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Project:	Arrochar,	FLOOD FLOW LTD
	Prescot Road Melling	CONSULTING CIVIL AND STRUCTURAL ENGINEERS
	Sefton L31 1AT	36 Holdenbrook Close Leigh WN7 2HL
Subject:	SuDS Drainage Statement	Tel 07940 523819 Website: www.floodflow.co.uk Email: mail@floodflow.co.uk
Date:	7 th December 2023 (Issue 02)	

1.0 GENERAL

Project No: 23113

This drainage statement has been prepared to support the planning application for a proposed Erection of 4 no. dwellings to be accessed from existing entrance off Prescot Road.

The site is currently featuring an existing dwelling and outbuildings as shown below.



Figure 1 – Site Location Plan

Director: Jason.Jones B.Eng (Hons) MCIHT NMICE, Associate Director: Adam John Jones B.Eng (Hons) MEng CIWEM

2.0 DRAINAGE DESIGN

2.1 Surface Water Drainage

2.1.1 Flood Risk

The site is less than 1.0 ha and lies wholly within the Zone 1 Flood Map. This means the land is assessed as having a low probability of flooding, which is less than 1 in 1000 (<0.1%). As such a site-specific Flood Risk Assessment is not required.



Figure 2 – Flood Map for Planning

2.1.2 Drainage Strategy

1st Choice - Discharge to Soakaway

In accordance with the Building Regulations hierarchy for the disposal of surface water drainage, the initial consideration will be for a soak-a-way system.

Percolation tests have not been carried out at the development site. From online soil data the site is "Naturally wet very acid sandy and loamy soils" and the BGS website states Till, Devensian - Diamicton superficial deposits.

Due to poor percolation results soakaways are not considered viable.

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2nd Choice - Discharge to Watercourse

The second consideration is disposal of surface water to watercourses/ ditches. There is a brook near the proposed development. This is not the preferred discharge option for surface water discharge due to the distance of the connection. However, the existing highways drainage is likely to connect into the brook.

3rd Choice - Discharge to Sewer

The third option in the drainage hierarchy is to discharge into the existing public sewers. There are no public surface water sewers recorded in Prescot Road– see Appendix A and Figure 3 below.



Figure 3.1 – Extract from United Utilities records

There is an existing highways drainage network serving Prescot Road and it is preferred to connect into this network.



Figure 3.2 – Image of existing pipe and gully network

The surface water runoff generated from the existing site has been calculated at 9.87 l/s based on an existing impermeable area of 1418m^2 , using the modified rational method and rainfall of 50mm/hr. Including 50% betterment would provide 4.9 l/s maximum discharge.

The proposed impermeable area has been calculated at $1604m^2$, which is more than the existing impermeable area. The flow rate proposed above has been used to restrict the flow and estimate the required attenuation contained in Appendix B.



1% annual exceedance rainfall event

Epoch

	Central allowance	Upper end allowance
2050s	25%	40%
2070s	30%	45%

*Use '2050s' for development with a lifetime up 2060 and use the 2070s epoch for development with a lifetime between 2061 and 2125.

Figure 4 – Climate Change

Director: Jason.Jones B.Eng (Hons) MCIHT NMICE, Associate Director: Adam John Jones B.Eng (Hons) MEng CIWEM The design criteria for network modelling to be used for the site is:

- 1 in 30 year no surcharging of the surface water network.
- 1 in 100 year (+ 45% climate change, see Figure 4 above) will be contained within the attenuation, and the volume has been indicated for the development site.
- The roof water for the proposed development will also enter the existing ditch.
- A bypass separator upstream of the proposed attenuation structure is not considered a requirement due to the limited number of parking bays for the proposed development.

2.2 Foul Drainage

The foul drainage for the proposed development is to be connected into a proposed Bio Disc sewage treatment plant as shown on the drainage layout.

