	Pressure II	Uplift ⊥	Uplift II	Pr	ressure ⊥	Pressure II	Uplift ⊥	Uplift II
field area	2.00	713.1	153.3	-597.2	28.7		564.3	121.6	-445.3	28.7
ridge	2.00	713.1	153.3	-1,418.9	28.7		564.3	121.6	-1,089.8	28.7
gableboard	2.00	713.1	153.3	-1,653.7	28.7		564.3	121.6	-1,273.9	28.7
corner region (eave)	2.00	713.1	153.3	-1,771.1	28.7		564.3	121.6	-1,366.0	28.7
eaves	2.00	713.1	153.3	-832.0	28.7		564.3	121.6	-629.4	28.7

#### Utilisation result

		ultima	ultimate limit state		ultimate limit state Usab. Distances ma		Distances		maxim	maximum values	
No.	roof areas	Pr	CL	Fst	Pr	Fst	BR	CL	Fst		
Module Array		σ[%]	σ[%]	F[%]	f[%]	[m]	[m]	L <sub>max</sub> [m]	Fst D <sub>max</sub> [m]		
1	field area	22.4	6.7	47.6	32.6	0.800		0.466	0.850		
1	ridge	23.1	0.0	82.4	19.9	0.600		0.411	0.728		
1	gableboard	26.9	8.3	95.8	23.2	0.600		0.394	0.627		
1	corner region (eave)	12.8	8.9	68.3	4.9	0.400		0.400	0.586		
1	eaves	24.3	0.0	65.4	36.3	0.800		0.452	0.850		



## Results | Roof 1

Pr	Profile	$Fst \ D_{max}$	maximum fastener spacing
Fst	Fastener	BR	Base Rail
σ	Stress	Usab.	serviceability limit state
f	Deflection	CL	Cantilever
F	Force		

 $\text{CL/L}_{\text{max}}$  maximum cantilever length

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### Notes

- The quantity for K2 BasicRail BasicClips is calculated in such a way, that according to the Assembly Instructions a BasicClip can be installed on the crest to the left and right of a Rail Connector Set.
- The structural design complies with BS EN 1990 Basis of Structural Design.
- Snow loads are determined in accordance with National Annex BS NA EN 1991-1-3 (2018) UK National Annex to EC1 Action on structures general actions snow loads.
- Wind loads are determined in accordance with National Annex BS NA EN 1991-1-4 UK National Annex to EC1 Action on structures, general actions wind actions.
- Service life is recognised according to 'Eurocode EN 1991 Action on structures, Snow loads' and 'Eurocode EN 1991 Actions on structures, Wind actions'. Subject to the Building Regulations and for security-relevant reasons the installation has to be dismantled at the end of its service life.
- Failure consequence class is considered according to 'Eurocode EN 1990 Basis of structural design'.
- Data and results must be verified with regard to local conditions and checked by a suitably qualified person. Please see our TCU under https://k2-systems.com/en/base-tcu, in particular § 2 ("technical and specialist requirements for the customer"), § 7 ("warranty provisions") and § 8 ("limitation of liability").

## Connecting Strength Structural analysis report | Roof 1

### General information

Name	shed 4 DR Collin
Mounting System	K2 BasicRail
Author	Conor Maden

## Location information

Address	39 Coldingham Rd, Eyemouth TD14 5AR, UK
Ground elevation	35.92 m

## Roof information

Building height	6.00 m
Roof type	Gable roof
Roof pitch	15°
Fastening method	Roof cover
Roof covering	Trapezoidal
Min. roof edge distance	0.00 m
Crest distance	200.0 mm
Crest width	22.0 mm
Crest height	40.0 mm
Roof material	Steel
Sheet quality	S235
Sheet thickness	0.500 mm

## Loads

Design method	BS EN
Failure consequence class (CC)	CC1
Design working life	25 years
Terrain category	Sea

#### Wind load

Velocity pressure	<b>q</b> <sub>p,50</sub>	= 1.000 kN/m <sup>2</sup>
Adjustment factor for service life	$f_w$	= 1.000
Velocity pressure	<b>q</b> <sub>p,25</sub>	= 0.921 kN/m <sup>2</sup>

