



**SEJC Consulting Engineers**

Consulting Structural & Civil Engineers

## STRUCTURAL CALCULATIONS

<b>PROJECT</b>	Annex Lift
<b>PROJECT No.</b>	23731
<b>CLIENT</b>	
<b>DATE</b>	February 2017
<b>NOTES</b>	Design of temporary lifting beams to be installed under an annex structure to enable safe lifting for transportation.
<b>BY</b>	<i>JG Smith</i>

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LOADING ( $\text{kN/m}^2$ ) - dead load only considered

Roof			
	Concrete tiles	0.8	(work)
	Battens	.03	
	on slope	.83	(22 1/2°)
	on plan	.90	
	Tressed rafters	.20	
	Ceiling	.15	
	Services (nom)	.05	
	Insulation	.03	
		<u>1.48</u>	say <u>1.5</u>

Floor		
	Boards	.10
	Joints	.15
	Insulation	.05
	"Under" board	.10
	Partitions (nom)	.50
		<u>.90</u>



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Walls	Central panels	·20
	Studs	·15
	Insulation	·05
	Plasterboard	·2
		<hr/>
		·60



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Lifting beams designed to support loads at foundation positions. Refer to plan for references - loads in kN

Position A

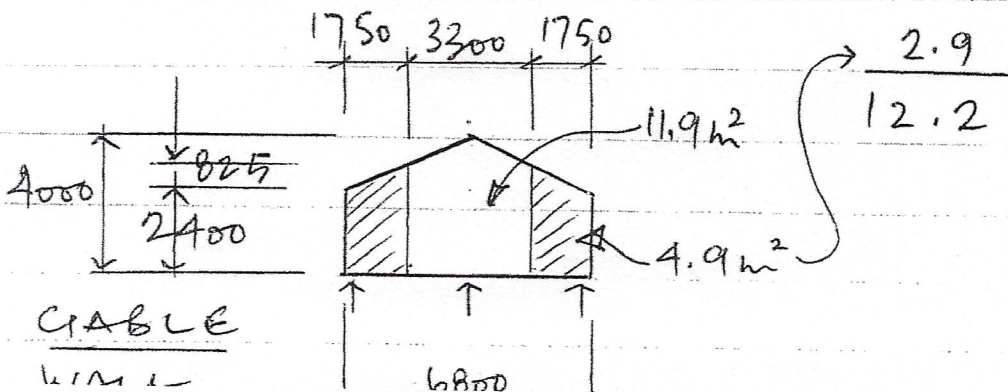
3.4 x 2.3m roof	11.7
1.65 x 2.3m floor	3.4
2.3 x 2.4m wall	3.3
	<u>18.4</u>

