

flood risk assessment

Land East of High Road, High Cross

Hertfordshire

CCE/U321/FRA-03

November 2023

For

M Scott Properties Ltd

Document Review Sheet

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Reference	Date	Author	Checked
CCE/U321/FRA-01	February 2018	RTW	RBT
CCE/U321/FRA-02	October 2023	JH	RT
CCE/U321/FRA-03	November 2023	JH	

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Thames Water asset plans
BGS Infiltration GeoReport
Site Infiltration testing report

Appendix B - Proposed Site

Illustrative development layout Surface water management plan Impermeable catchment plan Surface water maintenance plan Surface water calculations

Summary Table

Site location Land East of High Road, High Road, High Cross

Hertfordshire SG11 1BE

Grid reference- 536586, 218803

Planning application Outline

Existing site Greenfield, agricultural farmland

Site area Approximately 7.75 ha

Proposed development Residential, up to 95 units

Flood Zone Zone 1

Surface Water Flooding Some areas of Medium/Low

Reservoir Inundation Zone None

Surface water management Attenuation sized to manage the 1 in 100

annual probability storm inclusive of 40 %

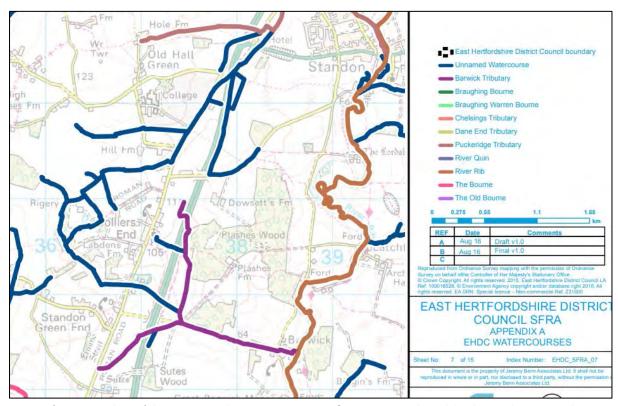
climate change.

Discharge limited to the mean annual

greenfield rate.

1.0 Introduction

- 1.1 This Flood Risk Assessment (FRA) has been prepared on behalf of M Scott Properties Ltd to support an outline planning application for the proposed development of land to the south of Sutes Farm, High Cross, Hertfordshire (refer to Figure 1 for a site location plan).
- 1.2 The proposals comprise the erection of up to 95 dwellings, with associated access, drainage, parking, landscaping and infrastructure (refer to Appendix B for an illustrative layout).
- 1.3 This assessment takes account of the National Planning Policy Framework (NPPF) and its associated Planning Practice Guidance (PPG) and the definitions of sources of flooding within the Flood and Water Management Act (FWMA) 2010.
- 1.4 This assessment has been prepared following a review of the East Hertfordshire District Council Strategic Flood Risk Assessment (SFRA) and a site visit.
- 1.5 The site is approximately centred on Ordnance Survey grid reference 536586, 218803. The site is approximately 7.75 ha and is currently undeveloped agricultural land.
- 1.6 The site is located in the north of High Cross, to the east of High Road. Residential/agricultural buildings are located to the west, south and north of the site. Undeveloped, agricultural land is located to the east and further to the north beyond the farm buildings.
- 1.7 The site survey (refer to Appendix A) shows that the majority of the site slopes south-eastwards, with the low point being midway along the southern boundary. Levels fall from approximately 93.0 m AOD at the boundary with the existing farm buildings to approximately 89.0 m AOD on the southern boundary. The strip of land which will provide the spine access route to High Road slopes broadly northwards, falling from approximately 91.0 m AOD (adjacent the existing farm buildings) to approximately 88.0 m AOD where it meets High Road.
- 1.8 Some 130 m to the north of the proposed site access (shown on the illustrative layout in Appendix B) the Barwick Tributary (a part ordinary watercourse and part main river) runs north-eastwards from High Road to (and beneath) the A10. The watercourse is shown in a plum colour on the extract from Appendix A of the JBA SFRA overleaf. The watercourse goes on to drain into the River Rib.



Extract from Appendix A of the 2016 SFRA showing watercourses © Crown copyright and database rights 2023 Ordnance Survey 100047325

- 1.9 British Geological Survey (BGS) mapping shows that the site is underlain by superficial deposits of the Lowestoft Formation (Diamicton) and bedrock geology of the Lewes Nodular Chalk Formation and Seaford Chalk Formation.
- 1.10 The site lies within Groundwater Source Protection Zone 3 (refer to Figure 3).

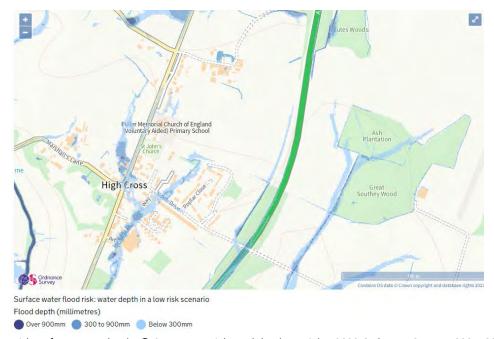
2.0 Forms of Flooding

Watercourses

2.1 The site lies in Flood Zone 1 (the low probability area) and is not therefore considered to be at risk of inundation from a tidal source, a named river, or other significantly sized watercourse (refer to Figure 2).

Surface Water

2.2 Surface water flood maps (refer to Figure 4 and the extract below) show that whilst the majority of the site is located in the very low flood risk area, there are localised areas of surface water flooding within the west and south of the site. These areas of medium and low risk are shown with a corresponding flood depth as below 300 mm and are therefore not considered to pose a significant or unmanageable risk to the development. The development of the site and the introduction of a formal surface water management strategy provides an opportunity to help reduce the predicted pooling along the southern boundary of the site with a view to reducing flows through the land to the south. This is discussed later in the surface water management section (Section 3).



 $Low\ risk\ surface\ water\ depths\ \textcircled{o}\ Crown\ copyright\ and\ database\ rights\ 2023\ Ordnance\ Survey\ 100047325$

Surface Water Sewers

2.3 Thames Water plans (provided at Appendix A) show there are no surface water sewers in the immediate vicinity of the site and therefore flooding from this source is not considered a notable threat at the site.

Groundwater

2.4 The BGS Infiltration GeoReport (refer to Appendix A) indicates that groundwater is likely to be more than 5 m below the ground surface throughout the year. The site investigation (Appendix A) recorded a potential perched water table (approximately 1.5m below ground level) in the western area of the site. In the unlikely event that groundwater levels were to rise to a point where they met ground levels then the resulting flows would tend to follow the topography and exit the site as overland flow (as opposed to pooling within the site boundaries).

Reservoirs/Lakes/Canals

2.5 The site is not shown to lie within an area denoted as being at risk of flooding from a breach (failure) of a reservoir (refer to Figure 2). There are no lakes or canals in the vicinity of the site.

3.0 Surface Water Management

- 3.1 The topography of the site dictates that runoff from the majority of the site will flow south (as suggested by the flow routes and pooling shown on the surface water flood mapping) with the land on which the spine access road will be constructed draining broadly westwards.
- 3.2 Site infiltration testing (included in Appendix A) shows that conditions are not suitable to support the disposal of surface water runoff via infiltration. It is therefore proposed to discharge surface water runoff to the Barwick Tributary, the watercourse to the north within the landowners control. Although this represents a local redistribution of flows from the site which would ordinarily run overland southwards, the flows are simply entering the same watercourse (the River Rib) further upstream than they would currently do. This is demonstrated on the extract of Appendix A of the SFRA (showing the watercourse locations and connectivity).



Annotated extract from Appendix A of the 2016 SFRA showing the route of the River Rib relative to the site © Crown copyright and database rights 2023 Ordnance Survey 100047325

- 3.3 Sufficient on-site surface water storage will be provided in order to manage the 1 in 100 annual probability storm including the requisite 40 % allowance for climate change. The discharge rate will be limited to the mean annual greenfield rate of circa 3.0 l/s/ha.
- 3.4 It is proposed to attenuate flows in a one of three attenuation basins (refer to the surface water strategy drawing in Appendix B). It is proposed that sub-catchment A, C, and D will drain via swales, filter drains (or other roadside SuDS features) to the attenuation basin on the southern boundary. Sub-catchment B will drain to the smaller basin just to the north of the sub-catchment. The access road will drain (again via swales, roadside planters etc) to the basin just south of the proposed junction with High Road.

- 3.5 The greenfield discharge from the basins will be conveyed northwards to the receiving watercourse (discussed in paragraph 3.2, and shown on the surface water management strategy in Appendix B) with a shallow gradient pipe network. Total flow in the network would be minimal (less than circa 7.0 l/s).
- 3.6 Each of the three flow control structures will be suitably protected by debris filters at outlets from the basins, and in-chamber filters (perforated risers/boxes etc).
- 3.7 Causeway flow calculations showing the performance of each of the basins for the 1 in 2, 1 in 30, and 1 in 100 annual probability events are included in Appendix B. An urban creep rate of 5 % has been applied to the total proposed impermeable catchment area for the residential parcels to simulate a 10 % increase in roof/plot area.
- 3.8 Exceedance flows will be managed via overland routing along the roads and swales to open space and/or one of the attenuation basins.
- 3.9 All proposals and rates are subject to detailed design and the approval of relevant parties.