# I Connecting StrengthStructural analysis report | Roof 1

#### Roof areas

Array	load impact area [m²]	maxCpe <sub>10</sub>	minCpe <sub>10</sub>	wind pressure [kN/m²]	wind suction [kN/m²]
field area	10.00	0.200	-0.600	0.184	-0.552
ridge	10.00	0.200	-1.300	0.184	-1.197
gableboard	10.00	0.200	-1.500	0.184	-1.381
corner region (eave)	10.00	0.200	-1.600	0.184	-1.473
eaves	10.00	0.200	-0.800	0.184	-0.737

#### Snow load

Snow load zone	3	
Environment	Norn	nal terrain
Snow guard	No	
Snow load on ground level	S <sub>k</sub>	= 0.500 kN/m <sup>2</sup>
Shape Coefficient for Snow	$\mu_{i}$	= 0.800
Factor for roof pitch	$\mathbf{d}_{i}$	= 0.966
Snow load on roof	<b>S</b> <sub>i,50</sub>	= 0.386 kN/m <sup>2</sup>
Adjustment factor for service life	$\mathbf{f}_{s}$	= 1.000
Snow load on roof	<b>S</b> <sub>i,25</sub>	= 0.359 kN/m <sup>2</sup>

#### Dead Load

Weight of module	$\mathbf{G}_{M}$	= 21.1 kg
Weight of mounting system per module		= 1.5 kg
Module area	$A_{M}$	= 2.00 m <sup>2</sup>
Dead weight of module per m <sup>2</sup>		= 10.56 kg/m <sup>2</sup>
Dead weight of mounting system per m <sup>2</sup>		= 0.75 kg/m²
Total Dead Load (excl. ballast) per m²		= 0.11 kN/m <sup>2</sup>

# I Connecting StrengthStructural analysis report | Roof 1

## Load Combinations

#### Ultimate limit state

Partial safety factor unfavourable permanent load	$\gamma_{G, sup}$	= 1.35
Partial safety factor favourable permanent load	$\gamma_{G,inf}$	= 1.00
Partial safety factor destabilising permanent load	$\gamma_{\rm G,dst}$	= 1.10
Partial safety factor stabilising permanent load	$\gamma_{\text{G,stb}}$	= 0.90
Partial safety factor first variable load	γ <sub>Q</sub>	= 1.50
Partial safety factor variable loads	$\gamma_{\text{Q}}$	= 1.50
Combination coefficient with regards to wind	$\psi_{o,w}$	= 0.60
Combination coefficient with regards to wind (additional varying influences)	Ψ <sub>1,w</sub>	= 0.20
Combination coefficient with regards to Snow	$\psi_{\text{o},\text{s}}$	= 0.50
Importance factor permanent	$\mathbf{K}_{Fl,G}$	= 0.90
Importance factor variable	$\mathbf{K}_{Fl,Q}$	= 0.85
Characteristic dead weight	G <sub>k</sub>	
Characteristic snow load on the roof	S <sub>i,n</sub>	
Characteristic wind load	$\mathbf{W}_{k}$	
Load case combination 01 $E_d = \gamma_{G,sup} * \kappa_{FLG} * G_k + \gamma_0 * \kappa_{FLO} * S_{i,n}$		

Load case combination of	$\mathbf{E}_{d} = \mathbf{\gamma}_{G, sup} \cdot \mathbf{K}_{Fl,G} \cdot \mathbf{G}_{k} + \mathbf{\gamma}_{Q} \cdot \mathbf{K}_{Fl,Q} \cdot \mathbf{S}_{i,n}$
Load case combination 02	$E_{d} = \gamma_{G,sup} * \kappa_{Fl,G} * G_{k} + \gamma_{Q} * \kappa_{Fl,Q} * W_{k,Pressure}$
Load case combination 03	$E_{d} = \gamma_{G,sup} \ast \kappa_{Fl,G} \ast G_{k} + \gamma_{Q} \ast \kappa_{Fl,Q} \ast (W_{k,Pressure} + \psi_{Q,S} \ast S_{i,Pressure})$
Load case combination 04	$E_{d} = \gamma_{G,sup} \ast \kappa_{Fl,G} \ast G_{k} + \gamma_{Q} \ast \kappa_{Fl,Q} \ast (S_{i,n} + \psi_{0,W} \ast W_{k,Pressure})$
Load case combination 06	$E_{d} = \gamma_{G,inf} * G_{k} + \gamma_{Q} * \kappa_{Fl,Q} * W_{k,Uplift}$