Position B

3.3 x 2.3m floor	6.8
------------------	-----

Position C

3.4 x 1.15m roof	5.9
1.65 x 1.15m floor	1.7
1.15 x 2.4m side wall	1.7







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Position D

3.3 x 1.15 floor

3.4

11.9 m<sup>2</sup> gable wall

7.1

10.5



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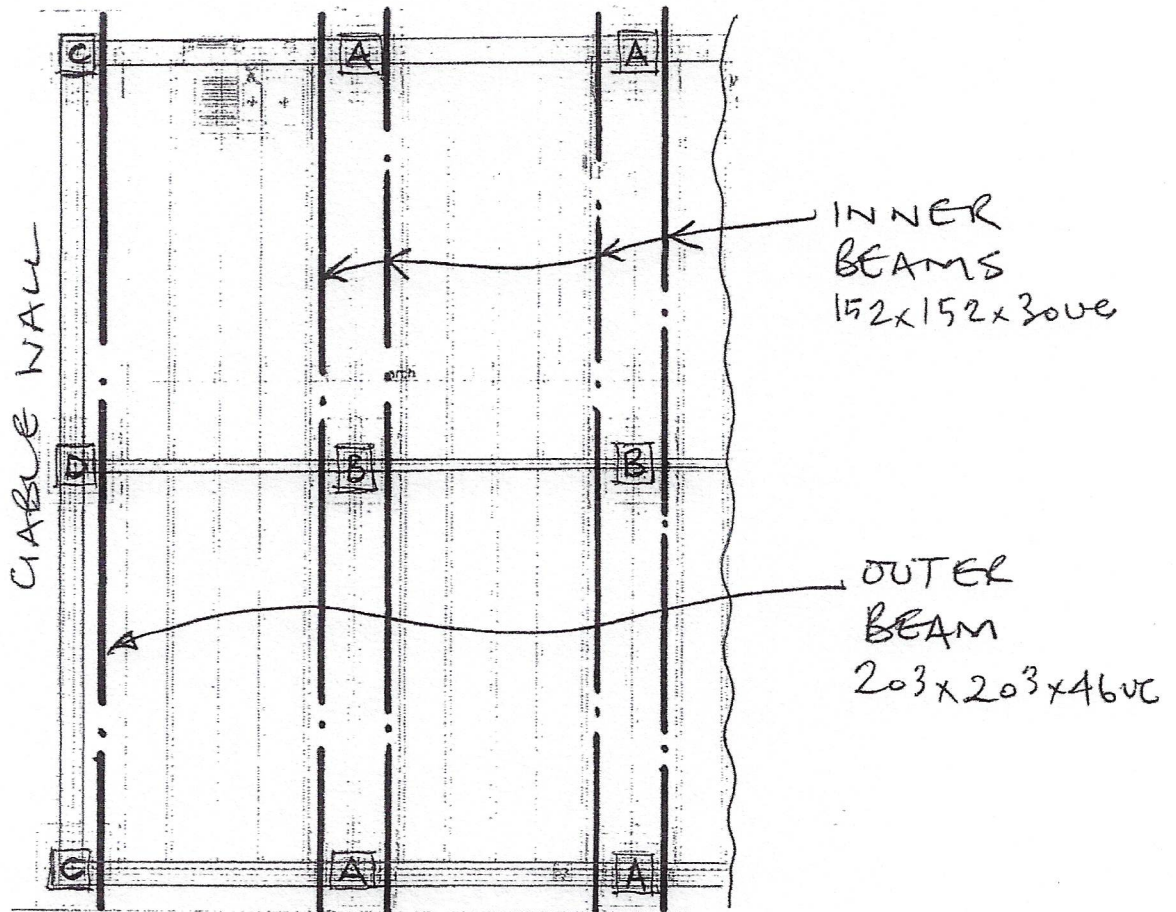
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Position lifting beams under the annex next to foundation positions - on inner lines of support use a beam either side of the foundation connection point, on outer lines of support position just inside the line of the wall:



PART PLAN NTS.



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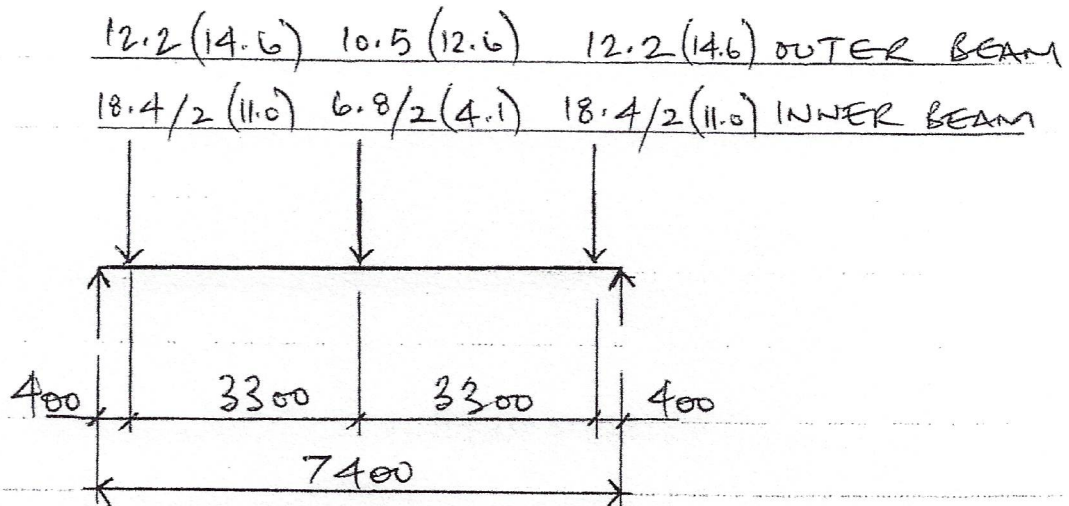
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Loads onto lifting beams (RN):



Values in parentheses include 20% for dynamic effect

See overleaf:

Outer beams 203 x 203 x 460e

Inner beams 152 x 152 x 300e in pairs.

Beam reactions for crane lifting including 20% allowance for dynamic effect.