Treatment

- 3.10 The maximum pollution hazard level for the proposals can be considered as medium based on the assumption of the associated traffic movements for the development road are likely to be greater than 300 traffic movements per day (i.e. will likely fall outside of the low pollution hazard designation). CIRIA C753 (Table 26.2) presents an associated pollution score for medium pollution hazard of 0.7 for total suspended solids (TSS), 0.6 for metals (M), and 0.7 for hydrocarbons (HC).
- 3.11 The attenuation basins provide sufficient treatment for the lower trafficked roads and roof areas (i.e. both with a corresponding low pollution hazard designation). It is currently proposed that treatment and conveyance of runoff from the busier roads (those serving more than 50 properties) will be via roadside SuDS features (swales, filter drains, linked SuDS planters etc). These would however be determined by the later stage detailed layout and any restrictions re adoption imposed by third parties (Highways, Thames Water etc).
- 3.1 The combination of proposed roadside SuDS (analogous to bio-retention) for the busier roads and the proposed basins would provide a suitable level of treatment for the development runoff.

Treatment scores

TSS – swale 0.5 + basin (0.5/2) = 0.75Metals – swale 0.6 + basin (0.5/2) = 0.85Hydrocrabons – swale 0.6 + basin (0.6/2) = 0.9

Maintenance

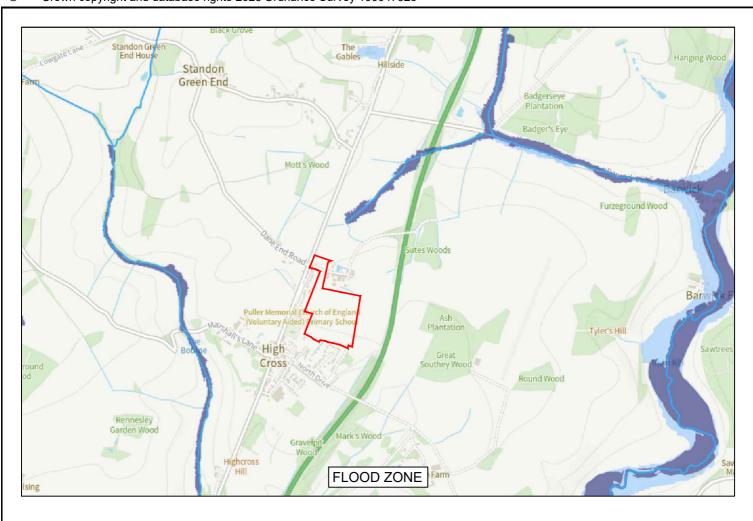
3.12 Maintenance responsibilities would be established at the usual post planning stage. Suggested maintenance activities for are included on the maintenance plan in Appendix B.

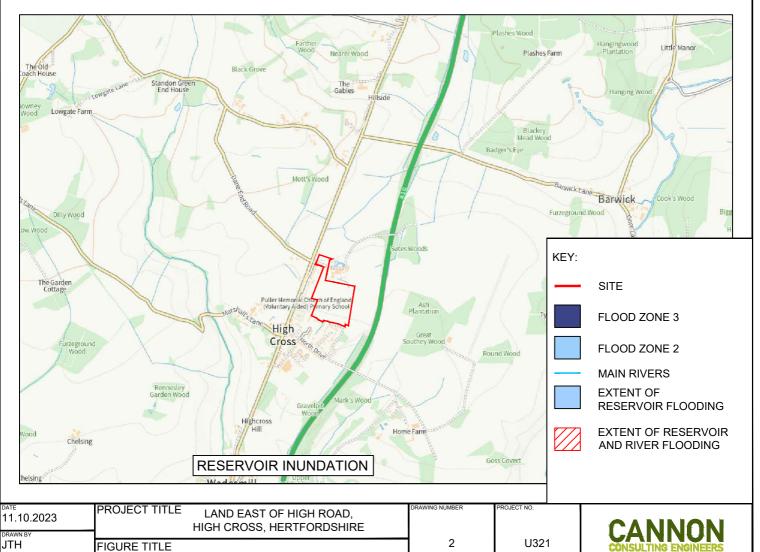
4.0 Conclusions

- 4.1 The site is not considered to be liable to significant or unmanageable flooding from the sources identified in the Flood and Water Management Act 2010 (FWMA).
- 4.2 Surface water runoff from the development will be discharged at greenfield rates to the watercourse located to the north of the site (within the same land ownership as the site). Onsite attenuation will be provided in order to manage flows up to and including the 1 in 100 annual probability storm event inclusive of 40 % climate change allowance.
- 4.3 The proposed outline surface water management strategy allows for sufficient space within the layout to be set aside for water management. The strategy also allows for sufficient treatment of runoff prior to it leaving the proposed development.
- 4.4 It is envisaged that maintenance of the scheme will be undertaken by a private management company with some elements potentially adopted by Thames Water.

Figures

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D.N.S.

FLOOD ZONE & RESERVOIR INUNDATION PLAN

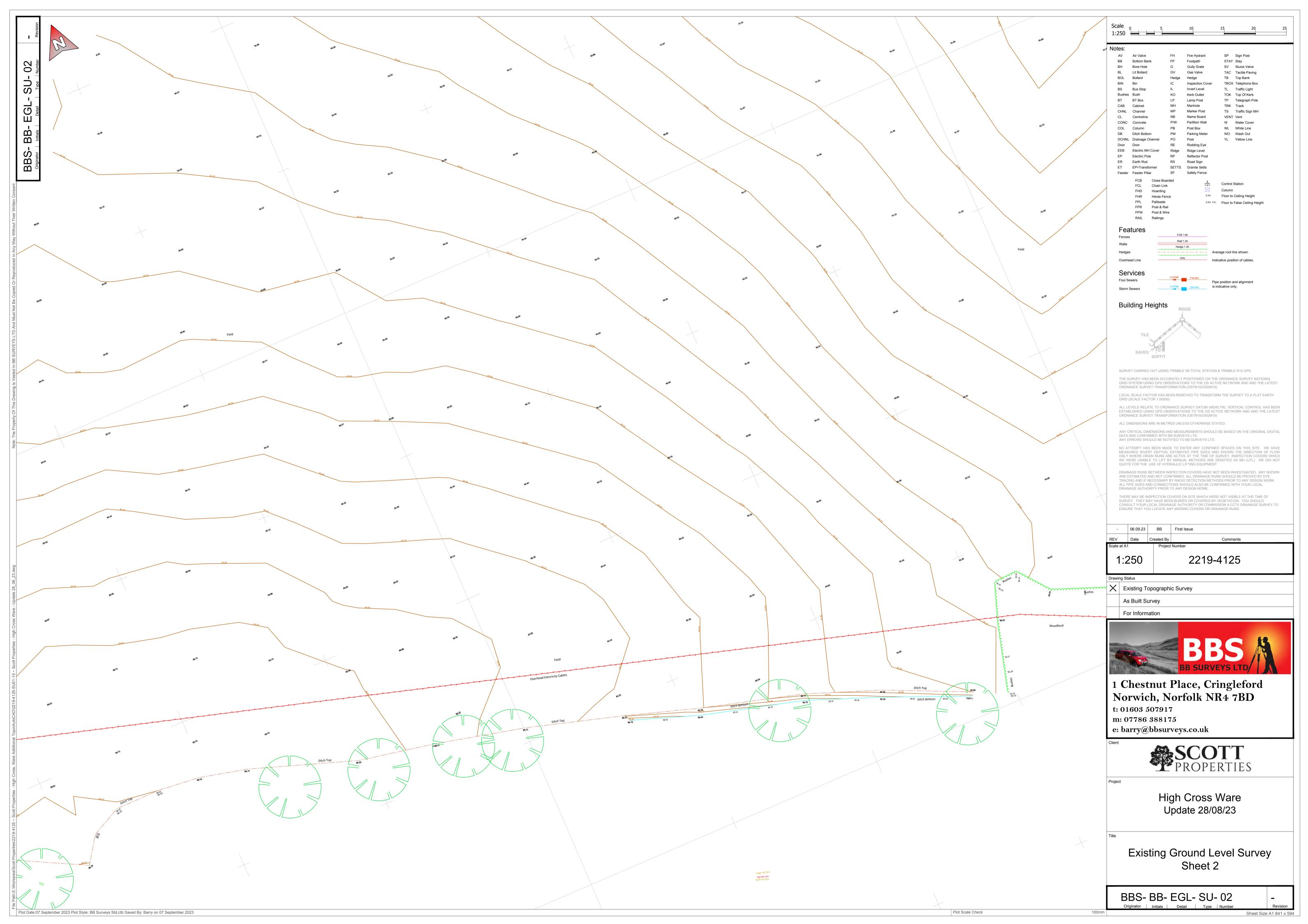
OUTER ZONE Gravelpit Wo TOTAL CATCHMENT Highcross Hill SPECIAL INTEREST Home DATE 11.10.2023 PROJECT TITLE FIGURE NUMBER PROJECTNO. LAND EAST OF HIGH ROAD, HIGH CROSS, HERTFORDSHIRE DRAWN BY 3 U321 JTH FIGURE TITLE SCALE @ A4 SIZE D.N.S. **GROUNDWATER SOURCE PROTECTION ZONES PLAN** \cdcsvr\Data\U321 Sutes Farm High Cross, HERTS\DRAWINGS\AUTOCAD\CURRENT DRGS\FLOOD MAPS\SITE LOCATION AND GWSPZ