#### Serviceability limit state

Combination coefficient with regards to wind	ψ <sub>ο,</sub> w	= 0.60
Combination coefficient with regards to Snow	$\psi_{\text{o},\text{s}}$	= 0.50

Load case combination 01	$E_d = G_k + S_{i,n}$
Load case combination 02	$E_d = G_k + W_{k,Pressure}$
Load case combination 03	$E_d = G_k + W_{k,Pressure} + \psi_{0,S} * S_{i,n}$
Load case combination 04	$\mathbf{E}_{d} = \mathbf{G}_{k} + \mathbf{S}_{i,n} + \mathbf{\psi}_{0,W} * \mathbf{W}_{k,Pressure}$
Load case combination 06	$E_d = G_k + W_{k,Uplift}$

## Structural analysis report | Roof 1

## Maximum load on modules (Mounting system dimensioning)

Array	۸_Tr۸	ultimate state [kN/m²]			Se	Serviceability [kN/m²]			
	[m <sup>2</sup> ]	Pressure ⊥	Pressure II	Uplift ⊥	Uplift II	Pressure ⊥	Pressure II	Uplift ⊥	Uplift II
field area	10.00	0.713	0.153	-0.597	0.029	0.564	0.122	-0.445	0.029
ridge	10.00	0.713	0.153	-1.419	0.029	0.564	0.122	-1.090	0.029
gableboard	10.00	0.713	0.153	-1.654	0.029	0.564	0.122	-1.274	0.029
corner region (eave)	10.00	0.713	0.153	-1.771	0.029	0.564	0.122	-1.366	0.029
eaves	10.00	0.713	0.153	-0.832	0.029	0.564	0.122	-0.629	0.029

### Max. load on fastener

Arrow	∧_Tr∧ _	ultimate state [kN]				Serviceability [kN]			
Анау	A-11A - [m <sup>2</sup> ]	Pressure ⊥	Pressure II	Uplift ⊥	Uplift II	Pressure ⊥	Pressure II	Uplift ⊥	Uplift II
field area	10.00	0.553	0.119	-0.463	0.022	0.438	0.094	-0.345	0.022
ridge	10.00	0.415	0.089	-0.825	0.017	0.328	0.071	-0.634	0.017
gableboard	10.00	0.415	0.089	-0.962	0.017	0.328	0.071	-0.741	0.017
corner region (eave)	10.00	0.276	0.059	-0.687	0.011	0.219	0.047	-0.530	0.011
eaves	10.00	0.553	0.119	-0.645	0.022	0.438	0.094	-0.488	0.022

## Resistance Values of Components

#### Base Rails

Base Rails	А	$I_y$	I <sub>z</sub>	$W_y$	$W_z$
	[cm <sup>2</sup> ]	[cm^4]	[cm^4]	[cm³]	[cm <sup>3</sup> ]
K2 BasicRail 22	2.380	1.52	7.74	1.08	2.46

#### Fastener

Fastener	$R_{D,Uplift,Perpendicular}$ [kN]	$R_{D,Pressure,Perpendicular}$ [kN]	$R_{D,Pressure,Parallel}$ [kN]
K2 BasicClip	1.02	-	0.96
Thread-forming metal screw 6.0×38	0.65	-	0.62

## I Connecting Strength Structural analysis report | Roof 1

#### Utilisation result

		ultima	te limit	state	Usab.	Distan	ces	maxim	um values
No.	roof areas	Pr	CL	Fst	Pr	Fst	BR	CL	Fst
Module Array		σ[%]	σ[%]	F[%]	f[%]	[m]	[m]	L <sub>max</sub> [m]	Fst D <sub>max</sub> [m]
1	field area	22.4	6.7	47.6	32.6	0.800		0.466	0.850
1	ridge	23.1	0.0	82.4	19.9	0.600		0.411	0.728
1	gableboard	26.9	8.3	95.8	23.2	0.600		0.394	0.627
1	corner region (eave)	12.8	8.9	68.3	4.9	0.400		0.400	0.586
1	eaves	24.3	0.0	65.4	36.3	0.800		0.452	0.850