Inner beams 13 RN

Outer beams 21 RN



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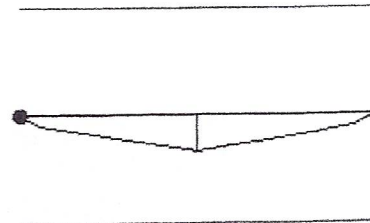
## OUTER BEAM

Loads described as "live" for analysis but  $\gamma = 1.4$  used for design

### Member Loading and Member Forces

Loading Combination : 1 UT + 1.4 D1 + 1.4 L1

L1 PY -014.600 0.400 ( kN, m )  
L1 PY -012.600 3.700 ( kN, m )  
L1 PY -014.600 7.000 ( kN, m )



Member Forces in Load Case 1 and Maximum Deflection from Load Case 2

Lev el No.	Node End1 End2	Axial Force (kN)	Shear Force (kN)	Bending Moment (kN.m)	Maximum Moment (kN.m @ m)	Maximum Deflection (mm @ m)
1	1 2	0.000C 0.000C	31.603 -31.603	0.000 0.000	45.144 @ 3.700	15.601 @ 3.700

### Classification and Properties (BS 5950: 2000)

Section (46.1 kg/m) 203x203 UC 46 [Grade 43]  
Class =  $F_n(b/T, d/t, p_y, F, M_x, M_y)$  9.25, 22.33, 275, 0, 45.14, 0 (Axial: Non-Slender) Compact  
Auto Design Load Cases 1

### Moment Capacity Check $M_c$

$F_v/P_v$  8.82 / 241.402 = 0.037 Low Shear  
 $M_c = p_y.S_{xx} \leq 1.2 p_y.Z_{xx}$  275 x 497.4  $\leq$  1.2 x 275 x 449.87 = 136.785 kN.m  
 $M_A/M_c$  45.143 / 136.785 = 0.330 OK

### Equivalent Uniform Moment Factor $m_{LT}$

$m_{LT} = 0.2 + (.15M_2 + .5M_3 + .15M_4)/M_{max}$  0.2 + (.15x28 + .5x45 + .15x28)/45 = 0.44 0.884 Table 18

### Lateral Buckling Check $M_b$

$L_e = 1.00 L$  1 x 7.4 = 7.4 m  
 $\lambda = L_e/r_{yy}$  7.4 / 5.14 143.97 OK  
 $v = F_n(x, L_e, r_{yy}, \lambda)$  17.71, 7.4, 5.14, 143.97 Table 19  
 $\lambda_{LT} = u.v.\lambda.\sqrt{\beta_w}$  0.847 x 0.694 x 143.97  $\sqrt{1}$  84.65  
 $p_b = F_n(p_y, \lambda_{LT})$  275, 84.65 154.6 N/mm<sup>2</sup> Table 16  
 $M_b = S_{xx}.p_b \leq M_c$  497.4 x 154.6  $\leq$  136.785 = 76.900 kN.m  
 $M_A/(M_b/m_{LT})$  45.144 / (76.9 / 0.884) 0.519 OK

### Deflection Check - Load Case 2

$\delta \leq \text{Span}/360$  15.6  $\leq$  7400 / 360 15.6 mm OK



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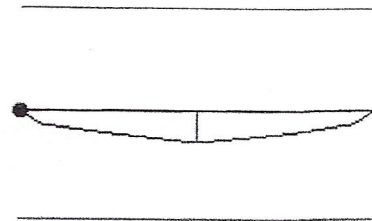
## INNER BEAM

Loads described as "live" for analysis but  $\gamma = 1.4$  used for design

### Member Loading and Member Forces

Loading Combination : 1 UT + 1.4 D1 + 1.4 L1

L1 PY -011.000 0.400 ( kN, m )  
L1 PY -004.100 3.700 ( kN, m )  
L1 PY -011.000 7.000 ( kN, m )



Member Forces in Load Case 1 and Maximum Deflection from Load Case 2						
Level No.	Node End1 End2	Axial Force (kN)	Shear Force (kN)	Bending Moment (kN.m)	Maximum Moment (kN.m @ m)	Maximum Deflection (mm @ m)
1	1 2	0.000C 0.000C	19.796 -19.796	0.000 0.000	19.602 @ 3.700	18.020 @ 3.700

### Classification and Properties (BS 5950: 2000)

Section (30.03 kg/m) 152x152 UC 30 [Grade 43]  
Class =  $F_n(b/T, d/t, p_y, F, M_x, M_y)$  8.13, 19.02, 275, 0, 19.6, 0 (Axial: Non-Slender) Plastic  
Auto Design Load Cases 1