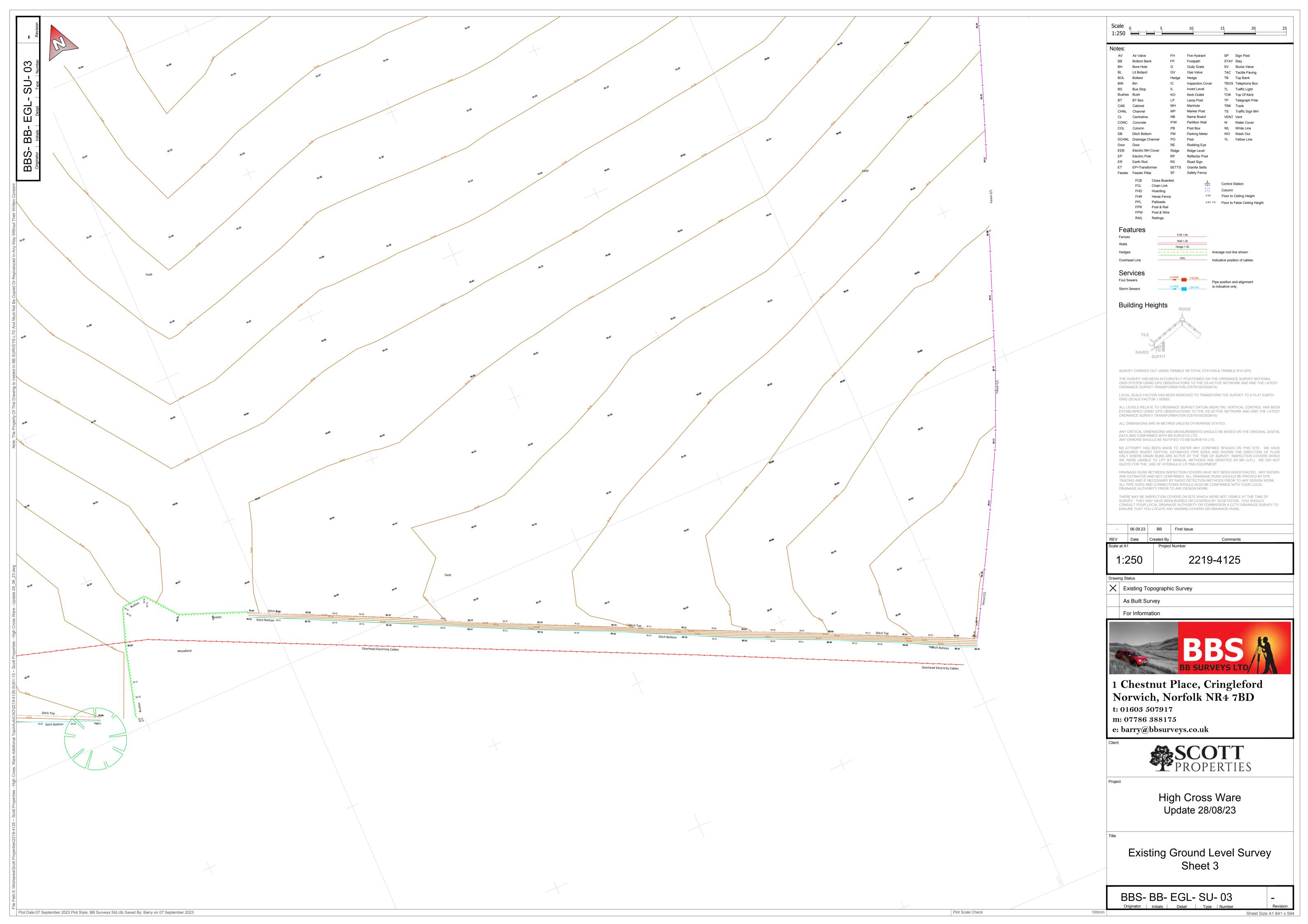
© Crown copyright and database rights 2023 Ordnance Survey 100047325 The Old Coach House Standon G End Hou Lowgate Farm Barwick Furzeground Wood Barwi High Cross Round Wood Hill Home Farm Goss Covert Chelsing Cottages utes Woods Puller Memorial Church of England Voluntary Aided) Primary School Ash KEY: SITE High Cross HIGH **MEDIUM** LOW **VERY LOW** PROJECT TITLE LAND EAST OF HIGH ROAD, 11.10.2023 HIGH CROSS, HERTFORDSHIRE U321 JTH FIGURE TITLE SURFACE WATER FLOOD MAP SCALE @ A4 D.N.S.

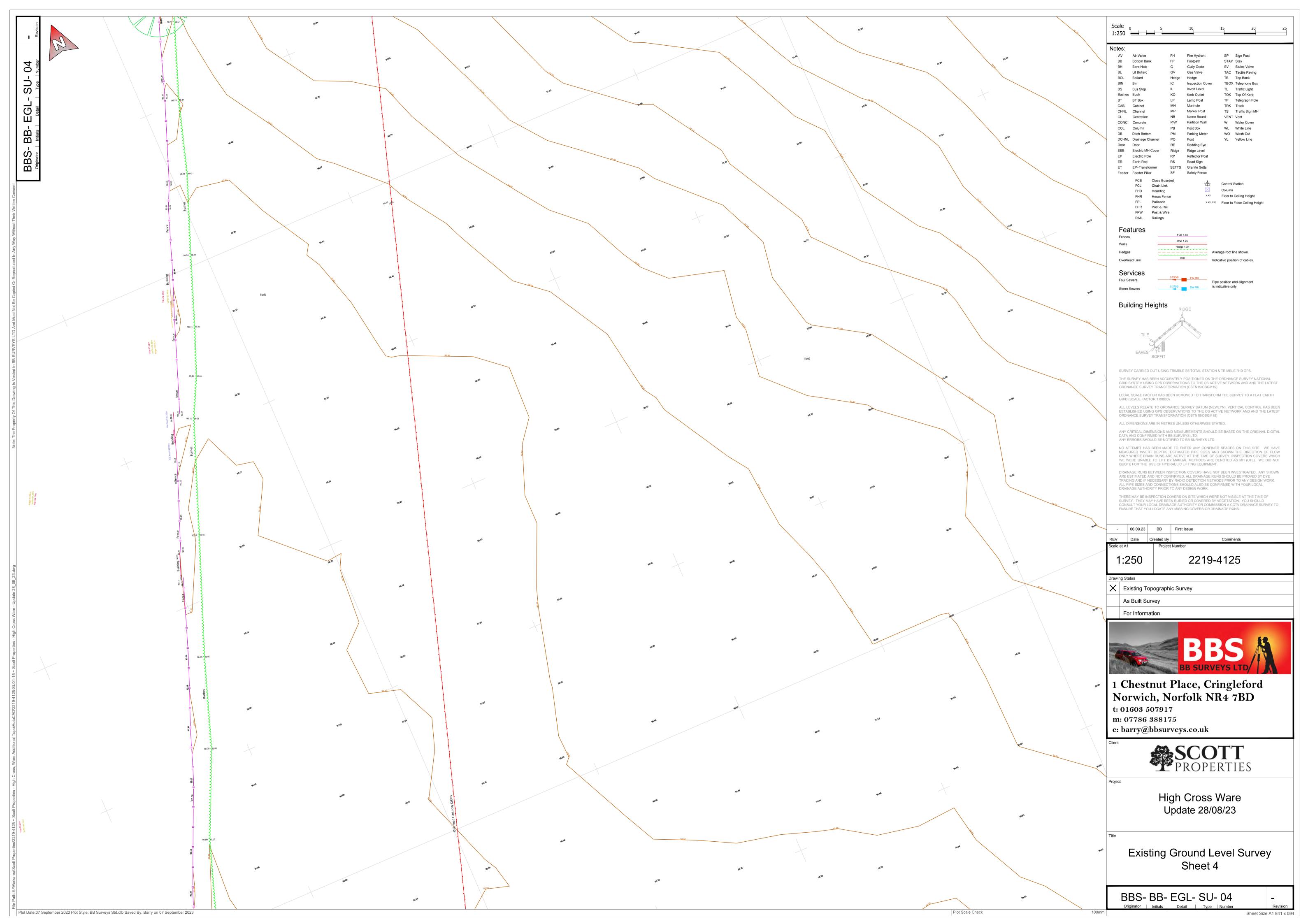
Appendix A – Existing Site
Topographical survey
Thames Water asset plans
BGS Infiltration GeoReport
Site Infiltration testing report