Pr	Profile			
Fst	Fastener			

- σ Stress
- f Deflection
- F Force
- $\text{CL}/\text{L}_{\text{max}}$  maximum cantilever length

 $\mathsf{Fst} \; \mathsf{D}_{\mathsf{max}} \quad \textbf{maximum fastener spacing}$ 

BR Base Rail

- Usab. serviceability limit state
- CL Cantilever

## Connecting StrengthRoofs | Roof 1 | Bill of material

Position	ltem no.	Item description	Quantity	Weight
1	1001164	K2 BasicClip	467	14.0 kg
2	1005193	Thread-forming metal screw 6.0×38	934	6.5 kg
3	2003072	OneMid Black Set 30-42	216	17.1 kg
4	2002589	OneEnd Black Set 30-42	64	5.6 kg
5	2003240	K2 BasicRail 22; 4.40m	68	192.4 kg
6	1003571	K2 BasicRail BasicConnector Set	56	2.7 kg
7	1003558	K2 BasicRail BasicLock 22 Set	32	1.6 kg
Total				239.9 kg

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37.85	m

Roof	System	Module	Power	Quantity	Total power
Roof 2	<u>K2 BasicRail</u>	TSM-440NEG9R.28 (Vertex S+)	440 Wp	124	54.56 kWp

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## Module arrays

Module array	Width[m]	Length[m]	Width in modules	Length in modules
1	37.45	7.08	31	4

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#### Roof (2) Module array (1)

Mounting System Module

Row spacing

#### K2 BasicRail

124(54.56 kWp) x TSM-440NEG9R.28 (Vertex S+) 1.77 m



















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## Connecting StrengthResults | Roof 2

Roof	System	Module	Power	Quantity	Total power
Roof 2	<u>K2 BasicRail</u>	TSM-440NEG9R.28 (Vertex S+)	440 Wp	124	54.56 kWp

### Module

Name	TSM-440NEG9R.28 (Vertex S+)
Manufacturer	Trina Solar Energy
Output power	440 Wp
Dimensions	1,762×1,134×30 mm
Weight	21.1 kg

### Components

Fastener	K2 BasicClip
Base rails	K2 BasicRail 22
Metal screw	Thread-forming metal screw 6.0×38

## Loads on modules (module dimensioning)

Arrow	Λ_TrΛ	ultimate state [Pa]					Serviceability [Pa]			
Array	[m <sup>2</sup> ]	Pressure ⊥	Pressure II	Uplift ⊥	Uplift II	Pressu ⊥	re Pressure II	Uplift ⊥	Uplift II	
field area	2.00	713.1	153.3	-597.2	28.7	564	.3 121.6	-445.3	28.7	
ridge	2.00	713.1	153.3	-1,418.9	28.7	564	.3 121.6	-1,089.8	28.7	
gableboard	2.00	713.1	153.3	-1,653.7	28.7	564	.3 121.6	-1,273.9	28.7	
corner region (eave)	2.00	713.1	153.3	-1,771.1	28.7	564	.3 121.6	-1,366.0	28.7	
eaves	2.00	713.1	153.3	-832.0	28.7	564	.3 121.6	-629.4	28.7	

#### Utilisation result

		ultima	te limit	state	Usab.	Distan	ces	maxim	um values
No.	roof areas	Pr	CL	Fst	Pr	Fst	BR	CL	Fst
Module Array		σ[%]	σ[%]	F[%]	f[%]	[m]	[m]	L <sub>max</sub> [m]	Fst D <sub>max</sub> [m]
1	field area	22.4	1.9	47.6	32.6	0.800		0.466	0.850
1	ridge	23.1	3.4	82.4	19.9	0.600		0.411	0.728
1	gableboard	26.9	4.6	95.8	23.2	0.600		0.394	0.627
1	corner region (eave)	12.8	4.9	68.3	4.9	0.400		0.400	0.586
1	eaves	24.3	1.8	65.4	36.3	0.800		0.452	0.850



## Results | Roof 2

Pr	Profile	$Fst \; D_{max}$	maximum fastener spacing
Fst	Fastener	BR	Base Rail
σ	Stress	Usab.	serviceability limit state
f	Deflection	CL	Cantilever
F	Force		