### Moment Capacity Check $M_c$

$F_v/P_v$  2.87 / 169.026 = 0.017 Low Shear  
 $M_c = p_y.S_{xx} \leq 1.2 p_y.Z_{xx}$  275 x 247.7  $\leq$  1.2 x 275 x 221.94 = 68.118 kN.m  
 $MA/M_c$  19.6 / 68.118 = 0.288 OK

### Equivalent Uniform Moment Factor $m_{LT}$

$m_{LT} = 0.2 + (.15M_2 + .5M_3 + .15M_4)/M_{max}$   $0.2 + (.15 \times 14 + .5 \times 20 + .15 \times 14)/20 = 0.44$  0.908 Table 18

### Lateral Buckling Check $M_b$

$L_e = 1.00 L$  1 x 7.4 = 7.4 m  
 $\lambda = L_e/r_{yy}$  7.4 / 3.83 193.21 OK  
 $v = F_n(x, L_e, r_{yy}, \lambda)$  15.946, 7.4, 3.83, 193.21 0.588 Table 19  
 $\lambda_{LT} = u.v.\lambda.\sqrt{\beta_w}$  0.849 x 0.588 x 193.21  $\sqrt{1}$  96.53  
 $p_b = F_n(p_y, \lambda_{LT})$  275, 96.53 131.05 N/mm<sup>2</sup> Table 16  
 $M_b = S_{xx}.p_b \leq M_c$  247.7 x 131.05  $\leq$  68.118 = 32.461 kN.m  
 $MA/(M_b/m_{LT})$  19.6 / (32.461 / 0.908) 0.548 OK

### Deflection Check - Load Case 2

$\delta \leq \text{Span}/360$  18.02  $\leq$  7400 / 360 18.02 mm OK



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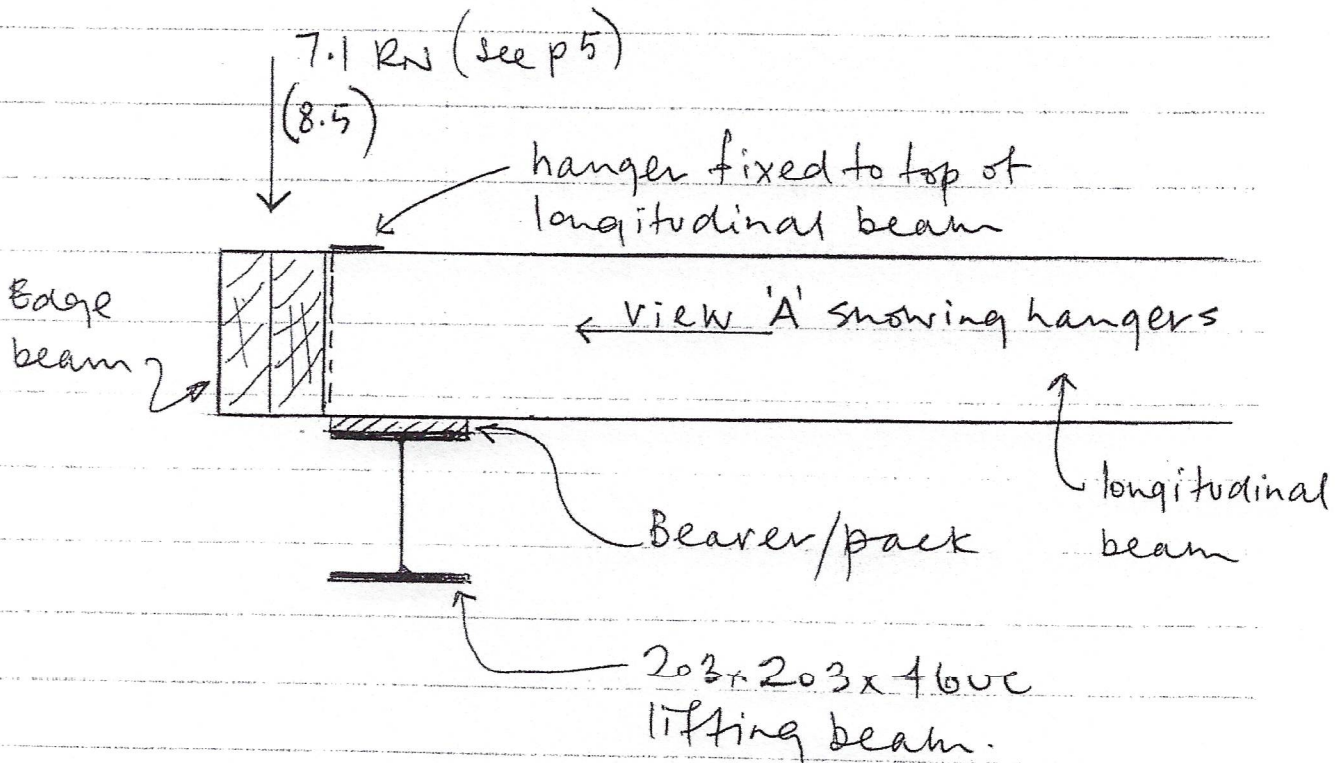
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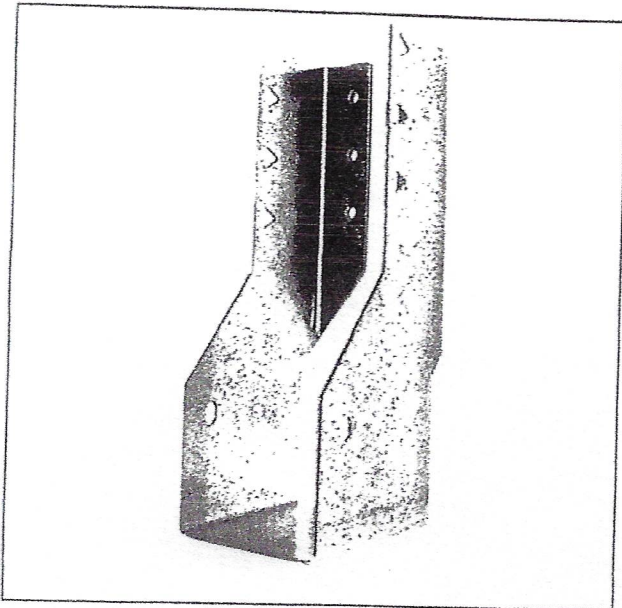
Inner beams will carry loads from the longitudinal timber beams  $\therefore$  downward forces directly onto the lifting beams.