Appendices

- Appendix A United Utilities Drainage Records
- Appendix B Attenuation Calculations
- Appendix C Flood Flow Drawing Number 23113.001A Drainage Layout.
- Appendix D Flood Map for Planning



APPENDIX A





APPENDIX B

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23113 Arrochar, Prescot Road, Melling

Attenuation Calculations

🖌 Quick Storage	e Estimate		
Micro	Variables FSR Rainfall	Cv (Summer)	0.750
Variables Results Design Overview 2D	Return Period (years) 100 Region England and Wales Map M5-60 (mm) Ratio R	Cv (Winter) Impermeable Area (ha) Maximum Allowable Discharge (l/s) Infiltration Coefficient (m/hr) Safety Factor Climate Change (%)	0.840 0.160 4.9 0.00000 2.0 45
Overview 3D Vt	- Enter Climate Change	Analyse OK between -100 and 600	Cancel Help

🕖 Quick Storage	Estimate				
	Results				
Micro Drainage	Global Variables require approximate storage of between 57 m ³ and 85 m ³ .				
Variables					
Results					
Design					
Overview 2D					
Overview 3D					
Vt					
Analyse OK Cancel Help					
Enter Climate Change between -100 and 600					

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APPENDIX C



DRAINAGE NOTES



- 1. THIS DRAWING IS TO BE READ IN CONJUNCTION WITH ALL RELEVANT ARCHITECTS, ENGINEERS DRAWINGS AND SPECIFICATIONS.
- 2. DO NOT SCALE THIS DRAWING. ANY AMBIGUITIES, OMISSIONS AND ERRORS ON THE DRAWINGS SHALL BE BROUGHT TO THE ENGINEERS ATTENTION IMMEDIATELY. ALL DIMENSIONS MUST BE CHECKED AND VERIFIED ONSITE.
- ALL DRAINAGE WORKS TO BE IN ACCORDANCE WITH THE CIVIL ENGINEERING SPECIFICATION FOR THE WATER INDUSTRY (CESW) AS INCLUDED IN SERVERS FOR ADOPTION TH'EDITION, THE REQUREMENTS OF APPROVED DOCUMENT H (2015 EDITION), BUILDING REGULATIONS 200 AND THE ENTLAS PROVIDED ON THE CONTRACT DRAWINGS.
- 4. THE CONTRACTOR MUST SURVEY THE RETAINED DRAINAGE AND REPORT THE LINE, LEVEL AND CONDITION OF THE EXISTING DRAINAGE TO THE ENGINEER. WE WOULD RECOMMEND THAT THE EXISTING DRAINAGE TO BE SURVEYED IS CLEANEED BEFORE UNDERTAINING THESE WORKS.
- 5. ANY REDUNDANT MANHOLES ARE TO BE BROKEN OUT AND BACKFILLED WITH A APPROVED COMPACTED GRANULAR MATERIA REDUNDANT PIPES ARE TO BE FILLED WITH A 10:1 PFA/CEMENT MIX OR BROKEN OUT AND BACKFILLED WITH A APPROVED COMPACTED GRANULAR MATERIAL
- ALL FOUL AND SURFACE WATER DRAINS UP TO AND INCLUDING 300MM DIAMETER ARE TO BE VITRIFIED CLAY SUCH AS HERWORTH SUPERSLEVISUIPERSEL OR SIMILAR APPROVED (ISIS EN 26-1). ALL PIPES GREATER THAN 300MM DIAMETER TO BE CONCRETE CLASS 120 (ISIS BH 162020). ALL CONCRETE MANHOLES TO BE IM ACCORDANCE WITH BS IN 1917.
- AS AN ALTERNATIVE (SUBJECT TO THE CLIENTS APPROVAL) THE CONTRACTOR MAY USE STRUCTURAL WALLED PIPES (WISA 4:35-01 & BS EN 13476) SUCH AS;
- POLYSEWER POLYPIPE BUILDING PRODUCTS SIZE 150mm TO 300mm
 QUANTUM- MARLEY SIZE 150mm TO 300mm
 ULTRARIB UPONOR SIZE 150mm TO 300mm
 ULTRARIB WAVIN SIZE 150mm TO 300mm
 WEHOLTE ASSET INTERNATIONAL SIZE 450mm TO 3000mm