 $\text{CL}/\text{L}_{\text{max}}$  maximum cantilever length

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### Notes

- The quantity for K2 BasicRail BasicClips is calculated in such a way, that according to the Assembly Instructions a BasicClip can be installed on the crest to the left and right of a Rail Connector Set.
- The structural design complies with BS EN 1990 Basis of Structural Design.
- Snow loads are determined in accordance with National Annex BS NA EN 1991-1-3 (2018) UK National Annex to EC1 Action on structures general actions snow loads.
- Wind loads are determined in accordance with National Annex BS NA EN 1991-1-4 UK National Annex to EC1 Action on structures, general actions wind actions.
- Service life is recognised according to 'Eurocode EN 1991 Action on structures, Snow loads' and 'Eurocode EN 1991 Actions on structures, Wind actions'. Subject to the Building Regulations and for security-relevant reasons the installation has to be dismantled at the end of its service life.
- Failure consequence class is considered according to 'Eurocode EN 1990 Basis of structural design'.
- Data and results must be verified with regard to local conditions and checked by a suitably qualified person. Please see our TCU under https://k2-systems.com/en/base-tcu, in particular § 2 ("technical and specialist requirements for the customer"), § 7 ("warranty provisions") and § 8 ("limitation of liability").

## I Connecting Strength Structural analysis report | Roof 2

## General information

Name	shed 4 DR Collin
Mounting System	K2 BasicRail
Author	Conor Maden

## Location information

Address	39 Coldingham Rd, Eyemouth TD14 5AR, UK
Ground elevation	35.92 m

## Roof information

Building height	6.00 m
Roof type	Gable roof
Roof pitch	15°
Fastening method	Roof cover
Roof covering	Trapezoidal
Min. roof edge distance	0.00 m
Crest distance	200.0 mm
Crest width	22.0 mm
Crest height	40.0 mm
Roof material	Steel
Sheet quality	S235
Sheet thickness	0.500 mm

## Loads

Design method	BS EN
Failure consequence class (CC)	CC1
Design working life	25 years
Terrain category	Sea

#### Wind load

Velocity pressure	<b>q</b> <sub>p,50</sub>	= 1.000 kN/m <sup>2</sup>
Adjustment factor for service life	$f_w$	= 0.921
Velocity pressure	<b>q</b> <sub>p,25</sub>	= 0.921 kN/m <sup>2</sup>

# Connecting StrengthStructural analysis report | Roof 2

#### Roof areas

Array	load impact area [m²]	maxCpe <sub>10</sub>	minCpe <sub>10</sub>	wind pressure [kN/m²]	wind suction [kN/m²]
field area	10.00	0.200	-0.600	0.184	-0.552
ridge	10.00	0.200	-1.300	0.184	-1.197
gableboard	10.00	0.200	-1.500	0.184	-1.381
corner region (eave)	10.00	0.200	-1.600	0.184	-1.473
eaves	10.00	0.200	-0.800	0.184	-0.737

#### Snow load

Snow load zone	3	
Environment	Norn	nal terrain
Snow guard	No	
Snow load on ground level	S <sub>k</sub>	= 0.500 kN/m <sup>2</sup>
Shape Coefficient for Snow	$\mu_{i}$	= 0.800
Factor for roof pitch	$\mathbf{d}_{i}$	= 0.966
Snow load on roof	<b>S</b> <sub>i,50</sub>	= 0.386 kN/m <sup>2</sup>
Adjustment factor for service life	$f_s$	= 0.929
Snow load on roof	<b>S</b> <sub>i,25</sub>	= 0.359 kN/m <sup>2</sup>

#### Dead Load

Weight of module	$\mathbf{G}_{M}$	= 21.1 kg
Weight of mounting system per module		= 1.5 kg
Module area	$\mathbf{A}_{M}$	= 2.00 m <sup>2</sup>
Dead weight of module per m <sup>2</sup>		= 10.56 kg/m <sup>2</sup>
Dead weight of mounting system per $m^2$		= 0.75 kg/m²
Total Dead Load (excl. ballast) per m²		= 0.11 kN/m <sup>2</sup>