The outer beams will transfer loading from the gable walls onto the ends of the longitudinal beams  $\therefore$  a suitable connection must be made between the timber beams:



Use Simpson Strong-Tie 1UC hangers -  
See table overleaf - for joists at least 50mm wide, 2 no hangers have SWL > 8.5 kN  
... 2 no hangers have SWL > 8.5 kN





IUC is a face fix concealed flange hanger for both I-joists and solid timber sections.

UK-DoP-e04/0042, ETA-04/0042

**FEATURES**

**Material**

Pre-galvanised mild steel.



\* Suitable hangers - for very short term  $4.88 \times 1.75 \times 2$  (2 hangers) = 17.1 kN.

**TECHNICAL DATA**

**IUC Installed with 3.75x30mm Nails - Safe Working Load**

References	Dimensions			Fasteners				Safe Working Loads [kN]	
	A	B	C	Header		Joist		CLG & J Joist Header	EST of LVL Header
				Qty	Specification	Qty	Specification	Long Term Download	Long Term Download
IUC142/40	40	142	51	6	3.75 x 30	2	3.75 x 30	2.09	2.44
IUC192/40	40	192	51	10	3.75 x 30	2	3.75 x 30	3.48	4.06
IUC217/40	40	217	51	12	3.75 x 30	2	3.75 x 30	4.17	4.88
IUC142/47	47	142	51	6	3.75 x 30	2	3.75 x 30	2.09	2.44
IUC192/47	47	192	51	10	3.75 x 30	2	3.75 x 30	3.48	4.06
IUC217/47	47	217	51	12	3.75 x 30	2	3.75 x 30	4.17	4.88
IUC192/50	50	192	51	10	3.75 x 30	2	3.75 x 30	3.48	4.06
IUC217/50	50	217	51	12	3.75 x 30	2	3.75 x 30	4.17	4.88
IUC192/53	53	192	51	10	3.75 x 30	2	3.75 x 30	3.48	4.06
IUC217/53	53	217	51	12	3.75 x 30	2	3.75 x 30	4.17	4.88
IUC192/61	61	192	51	10	3.75 x 30	2	3.75 x 30	3.48	4.06
IUC217/61	61	217	51	12	3.75 x 30	2	3.75 x 30	4.17	4.88
IUC192/66	66	192	51	10	3.75 x 30	2	3.75 x 30	3.48	4.06
IUC217/66	66	217	51	12	3.75 x 30	2	3.75 x 30	4.17	4.88
IUC192/72	72	192	51	10	3.75 x 30	2	3.75 x 30	3.48	4.06
IUC217/72	72	217	51	12	3.75 x 30	2	3.75 x 30	4.17	4.88





