

DRAINAGE STATEMENT

A/191327-MAINS OF CARNOUSIE, FORGLEN, TURRIFF.

1. **EXISTING SITE CONDITIONS.**

The site is located within agricultural land on the Mains of Carnousie in Forglen, between Turriff and Aberchirder. The site is in two parts, but effectively the same agricultural fields with a central tree belt strip between them.

The proposal is to construct new housing plots on the site, 8 plots in site 1 and 7 plots in site 2.

There are no known springs, wells or boreholes within 100m of the proposed site.

2. **GROUND CONDITIONS.**

Trial pit investigations were undertaken by Cameron + Ross on 20th September 2020. The summary of this would highlight the following:-

Topsoil was encountered as the uppermost horizon across the site with a general thickness of 0.3m-0.4m. The subsoils below this are generally described as dense slightly SILTY SANDS and GRAVELS. The subsoils are generally light brown or orange brown in colour. All trial pits remained stable. No ground water entries were encountered with all trial pits remaining dry to their final depths at the time of the investigation. Bedrock was not thought to have been encountered in any of the trial pits during the site investigation. Broken rock is present within the lower parts of the trial pits, this easily excavated through and providing good porosity through the open seams within this. Infiltration testing was carried out and confirmed that the strata is suitable for soakaways in respect to surface and foul water.

3. FOUL DRAINAGE PROPOSALS.

Each property is proposed to have its own foul water soakaway. On site 1, the Vp value is calculated as 60sec/mm. On site 2 the porosity is slightly better and the Vp is calculated at 10 sec/mm. Site 1 is proposed to have septic tanks and soakaways, and site 2 in accordance with SEPA WAT-RM-04 will have treatment plants and soakaways. Refer to Appendix 2.

4. **SURFACE WATER PROPOSALS.**

It is proposed that all surface water run-off will be collected and be discharged into designed soakaways. See attached design calculation for the private surface water soakaway **Appendix 1 and Drawing 900 in Appendix 2.** The soakaways are adequately sized to accommodate the run-off from each plot.



The proposed roads at the two sites will be self-draining to the sub-soils. The construction with be a hardcore sub-base with granite stones and dusting to top surface. These will be laid to a slight cross-fall to shed any water and not allowing any ponding. No specific soakaways are required due to these areas being self-draining.

5. CONSTRUCTION PHASE.

Should any existing surface water drains be encountered within the site they will inevitably be disturbed by construction works therefore, should it prove necessary these will be re-routed or connected to a new perimeter land drain to intercept site run-off. No such drains were encountered in the site investigation.

The measures for controlling surface water run-off will be continually reviewed in line with each stages of construction and any influencing factors.

Consideration is to be given, in the main, to surface water run off during and after topsoil strip, after any re-grading of the land and during site construction. Stripping of topsoil and vegetation is to be limited wherever possible and undertaken just prior to the construction in that particular area. This is to provide a means of reducing run off and to remove silts/fines from the water and aid natural absorption into the soils.

6. **FUTURE MAINTENANCE.**

The on-site drainage systems will <u>not</u> be adopted by Scottish Water and will therefore, in terms of the maintenance of the soakaways the responsibility will lie with the owner of the property.

Each plot owner will be aware that a suitable maintenance regime is required and that they are responsible for this. The regime will be that the soakaways should be inspected on an annual basis. The distribution pipe system should be monitored for blockages and if necessary, the end caps removed and the pipes flushed through with a high volume, high pressure pump to dislodge any silts / sludge which may be causing blockage.

In the event of a failure, the failed element will be excavated and replaced to the same specification as existing.



APPENDIX 1 – SUDS CALCULATIONS

CALCULATION

Cameron+Ross

Contract MAINS OF CARROUSIE, MINGE Part of Structure

FOUL ORAINAGE

Sheet No. | Rev

Contract No. | 91327

Date 5/2/21

Designer GE2

Checker

PONOSITY SITE	TESTINE	CAu	CHES	out	ON .
AVELAGE) (TE	2			secs/n
BLE 365 AVE	SOAKANAM LAZE a JINGS		OTH	<i>+</i> =	1-667 ×
PLOTS 1-	8				
A = Vpf	0.25				
PLOD 1,2,	3, 6 + 7	A=	60 x	6 X	0.25
PLOD 4,5	- + 8	A=	60 x 7 105	* 0	125
P60 9-	15				
A = 3.6	PE =	9,1 A-	0, 13, 1	4+1	5 21.6n
11 + 12	A=	1 1 1 1	7 =		1 1: 1 1:



Project: Address:

Houses Carnousie Forglen

Date: Turriff

A 191327 Feb 2021 **GGC**

Location:

Plot 1 soakaway

Job No: Calcs by: Page No: BRE365

Design Rainfall

Additional flow multiplier

30%

From Wallingford Procedure, Volume 3 - Maps Rainfall Depths (M5 - 60minutes)

 $M5_{60} =$ 16 mm

from BRE Digest 365, fig. 1

rainfall ratio r = 0.250

Design Storm Return Period,

P = 30 years

D	M5_D	Z2	R = MP_D	Rainfall
mins				Intensity
5	4.7 mm	1.872	8.7 mm	105 mm/hr
10	7.0 mm	1.898	13.3 mm	80 mm/hr
15	8.7 mm	1.919	16.6 mm	67 mm/hr
30	12.0 mm	1.938	23.2 mm	46 mm/hr
60	16.0 mm	1.936	31.0 mm	31 mm/hr
120	21.0 mm	1.912	40.1 mm	20 mm/hr
240	27.3 mm	1.888	51.5 mm	13 mm/hr
360	31.7 mm	1.869	59.3 mm	10 mm/hr
600	38.3 mm	1.844	70.6 mm	7 mm/hr
1440	52.7 mm	1.780	93.8 mm	4 mm/hr
2880	67.8 mm	1.747	118.5 mm	2 mm/hr

- Scotland and Nth Ireland
- England and Wales

Measured Infiltration Rate 1.67E-05

Infiltration Rate (eff) Impermeable Area Width Depth Fixed Lgth (optional)

1.67E-05	
136	m^2
5.00	m
1.00	m
0	m

(OR Outlet Flow Rate I/s) 0 m³/hr ie

Gravel Pit or Trench Soakaway

Gravel, free volume 30% Insert 100% for Net Storage Chamber Volume

D	Length	Inflow	Outflow	Storage Req	t _{s50} (hrs)	Storage Prov	Overflow
5	1	1.2	0.0	1.2	1.67	1.2	
10	1	1.8	0.1	1.8	2.37	1.8	
15	1	2.3	0.1	2.2	2.80	2.2	
30	2	3.2	0.2	2.9	3.53	2.9	
60	3	4.2	0.5	3.8	4.18	3.8	
120	3	5.5	1.0	4.5	4.69	4.5	
240	3	7.0	2.0	5.0	5.00	5.0	
360	3	8.1	3.0	5.0	5.03	5.0	
600	3	9.6	4.9	4.7	4.82	4.7	
1440	2	12.8	9.9	2.8	3.43	2.8	
2880	0	16.1	15.5	0.6	0.91	0.6	

Time until system can cope with additional influx of 50% design storage volume < 24 hrs ~ OK

Provide gravel filled soakaway, 3.5 m x 5 m x 1 m deep

Minimum Free Volume = 30% Total Pit Volume = 17.5m³



Project: Address:

Houses Carnousie Forglen

Date: **Turriff**

A 191327 Feb 2021

Location:

Plot 2 soakaway

Job No: Calcs by: **GGC** Page No: BRE365

Design Rainfall

Additional flow multiplier

30%

From Wallingford Procedure, Volume 3 - Maps Rainfall Depths (M5 - 60minutes)

 $M5_{60} =$ 16 mm

from BRE Digest 365, fig. 1

rainfall ratio r = 0.250

Design Storm Return Period,

P = 30 years

D a	M5_D	Z2	$R = MP_D$	Rainfall
mins				Intensity
5	4.7 mm	1.872	8.7 mm	105 mm/hr
10	7.0 mm	1.898	13.3 mm	80 mm/hr
15	8.7 mm	1.919	16.6 mm	67 mm/hr
30	12.0 mm	1.938	23.2 mm	46 mm/hr
60	16.0 mm	1.936	31.0 mm	31 mm/hr
120	21.0 mm	1.912	40.1 mm	20 mm/hr
240	27.3 mm	1.888	51.5 mm	13 mm/hr
360	31.7 mm	1.869	59.3 mm	10 mm/hr
600	38.3 mm	1.844	70.6 mm	7 mm/hr
1440	52.7 mm	1.780	93.8 mm	4 mm/hr
2880	67.8 mm	1.747	118.5 mm	2 mm/hr