# Connecting StrengthStructural analysis report | Roof 2

### Load Combinations

#### Ultimate limit state

Partial safety factor unfavourable permanent load	$\gamma_{G, sup}$	= 1.35
Partial safety factor favourable permanent load	$\gamma_{\text{G,inf}}$	= 1.00
Partial safety factor destabilising permanent load	$\gamma_{\text{G,dst}}$	= 1.10
Partial safety factor stabilising permanent load	$\gamma_{\text{G,stb}}$	= 0.90
Partial safety factor first variable load	γ <sub>Q</sub>	= 1.50
Partial safety factor variable loads	$\gamma_{\text{Q}}$	= 1.50
Combination coefficient with regards to wind	$\psi_{o,w}$	= 0.60
Combination coefficient with regards to wind (additional varying influences)	Ψ1,w	= 0.20
Combination coefficient with regards to Snow	$\psi_{\text{o},\text{s}}$	= 0.50
Importance factor permanent	$\mathbf{K}_{Fl,G}$	= 0.90
Importance factor variable	κ <sub>fl,Q</sub>	= 0.85
Characteristic dead weight	G <sub>k</sub>	
Characteristic snow load on the roof	S <sub>i,n</sub>	
Characteristic wind load	$\mathbf{W}_{\mathbf{k}}$	
Load case combination 01 $\mathbf{E}_{d} = \mathbf{\gamma}_{gsun} * \mathbf{\kappa}_{Flg} * \mathbf{G}_{k} + \mathbf{\gamma}_{0} * \mathbf{\kappa}_{Flg} * \mathbf{S}_{in}$		

Load case combination of	$\mathbf{E}_{d} = \mathbf{\gamma}_{G, sup} \cdot \mathbf{\kappa}_{Fl, G} \cdot \mathbf{G}_{k} + \mathbf{\gamma}_{Q} \cdot \mathbf{\kappa}_{Fl, Q} \cdot \mathbf{S}_{i, n}$
Load case combination 02	$E_{d} = \gamma_{G,sup} \ast \kappa_{Fl,G} \ast G_{k} + \gamma_{Q} \ast \kappa_{Fl,Q} \ast W_{k,Pressure}$
Load case combination 03	$E_{d} = \gamma_{G,sup} \ast \kappa_{Fl,G} \ast G_{k} + \gamma_{Q} \ast \kappa_{Fl,Q} \ast (W_{k,Pressure} + \psi_{O,S} \ast S_{i,n})$
Load case combination 04	$E_{d} = \gamma_{G,sup} * \kappa_{Fl,G} * G_{k} + \gamma_{Q} * \kappa_{Fl,Q} * (S_{i,n} + \psi_{0,W} * W_{k,Pressure})$
Load case combination 06	$E_{d} = \gamma_{G,inf} * G_{k} + \gamma_{Q} * \kappa_{FI,Q} * W_{k,Uplift}$

#### Serviceability limit state

Combination coefficient with regards to wind	ψ <sub>ο,</sub> w	= 0.60
Combination coefficient with regards to Snow	Ψ <sub>o,s</sub>	= 0.50

Load case combination 01	$E_d = G_k + S_{i,n}$
Load case combination 02	$E_d = G_k + W_{k,Pressure}$
Load case combination 03	$E_d = G_k + W_{k,Pressure} + \psi_{0,S} * S_{i,n}$
Load case combination 04	$E_{d} = G_{k} + S_{i,n} + \psi_{0,W} * W_{k,Pressure}$
Load case combination 06	$E_d = G_k + W_{k,Uplift}$

## Structural analysis report | Roof 2

## Maximum load on modules (Mounting system dimensioning)

Arrow	۸_Tr۸	ultimate state [kN/m²]				Se	Serviceability [kN/m²]			
Анау	[m <sup>2</sup> ]	Pressure ⊥	Pressure II	Uplift ⊥	Uplift II	Pressure ⊥	Pressure II	Uplift ⊥	Uplift II	
field area	10.00	0.713	0.153	-0.597	0.029	0.564	0.122	-0.445	0.029	
ridge	10.00	0.713	0.153	-1.419	0.029	0.564	0.122	-1.090	0.029	
gableboard	10.00	0.713	0.153	-1.654	0.029	0.564	0.122	-1.274	0.029	
corner region (eave)	10.00	0.713	0.153	-1.771	0.029	0.564	0.122	-1.366	0.029	
eaves	10.00	0.713	0.153	-0.832	0.029	0.564	0.122	-0.629	0.029	