- 8. PIPES LAID WITHIN VEHICLE TRAFFICKED AREAS WITH LESS THAN 900mm OF COVER SHALL BE SURROUNDED IN CLASS Z BEDDING, PIPES LESS THAN 300mm BELOW THE UNDERSIDE OF A GROUND FLOOR SLAB SHALL BE SURROUNDED IN CLASS Z BEDDING, WHERE CLASS Z BEDDING IS USED AS A SURROUND A COMPRESIBLE MATERIAL MUST BE PLACED AT EVERY PIPE JOINT. ALL OTHER PIPES ARE TO BE LAID IN A CLASS S BEDDING.
- 9. ALL DRAINAGE MUST BE PROTECTED DURING CONSTRUCTION WHERE INTERMEDIATE COVER IS LESS THAN 900mm
- 10. WHERE FOUL AND SURFACE DRAINS/SEWERS CROSS WITHIN 100mm OF EACH OTHER CONCRETE PROTECTION (CLASS Z BEDDING) MANY BE REQUIRED TO PREVENT ANY POTENTIAL CONTAMINATION.
- 11. ALL COVER LEVELS ARE APPROXIMATE ONLY. ALL MANHOLE COVERS TO BE SET AT THE PROPOSED FINISHED PAVEMENT OR FLOOR LEVEL REFER TO THE ARCHITECTS PROPOSED LEVELS DRAWING FOR LEVEL CONFIRMATION.
- FOR TYPICAL MANHOLE CONSTRUCTION DETAILS, PIPE BEDDING/TRENCH DETAILS AND OTHER ASSOCIATED DRAINAGE DETAILS REFER TO SGI TYPICAL DETAILS DRAWING.
- 13. ALL FOUL AND SURFACE WATER CONNECTIONS TO BE 100MM DIAMETER UNLESS STATED ALL EXTERNAL GULLY CONNECTION AND CHANNEL DRAN SUMPIGULLY CONNECTIONS TO BE 150mm DIAMETER UNLESS STATED ALL GULLY AND CHANNEL DRAN OUTLETS TO BE TRAFFED AND RODOBALEL ALL INTERNAL GULLES AND CHANNEL DRANS TO BE SPECIFIED BY OTHERS.
- 14. CHANNELS DRAINS TO BE FITTED WITH A HEALGUARD CAST IRON GRATING. GRATINGS TO BE TO LOAD CLASS D400 SPECIFICATION UNLESS OTHERWISE AGREED. LINEAR CHANNELS TO HAVE A 200mm MINIMUM CONCRETE BED AND HAUNCH.
- 15. ALL FOUL STACKS AND RWPS TO HAVE LOW LEVEL RODDING ACCESS PLATES UNLESS AN ALTERNATIVE MEANS OF ACCESS IS AGREED. ACCESS POINT SIZE TO BE IN ACCORDANCE WITH DOCUMENT H AND SITED ABOVE ANY CONNECTED GROUND FLOOR APPLIANCE SPLIL LEVEL.
- ALL CONNECTIONS PASSING THROUGH FOUNDATION BASES AND/OR EDGE BEAMS TO BE WITHIN SEALED SLEEVES. ALTERNATIVELY CONNECTIONS MAY BE CAST-IN WITH FLEXIBLE JOINTS NOT GREATER THAN 150mm FROM FACE OF THE CONCRETE.
- 17. ALL MANHOLE COVERS LOCATED WITHIN THE ROAD AND CAR PARKING AREAS TO BE D400 CLASS. COVERS WITHIN HARD AND SOFT LANDSCAPED AREAS WITH PEDESTRIAN TRAFFIC ONLY TO BE B125 CLASS. COVERS LOCATED WITHIN BLOCKSLABE PAVING AREAS TO BE RECESSED TO SUIT THE PROPOSED PAVING AND OF THE APPROPRIATE GRADE. ALL INTERNAL COVERS TO BE RECESSED WITH D0UBLE SEALED BOLT D00WI COVERS SULVA AS HOWE GREEN \$000 SENES OR SMILLAR APPROVED. 18. CHANNELS WITHIN TYPE 13. SUMAHOLES MISUT UBE REF.FORMEC OLVAWARE SECTIONS FOR IPPES UP TO AND INCLUDING 300mm DIAMETER. CHANNELS TO BE SET AT THE APPROPRIATE INCOMING AND OUTGOING PIPE GRADIENTS.
- 19. ALL MANHOLES CONNECTIONS TO BE FORMED AT SOFFIT TO SOFFIT UNLESS OTHERWISE STATED. ALL BRANCH CONNECTIONS TO BE MADE WITH SWEPT BENDS IN THE DIRECTION OF FLOW IN THE MAIN SEWER.
- 20. INTERNAL DRAINAGE CONNECTIONS ARE PROVIDED TO THE PENETRATION POSITIONS SHOWN ON AND COORDINATED BY THE ARCHITECTIM&E CONSULTANT.
- 21. EXTERNAL LINEAR CHANNEL DRAINS SHALL BE DESIGNED AND CONSTRUCTED IN ACCORDANCE WITH THE MANUFACTURERS REQUIREMENTS AND SHALL DISCHARGE VIA TRAPPED RODDABLE GULLY UNITS UNLESS ADVISED OTHERWISE.
- 22. THE TYPE AND SIZE OF THE SEPARATOR IS SPECIFIED ON THIS DRAWING AND SHALL BE A CLASS 1 TYPE. IT SHALL BE INSTALLED IN COMPLETE ACCORDANCE WITH THE MANUFACTURERS INSTRUCTIONS AND VETED VIA 78mm0 PIPE WORK. THE INTERCEPTOR REQUIRES POWER FOR THE HIGH LEVEL OL ALARM AND THE ARRANGEBURS SHALL BE INTSRUCTIONS AND EXPLOSION PROOF. THE CONTRACTOR SHALL PROVIDE THE REQUIRED DUCTIONS AND THE TRANSPORT SHALL BE INTSRUCTIONS AND POSITION TO RECHTER WITH AND/O AND VISUAL MONTORING INITIAL IN ACCORDANCE WITH PSSF71 2008 + 12.011
- 23. ALL RWP AND SVP POSITIONS ASSUMED, TO BE CONFIRMED
- 24. COVER LEVELS BASED ON ARCHITECT'S SITE LEVELS