- Scotland and Nth Ireland
- England and Wales

Measured Infiltration Rate 1.67E-05

Infiltration Rate (eff)
Impermeable Area
Width
Depth
Fixed Lgth (optional)

1.67E-05	
260	m ²
5.00	m
1.00	m
0	m

(OR Outlet Flow Rate		l/s)
ie	0	m ³ /hr

Gravel Pit or Trench Soakaway

Gravel, free volume 30%

Insert 100% for Net Storage Chamber Volume

D	Length	Inflow	Outflow	Storage Req	t _{s50} (hrs)	Storage Prov	Overflow
5	1	2.3	0.0	2.2	2.87	2.2	
10	2	3.5	0.1	3.4	3.89	3.4	
15	3	4.3	0.1	4.2	4.49	4.2	
30	4	6.0	0.3	5.8	5.43	5.8	
60	5	8.1	0.6	7.5	6.23	7.5	
120	6	10.4	1.3	9.1	6.85	9.1	
240	7	13.4	2.9	10.5	7.29	10.5	
360	7	15.4	4.4	11.0	7.42	11.0	
600	7	18.3	7.4	11.0	7.42	11.0	
1440	6	24.4	15.6	8.8	6.74	8.8	
2880	4	30.8	25.2	5.6	5.35	5.6	

Time until system can cope with additional influx of 50% design storage volume < 24 hrs ~ OK

Provide gravel filled soakaway, 7.5 m x 5 m x 1 m deep

Minimum Free Volume = 30% Total Pit Volume = 37.5m³



Project: Address:

Houses Carnousie Forglen

Turriff Da

A 191327 Feb 2021

Location:

Plot 4 soakaway

Job No: Date: Calcs by: Page No:

GGC BRE365

Design Rainfall

Additional flow multiplier

30%

From Wallingford Procedure, Volume 3 - Maps Rainfall Depths (M5 - 60minutes)

M5_60 = 16 mm

from BRE Digest 365, fig. 1

rainfall ratio r = 0.250

Design Storm Return Period,

P = 30 years

- 1					
	D	M5_D	Z2	$R = MP_D$	Rainfall
	mins				Intensity
	- 5	4.7 mm	1.872	8.7 mm	105 mm/hr
	10	7.0 mm	1.898	13.3 mm	80 mm/hr
	15	8.7 mm	1.919	16.6 mm	67 mm/hr
	30	12.0 mm	1.938	23.2 mm	46 mm/hr
	60	16.0 mm	1.936	31.0 mm	31 mm/hr
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- Scotland and Nth Ireland
- England and Wales

Measured Infiltration Rate 1.67E-05

In	filtration Rate (eff)
In	npermeable Area
W	/idth
D	epth
Fi	ixed Lgth (optional)

1.67E-05	m/s
260	m ²
5.00	m
1.00	m
0	m

(OR Outlet Flow Rate		l/s)
ie	0	m³/hr

Gravel Pit or Trench Soakaway

Gravel, free volume 30%

Insert 100% for Net Storage Chamber Volume

D	Length	Inflow	Outflow	Storage Req	t _{s50} (hrs)	Storage Prov	Overflow
5	1	2.3	0.0	2.2	2.87	2.2	
10	2	3.5	0.1	3.4	3.89	3.4	
15	3	4.3	0.1	4.2	4.49	4.2	
30	4	6.0	0.3	5.8	5.43	5.8	
60	5	8.1	0.6	7.5	6.23	7.5	
120	6	10.4	1.3	9.1	6.85	9.1	
240	7	13.4	2.9	10.5	7.29	10.5	
360	7	15.4	4.4	11.0	7.42	11.0	
600	7	18.3	7.4	11.0	7.42	11.0	
1440	6	24.4	15.6	8.8	6.74	8.8	
2880	4	30.8	25.2	5.6	5.35	5.6	

Time until system can cope with additional influx of 50% design storage volume < 24 hrs ~ OK

Provide gravel filled soakaway, 7.5 m x 5 m x 1 m deep

Minimum Free Volume = 30% Total Pit Volume = 37.5m³



APPENDIX 2- DRAINAGE PROPOSALS