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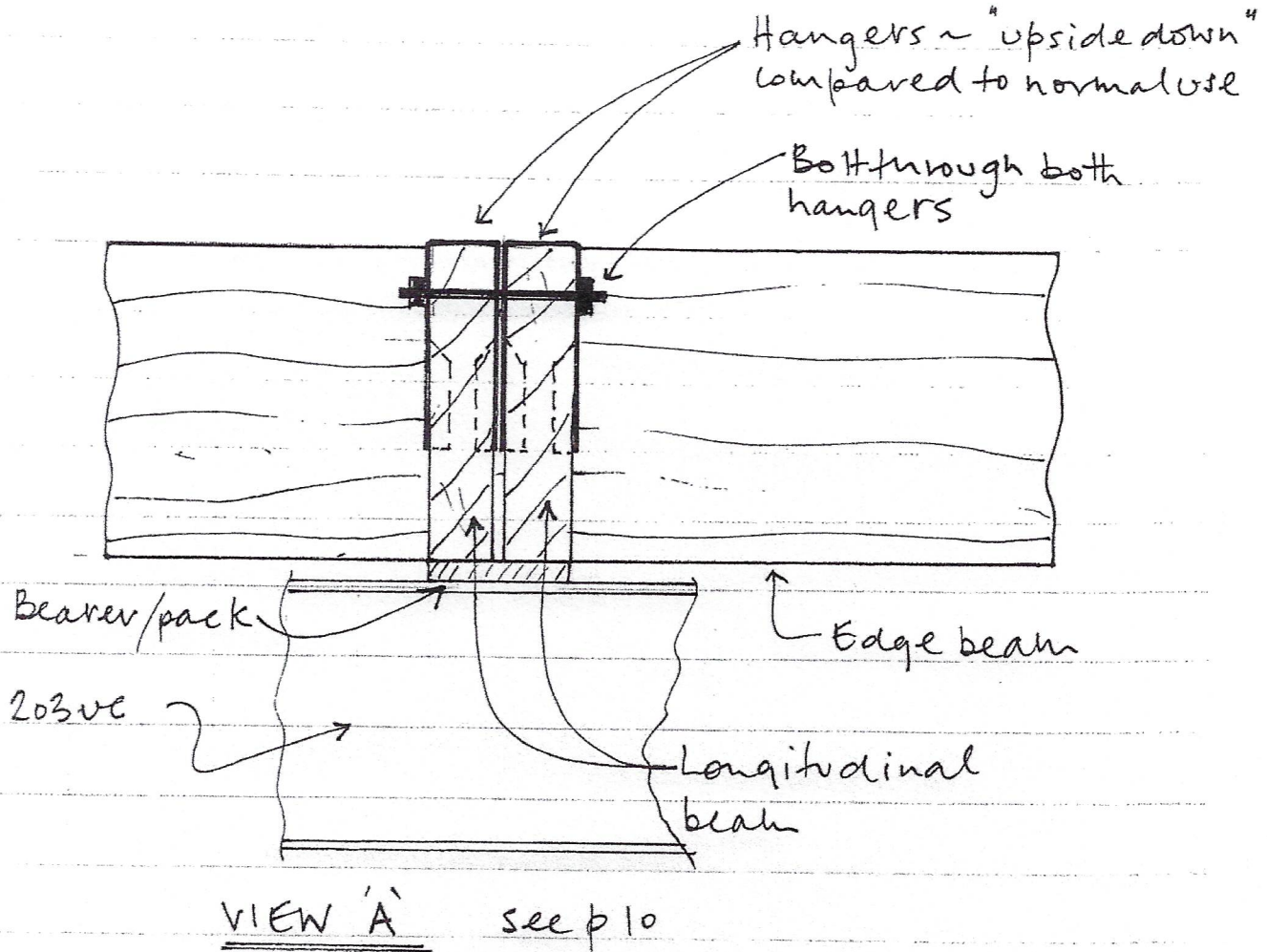
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## Appendix B – Construction Methodology

The photos show the following;