### Max. load on fastener

Arrow	∧_Tr∧ .	ultimate state [kN]					Serviceability [kN]			
Анау	[m <sup>2</sup> ]	Pressure ⊥	Pressure II	Uplift ⊥	Uplift II	Pressure ⊥	Pressure II	Uplift ⊥	Uplift II	
field area	10.00	0.553	0.119	-0.463	0.022	0.438	0.094	-0.345	0.022	
ridge	10.00	0.415	0.089	-0.825	0.017	0.328	0.071	-0.634	0.017	
gableboard	10.00	0.415	0.089	-0.962	0.017	0.328	0.071	-0.741	0.017	
corner region (eave)	10.00	0.276	0.059	-0.687	0.011	0.219	0.047	-0.530	0.011	
eaves	10.00	0.553	0.119	-0.645	0.022	0.438	0.094	-0.488	0.022	

### Resistance Values of Components

#### Base Rails

Base Rails	А	Ι <sub>y</sub>	I <sub>z</sub>	$W_y$	$W_z$
	[cm <sup>2</sup> ]	[cm^4]	[cm^4]	[cm <sup>3</sup> ]	[cm <sup>3</sup> ]
K2 BasicRail 22	2.380	1.52	7.74	1.08	2.46

#### Fastener

Fastener	$R_{D,Uplift,Perpendicular}$ [kN]	$R_{D,Pressure,Perpendicular}$ [kN]	$R_{D,Pressure,Parallel}$ [kN]
K2 BasicClip	1.02	-	0.96
Thread-forming metal screw 6.0×38	0.65	-	0.62

## Connecting StrengthStructural analysis report | Roof 2

#### Utilisation result

		ultimate limit state U		ultimate limit state		Distan	ces	maxim	um values
No.	roof areas	Pr	CL	Fst	Pr	Fst	BR	CL	Fst
Module Array		σ[%]	σ[%]	F[%]	f[%]	[m]	[m]	L <sub>max</sub> [m]	Fst D <sub>max</sub> [m]
1	field area	22.4	1.9	47.6	32.6	0.800		0.466	0.850
1	ridge	23.1	3.4	82.4	19.9	0.600		0.411	0.728
1	gableboard	26.9	4.6	95.8	23.2	0.600		0.394	0.627
1	corner region (eave)	12.8	4.9	68.3	4.9	0.400		0.400	0.586
1	eaves	24.3	1.8	65.4	36.3	0.800		0.452	0.850

Pr	Profile
Fst	Fastener

σ Stress

f Deflection

F Force

 $\text{CL}/\text{L}_{\text{max}}$  maximum cantilever length

 $\mathsf{Fst} \; \mathsf{D}_{\mathsf{max}} \quad \textbf{maximum fastener spacing}$ 

BR Base Rail

Usab. serviceability limit state

CL Cantilever

## Connecting StrengthRoofs | Roof 2 | Bill of material

Position	ltem no.	Item description	Quantity	Weight
1	1001164	K2 BasicClip	493	14.8 kg
2	1005193	Thread-forming metal screw 6.0×38	986	6.9 kg
3	2003072	OneMid Black Set 30-42	216	17.1 kg
4	2002589	OneEnd Black Set 30-42	64	5.6 kg
5	2003240	K2 BasicRail 22; 4.40m	68	192.4 kg
6	1003571	K2 BasicRail BasicConnector Set	64	3.1 kg
7	1003558	K2 BasicRail BasicLock 22 Set	32	1.6 kg
Total				241.4 kg

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## I Connecting StrengthBill of material

Position	ltem no.	Item description	Quantity	Weight
1	1001164	K2 BasicClip	960	28.8 kg
2	1005193	Thread-forming metal screw 6.0×38	1,920	13.4 kg
3	2003072	OneMid Black Set 30-42	432	34.1 kg
4	2002589	OneEnd Black Set 30-42	128	11.1 kg
5	2003240	K2 BasicRail 22; 4.40m	136	384.7 kg
6	1003571	K2 BasicRail BasicConnector Set	120	5.9 kg
7	1003558	K2 BasicRail BasicLock 22 Set	64	3.2 kg
Total				481.3 kg

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## Thank you for choosing a K2 mounting system.

Systems from K2 Systems are quick and easy to install. We hope these instructions have helped. Please contact us with any questions or suggestions for improvement.

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Our General Terms of Business apply. Please refer to <u>k2-systems.com</u>

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