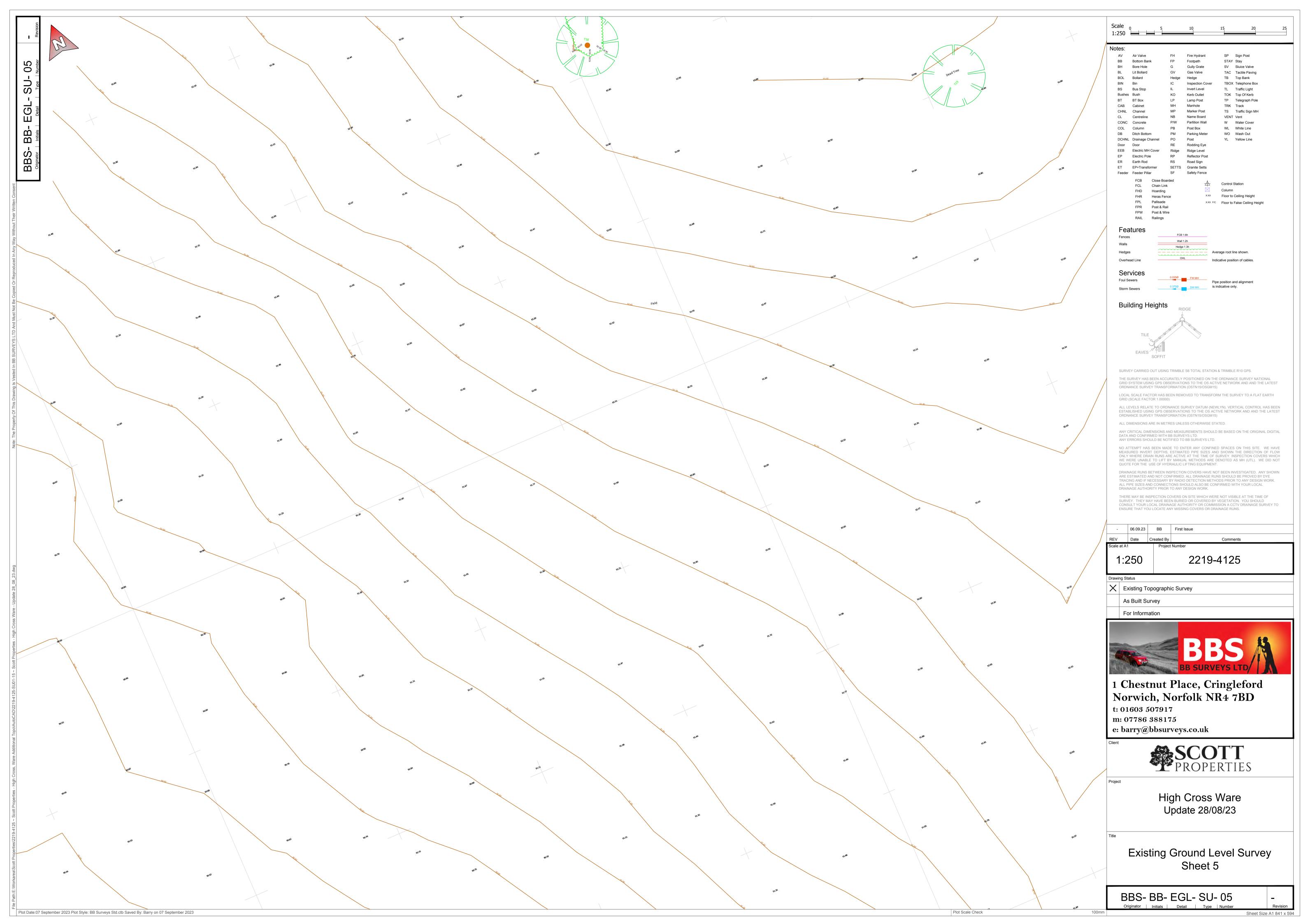


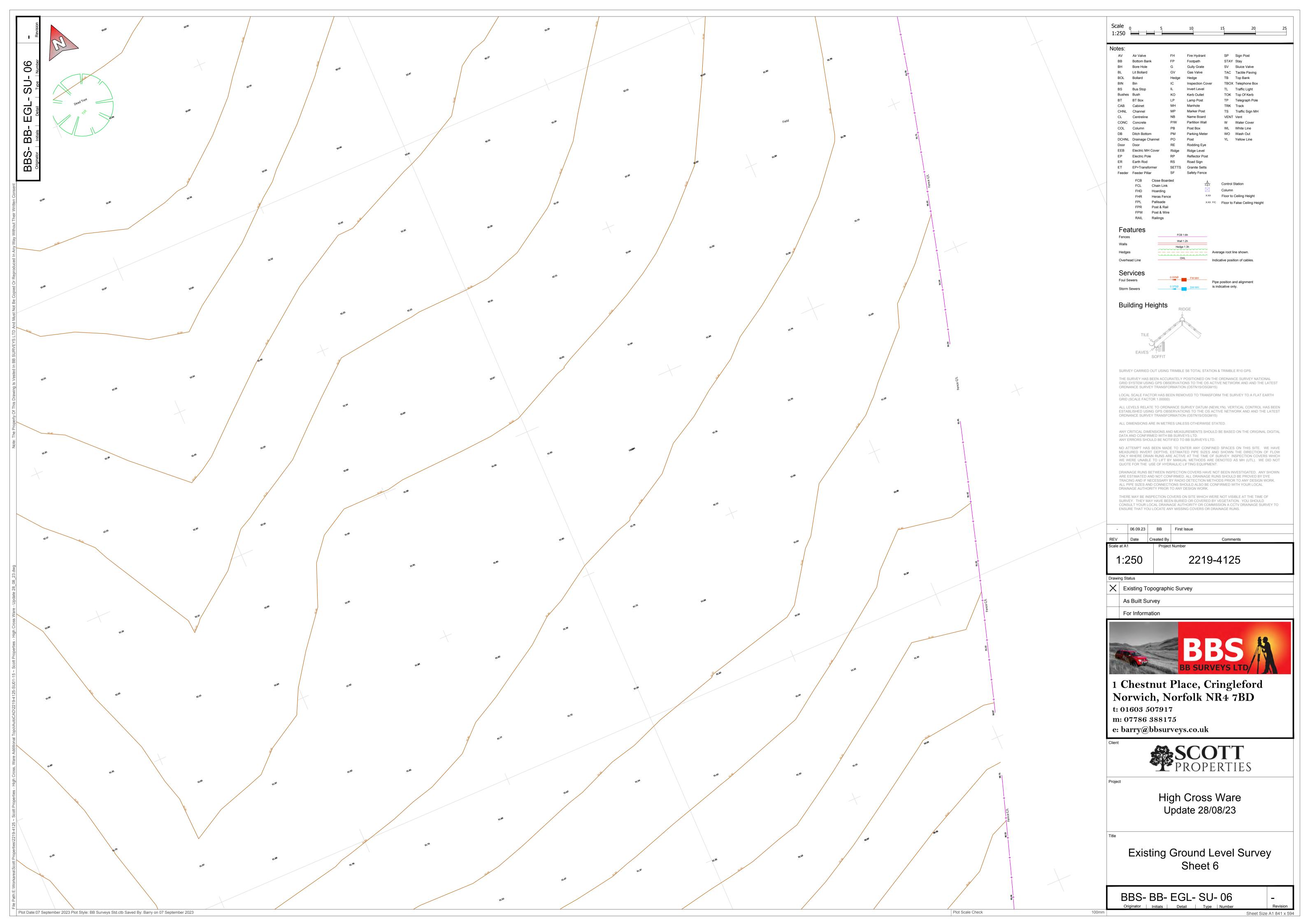






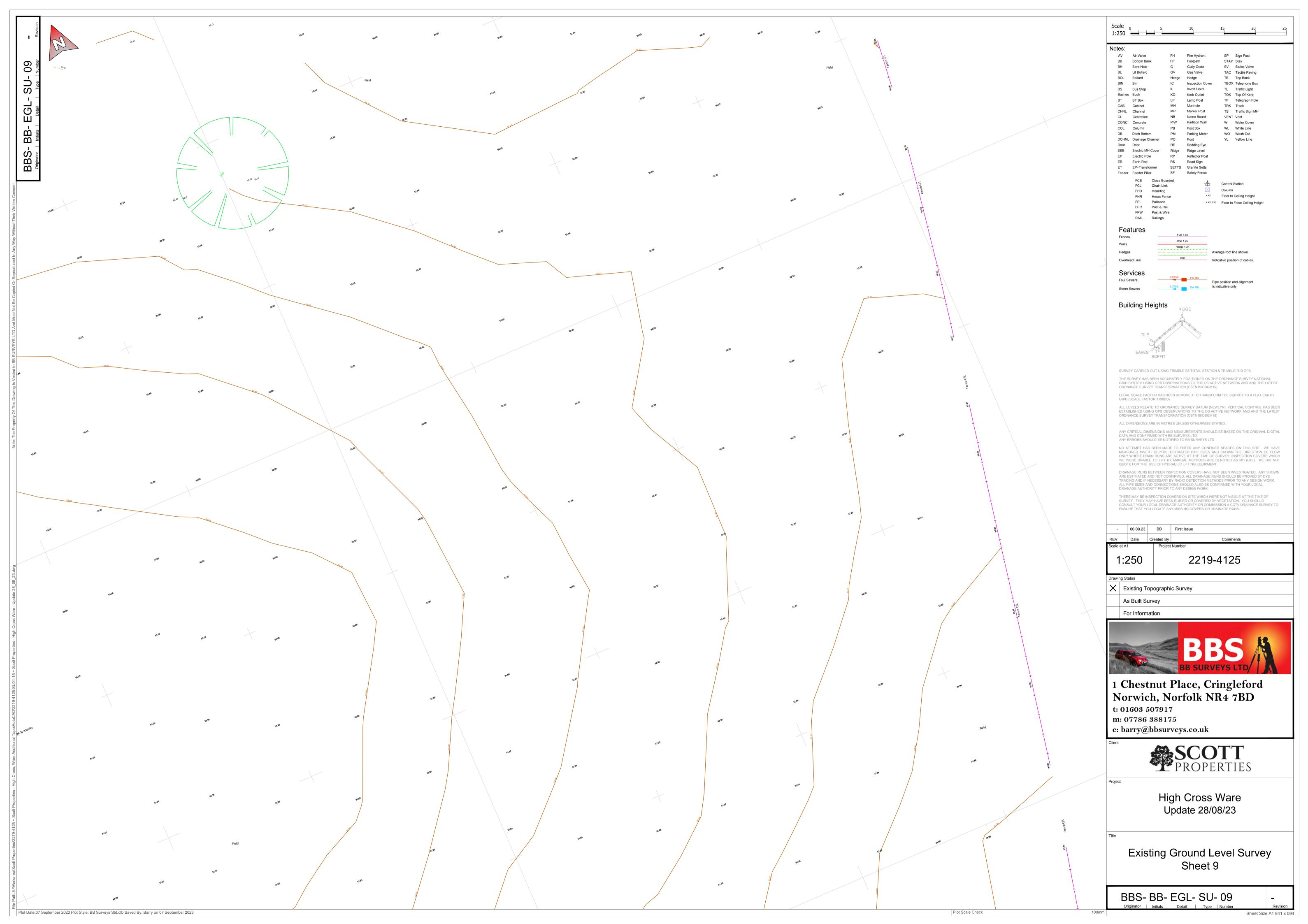


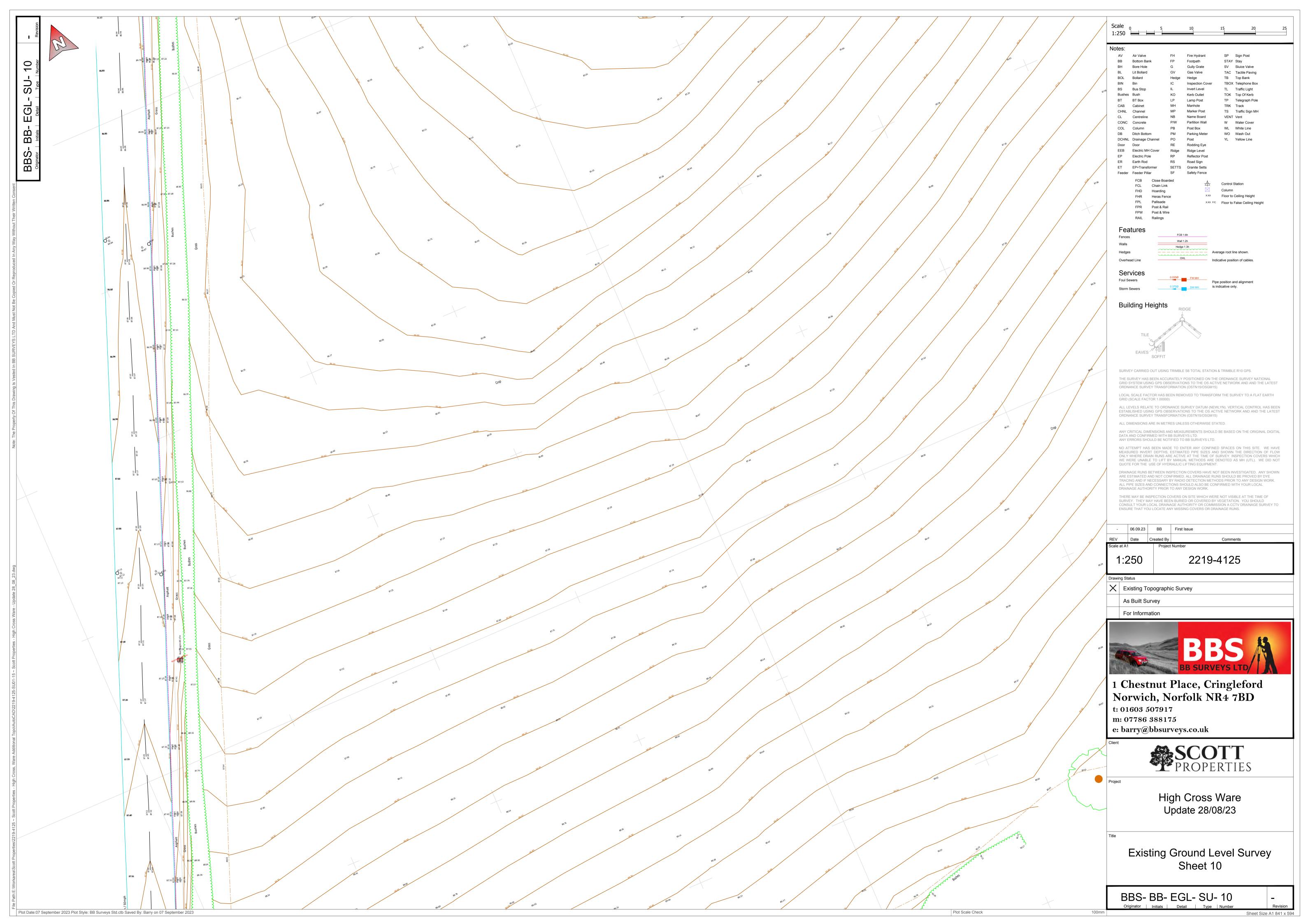


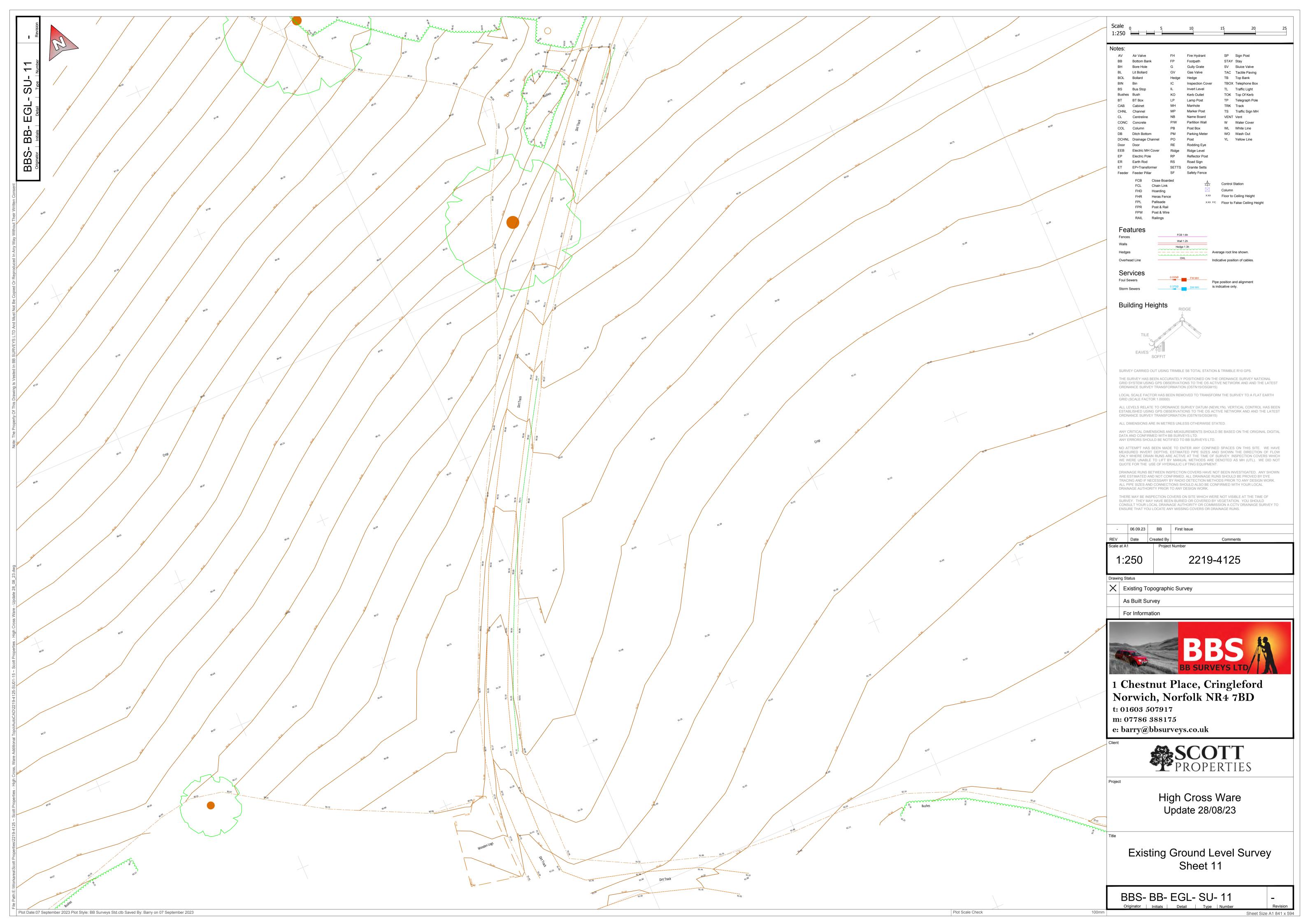


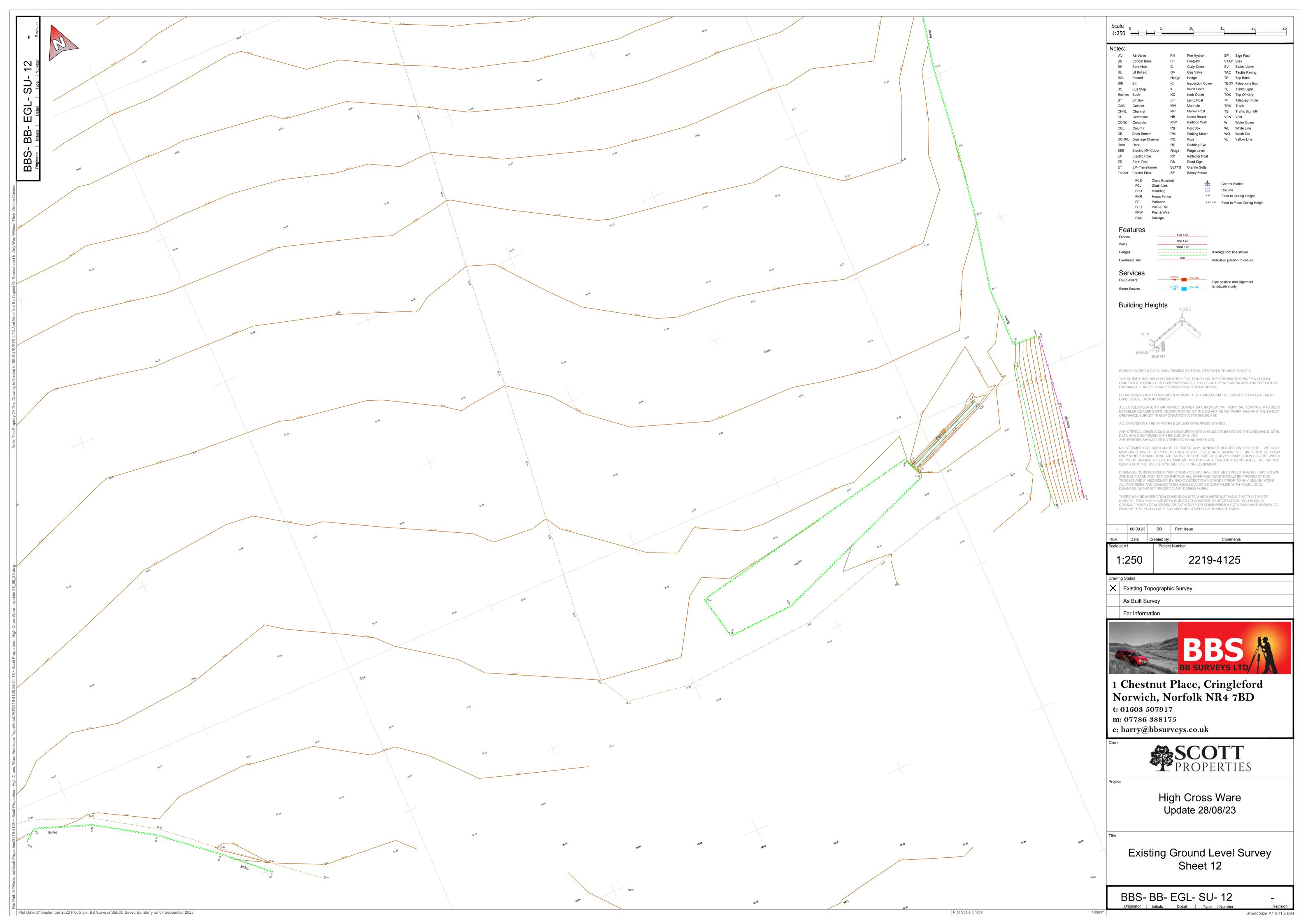


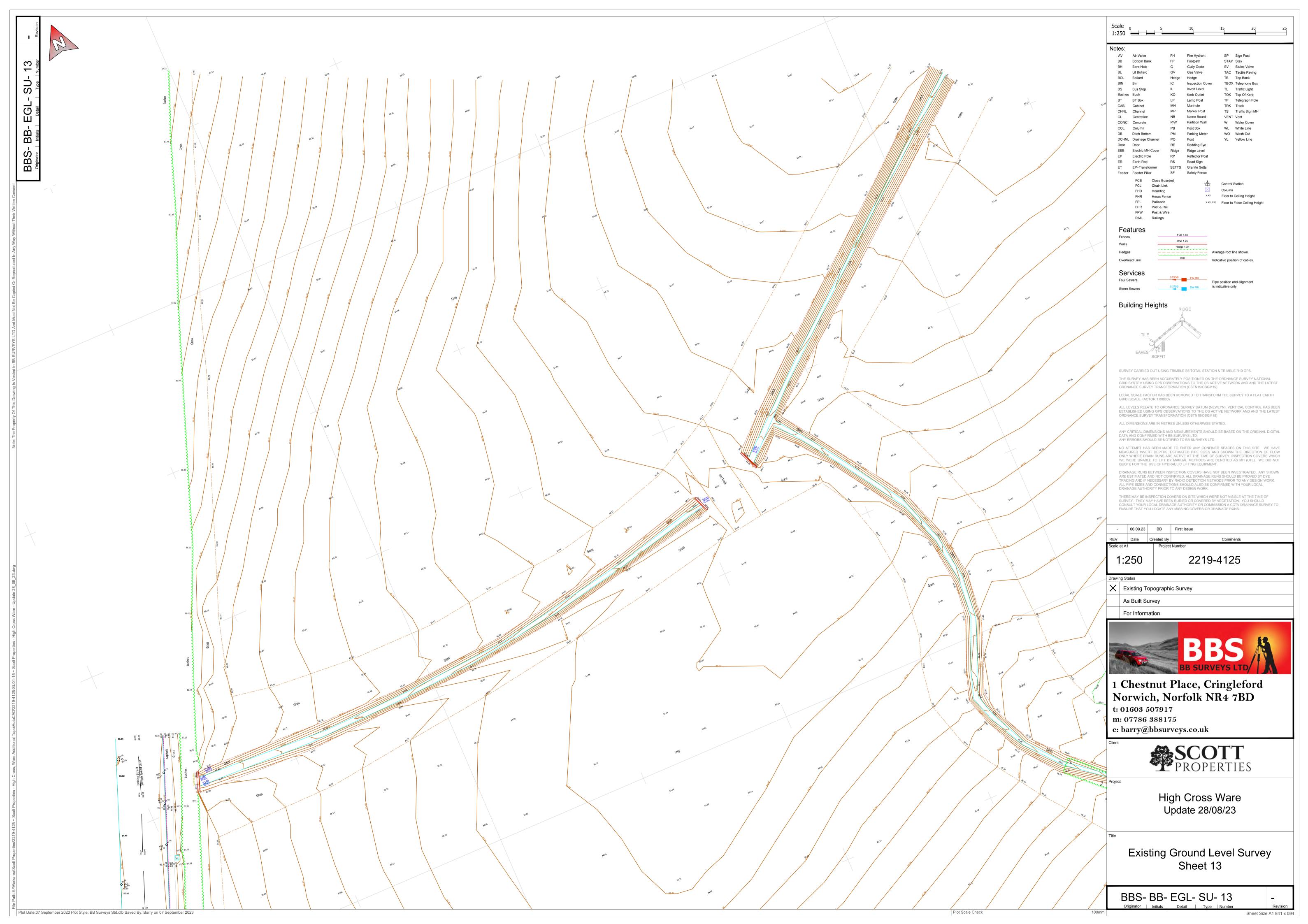


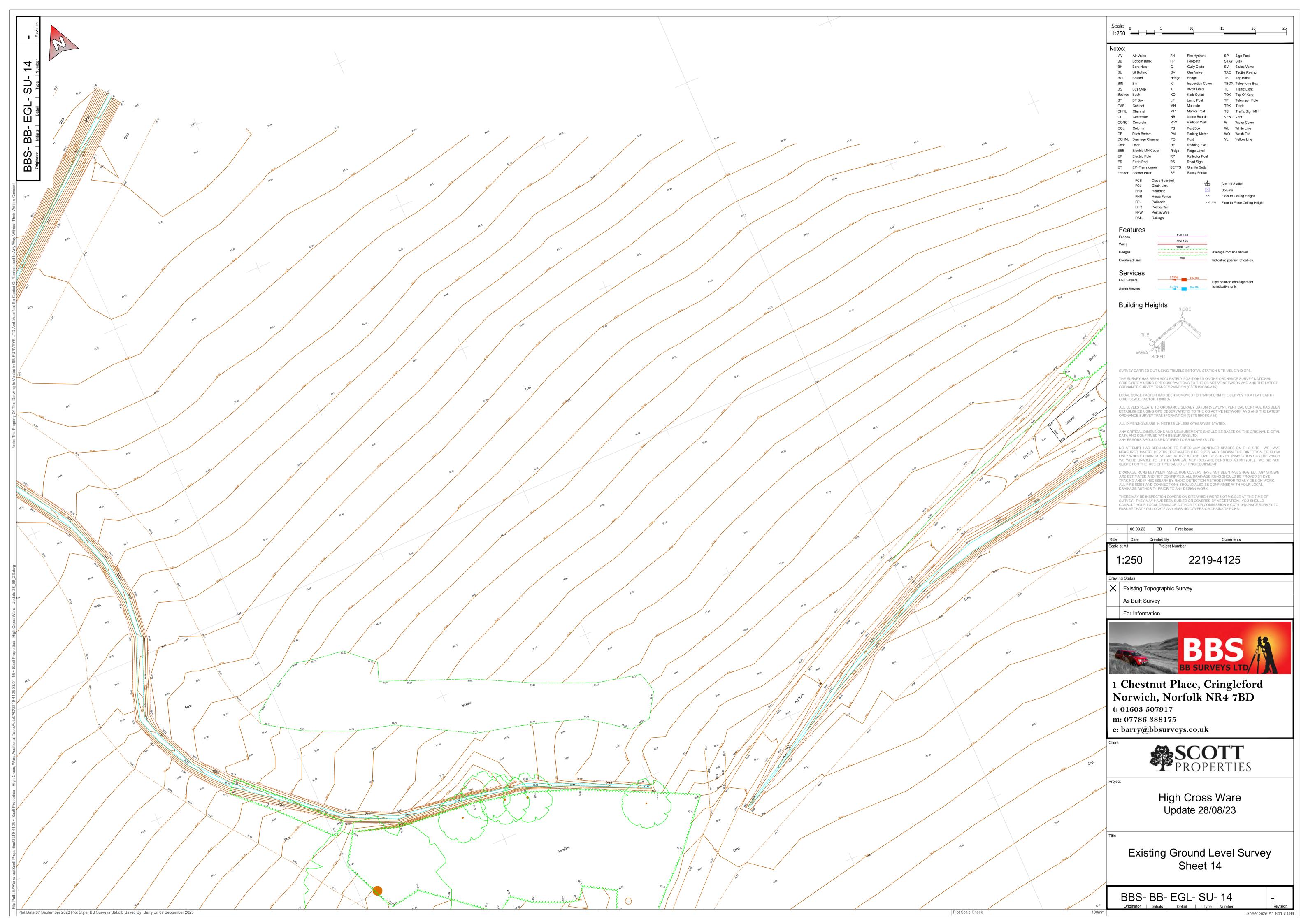


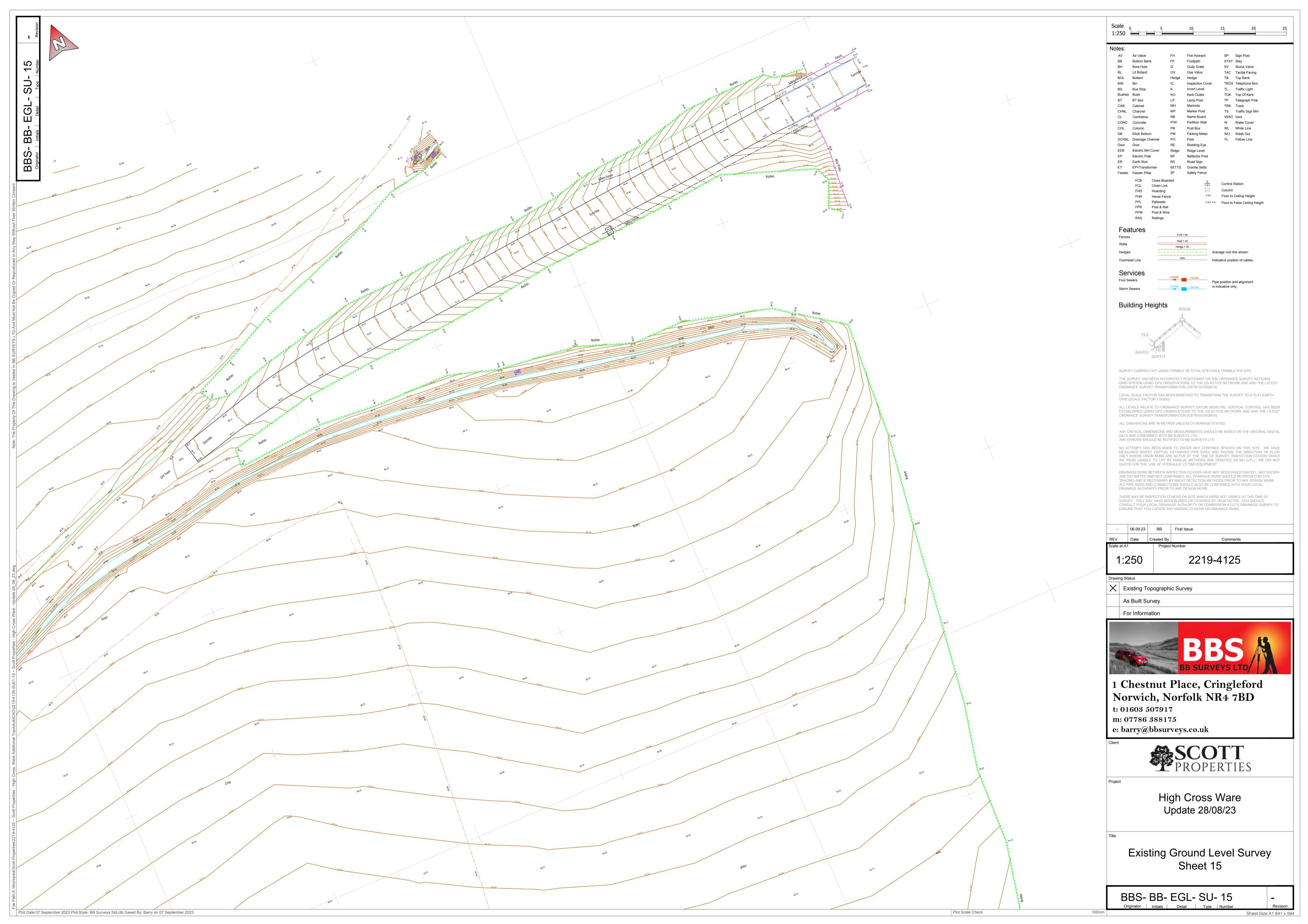












Asset location search



Cannon Consulting Engineers Cambridge House Lanwades Business Park NEWMARKET CB8 7PN

Search address supplied High Road

Ware

Hertfordshire SG11 1BE

Your reference JA-M/U321/TW2018.01.18

Our reference ALS/ALS Standard/2018_3723604

Search date 18 January 2018

Keeping you up-to-date

Knowledge of features below the surface is essential in every development. The benefits of this not only include ensuring due diligence and avoiding risk, but also being able to ascertain the feasibility for any commercial or residential project.

An asset location search provides information on the location of known Thames Water clean and/or wastewater assets, including details of pipe sizes, direction of flow and depth. Please note that information on cover and invert levels will only be provided where the data is available.



Thames Water Utilities Ltd Property Searches, PO Box 3189, Slough SL1 4WW DX 151280 Slough 13



searches@thameswater.co.uk www.thameswater-propertysearches.co.uk





Asset location search



Search address supplied: High Road, Ware, Hertfordshire, SG11 1BE

Dear Sir / Madam