F	ESIDUAL RISKS					
A	AMENDED SITE BOUNDARY	27	2	07112123		
AR.	DESCRIPTION	DRAWN	REPROVED	DATE		
STA						

PLANNING

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Client

SUSAN LOPPY

Project

PROPOSED DEVELOPMENT AT ARROCHAR, PRESCOT ROAD MELLING

Title

INDICATIVE DRAINAGE LAYOUT

Drawn	AJ	Checked JJ		J	Drawing number	
Date	01/08/23	Date	01/08	3/23	23113-001	Δ
Scale	1:200			A1	20110 001	<i>``</i>



APPENDIX D



Flood map for planning

Your reference 23113

Location (easting/northing) 340391/403039

Created **2 Aug 2023 7:02**

Your selected location is in flood zone 1, an area with a low probability of flooding.

You will need to do a flood risk assessment if your site is any of the following:

- bigger that 1 hectare (ha)
- In an area with critical drainage problems as notified by the Environment Agency
- identified as being at increased flood risk in future by the local authority's strategic flood risk assessment
- at risk from other sources of flooding (such as surface water or reservoirs) and its development would increase the vulnerability of its use (such as constructing an office on an undeveloped site or converting a shop to a dwelling)

Notes

The flood map for planning shows river and sea flooding data only. It doesn't include other sources of flooding. It is for use in development planning and flood risk assessments.

This information relates to the selected location and is not specific to any property within it. The map is updated regularly and is correct at the time of printing.

Flood risk data is covered by the Open Government Licence **which** sets out the terms and conditions for using government data. https://www.nationalarchives.gov.uk/doc/open-government-licence/version/3/

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