- A. Shows section 1 of floor laid on swift plinth foundation system
- B. Shows sections 1 & 2 of floor on central swift plinth
- C. Shows a concrete pad foundation system
- D. Shows the external walls for section 1 & 2 and the roofing spars for section 1
- E. Shows the roof with section 1 fitted with breather membrane and latts, section 2 roofing spars only. Shows the pitched roof with breather membrane and plats, with central divide between section 1 & 2.
- F. Shows the roof tiled with the divide used between sections 1 & 2
- G. Shows the 2 sections of floor deck, section 1 complete, section 2 to be insulated and board covered
- H. Shows a close up of the external wall cassettes for sections 1 & 2, with a coach bolt loosely inserted ready for the final structural act of joining the 2 sections together
- I. Shows section 1 & 2 wall frames ready to be jointed as the final structural act of bring the 2 sections together
- J. Shows a cross section photo through the 2 sections of roof joists, external wall cassettes and floor
- K. Shows the water pipes with connector joints splitting the water pipes between sections 1 & 2
- L. Shows the electrical wiring with connectors splitting the electrics between sections 1 & 2



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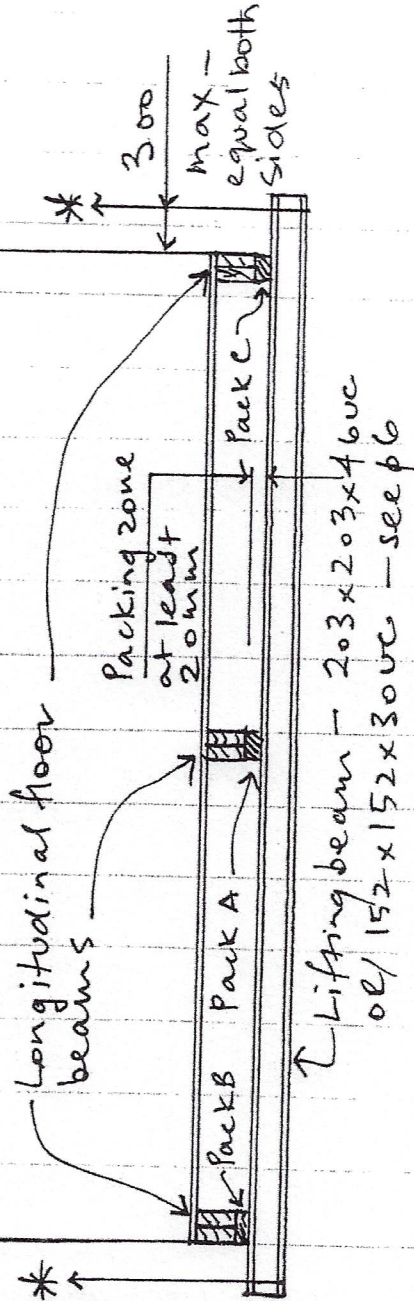
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Pack A to be fixed to top flange of steel lifting beam. Beams to be passed under the annex so pack A is located directly under central longitudinal floor beam. Crane to apply slight tension to ensure pack A is bearing correctly. Packs B and C then to be installed prior to lifting the annex.

\* Denotes lifting straps/chains fixed to the lifting beams below the annex floor and supported on a suitable steel spreader beam supplied by the crane company.





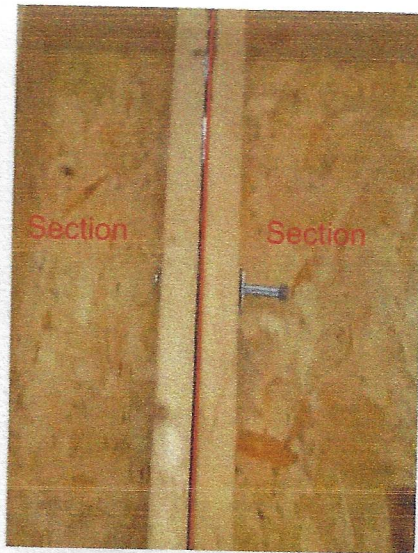
# Application for a certificate of lawfulness Under the Caravan Sites Act 1968