An Asset Location Search is recommended when undertaking a site development. It is essential to obtain information on the size and location of clean water and sewerage assets to safeguard against expensive damage and allow cost-effective service design.

The following records were searched in compiling this report: - the map of public sewers & the map of waterworks. Thames Water Utilities Ltd (TWUL) holds all of these.

This searchprovides maps showing the position, size of Thames Water assets close to the proposed development and also manhole cover and invert levels, where available.

Please note that none of the charges made for this report relate to the provision of Ordnance Survey mapping information. The replies contained in this letter are given following inspection of the public service records available to this company. No responsibility can be accepted for any error or omission in the replies.

You should be aware that the information contained on these plans is current only on the day that the plans are issued. The plans should only be used for the duration of the work that is being carried out at the present time. Under no circumstances should this data be copied or transmitted to parties other than those for whom the current work is being carried out.

Thames Water do update these service plans on a regular basis and failure to observe the above conditions could lead to damage arising to new or diverted services at a later date.

Contact Us

If you have any further queries regarding this enquiry please feel free to contact a member of the team on 0845 070 9148, or use the address below:

Thames Water Utilities Ltd Property Searches PO Box 3189 Slough SL1 4WW

Email: searches@thameswater.co.uk

Web: www.thameswater-propertysearches.co.uk

Asset location search



Waste Water Services

Please provide a copy extract from the public sewer map.

The following quartiles have been printed as they fall within Thames' sewerage area:

TL3618NE TL3618NW

Enclosed is a map showing the approximate lines of our sewers. Our plans do not show sewer connections from individual properties or any sewers not owned by Thames Water unless specifically annotated otherwise. Records such as "private" pipework are in some cases available from the Building Control Department of the relevant Local Authority.

Where the Local Authority does not hold such plans it might be advisable to consult the property deeds for the site or contact neighbouring landowners.

This report relates only to sewerage apparatus of Thames Water Utilities Ltd, it does not disclose details of cables and or communications equipment that may be running through or around such apparatus.

The sewer level information contained in this response represents all of the level data available in our existing records. Should you require any further Information, please refer to the relevant section within the 'Further Contacts' page found later in this document.

The following quartiles have not been printed as they contain no assets:

TL3619SE TL3619SW

For your guidance:

- The Company is not generally responsible for rivers, watercourses, ponds, culverts
 or highway drains. If any of these are shown on the copy extract they are shown for
 information only.
- Any private sewers or lateral drains which are indicated on the extract of the public sewer map as being subject to an agreement under Section 104 of the Water Industry Act 1991 are not an 'as constructed' record. It is recommended these details be checked with the developer.

Clean Water Services

Please provide a copy extract from the public water main map.

Following examination of our statutory maps, Thames Water has been unable to find

Asset location search



any plans of water mains within this area. If you require a connection to the public water supply system, please write to:

New Connections / Diversions Thames Water Network Services Business Centre Brentford Middlesex TW8 0EE

Tel: 0845 850 2777

Fax: 0207 713 3858

Email: developer.services@thameswater.co.uk

The following quartiles have not been printed as they are out of Thames' water catchment area. For details of the assets requested please contact the water company indicated below:

TL3618NE Affinity Water
TL3618NW Affinity Water
TL3619SE Affinity Water
TL3619SW Affinity Water

Affinity Water Ltd Tamblin Way Hatfield AL10 9EZ

Tel: 0845 7823333

For your guidance:

- Assets other than vested water mains may be shown on the plan, for information only.
- If an extract of the public water main record is enclosed, this will show known public
 water mains in the vicinity of the property. It should be possible to estimate the
 likely length and route of any private water supply pipe connecting the property to
 the public water network.