This is ADDENDUM 1 to the application submitted

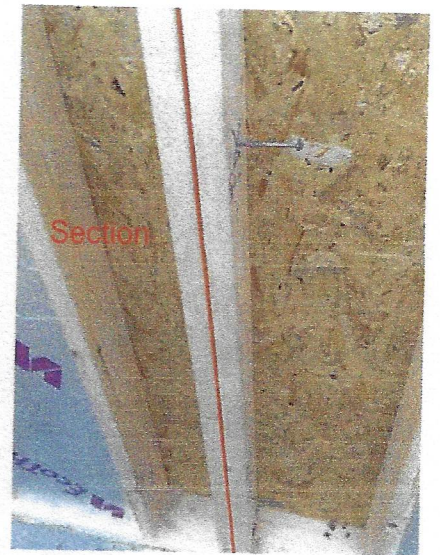
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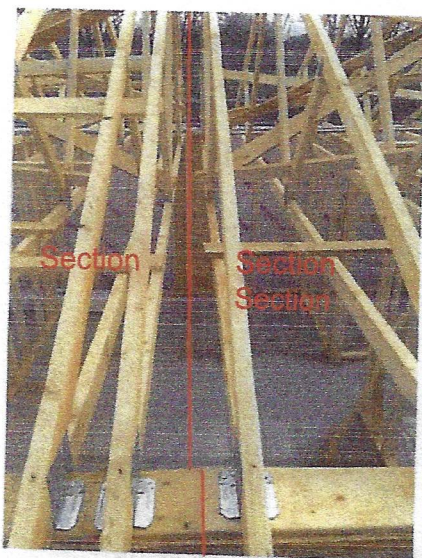
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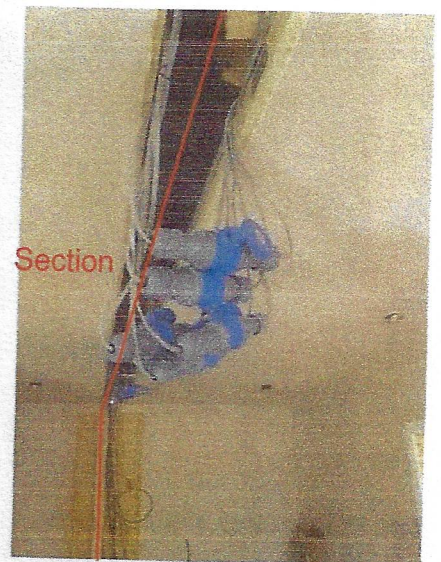
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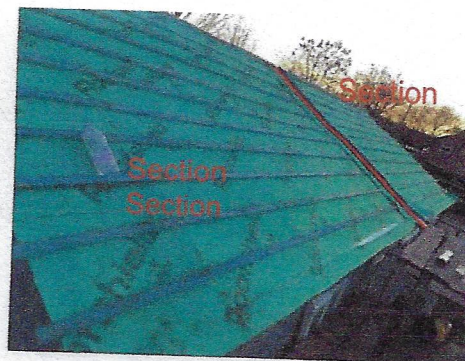
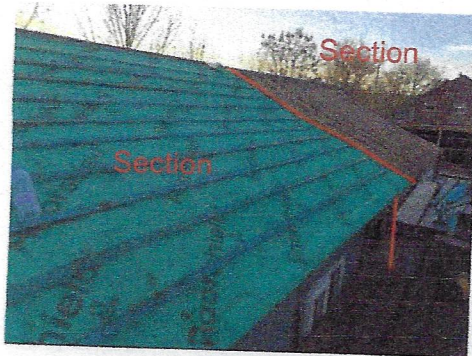
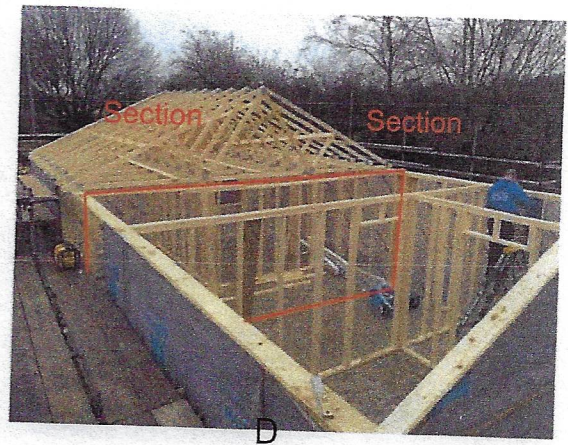
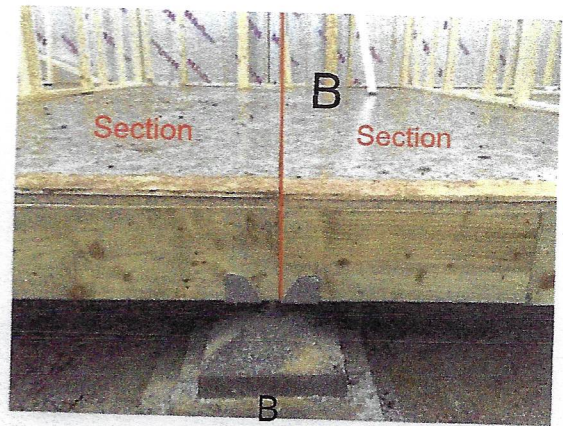
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## Photographic evidence



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