Payment for this Search

A charge will be added to your suppliers account.

Asset location search



Further contacts:

Waste Water queries

Should you require verification of the invert levels of public sewers, by site measurement, you will need to approach the relevant Thames Water Area Network Office for permission to lift the appropriate covers. This permission will usually involve you completing a TWOSA form. For further information please contact our Customer Centre on Tel: 0845 920 0800. Alternatively, a survey can be arranged, for a fee, through our Customer Centre on the above number.

If you have any questions regarding sewer connections, budget estimates, diversions, building over issues or any other questions regarding operational issues please direct them to our service desk. Which can be contacted by writing to:

Developer Services (Waste Water) Thames Water Clearwater Court Vastern Road Reading RG1 8DB

Tel: 0800 009 3921

Email: developer.services@thameswater.co.uk

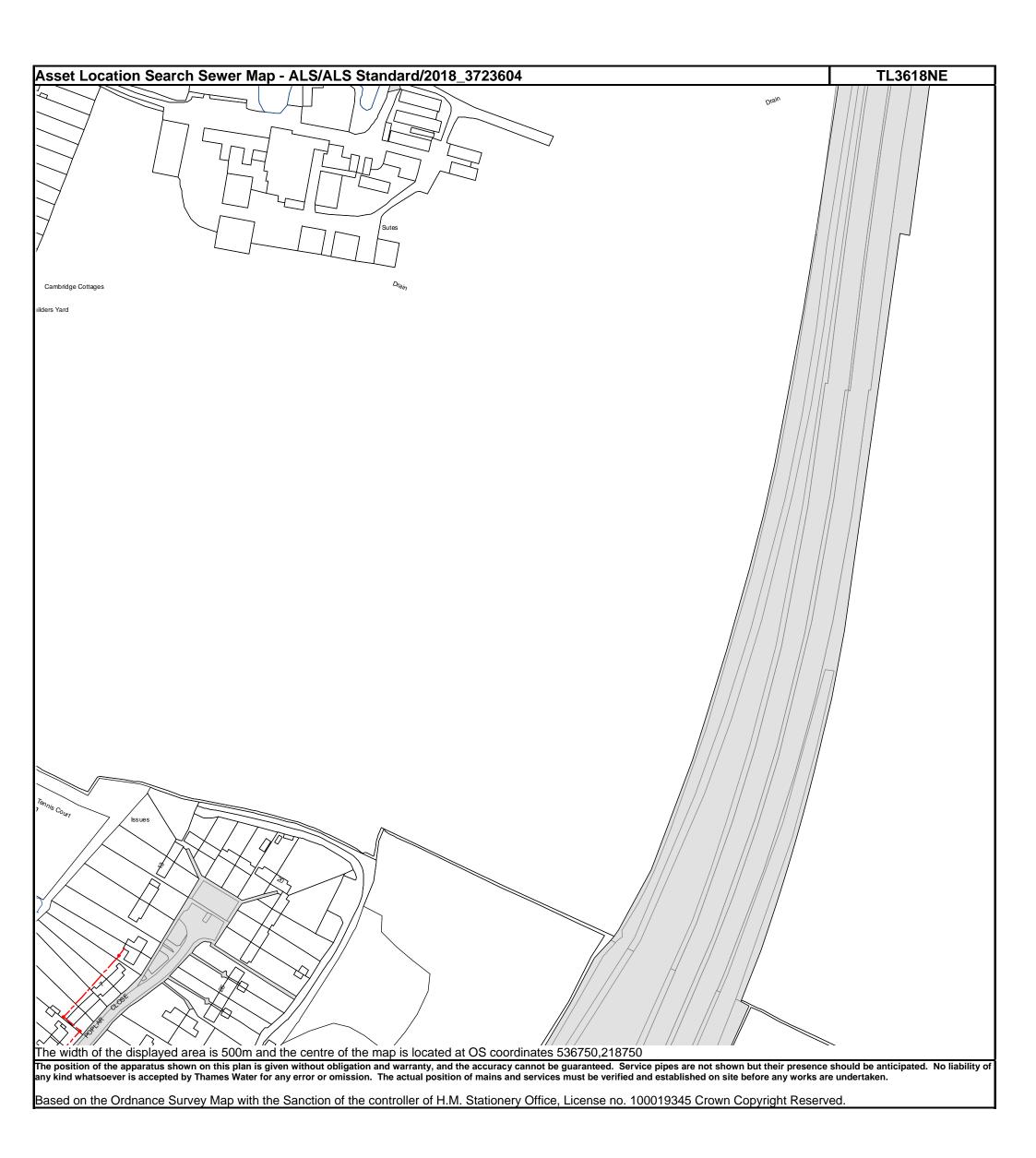
Clean Water queries

Should you require any advice concerning clean water operational issues or clean water connections, please contact:

Developer Services (Clean Water) Thames Water Clearwater Court Vastern Road Reading RG1 8DB

Tel: 0800 009 3921

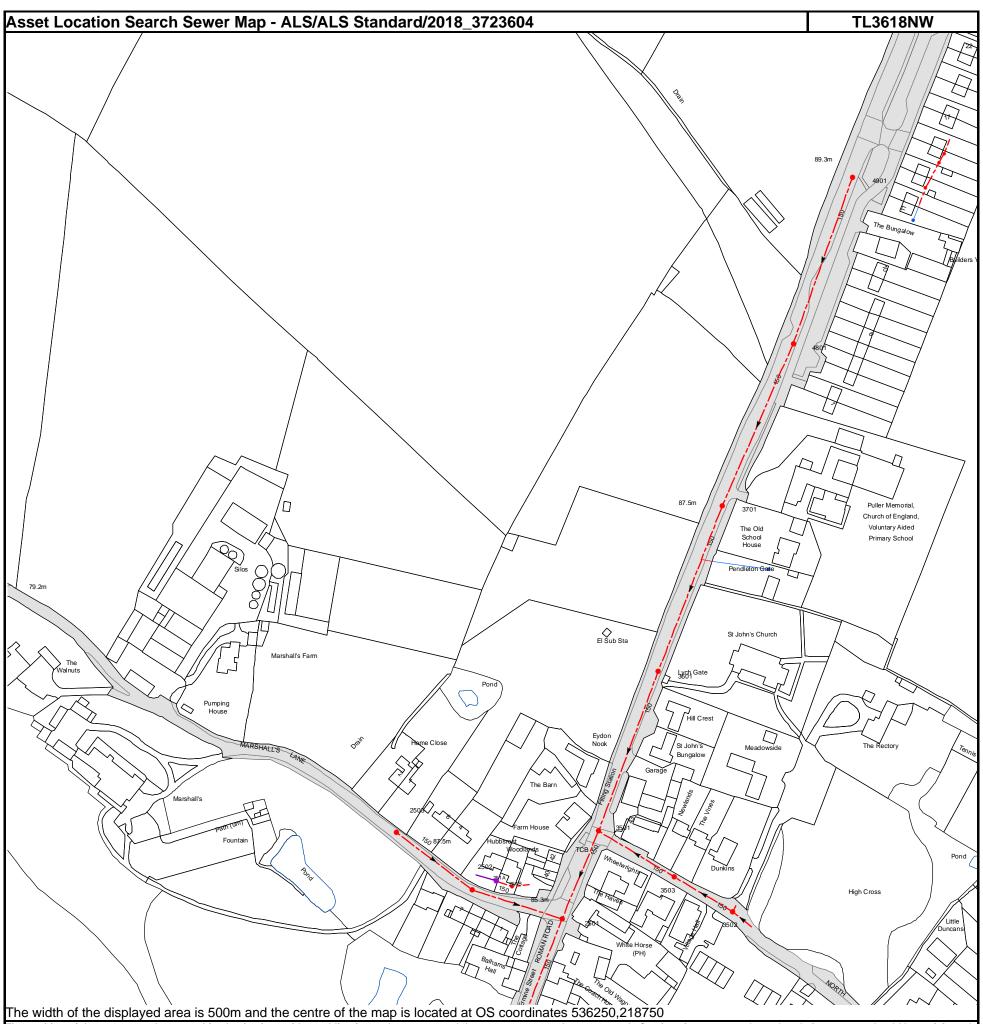
Email: developer.services@thameswater.co.uk



<u>Thames Water Utilities Ltd</u>, Property Searches, PO Box 3189, Slough SL1 4W, DX 151280 Slough 13 **T** 0845 070 9148 **E** <u>searches@thameswater.co.uk</u> **I** <u>www.thameswater-propertysearches.co.uk</u>

Manhole Reference	Manhole Cover Level	Manhole Invert Level
551C	n/a	n/a
551B	n/a	n/a
551A	n/a	n/a

The position of the apparatus shown on this plan is given without obligation and warranty, and the accuracy cannot be guaranteed. Service pipes are not shown but their presence should be anticipated. No liability of any kind whatsoever is accepted by Thames Water for any error or omission. The actual position of mains and services must be verified and established on site before any works are undertaken.



The position of the apparatus shown on this plan is given without obligation and warranty, and the accuracy cannot be guaranteed. Service pipes are not shown but their presence should be anticipated. No liability of any kind whatsoever is accepted by Thames Water for any error or omission. The actual position of mains and services must be verified and established on site before any works are undertaken.

Based on the Ordnance Survey Map with the Sanction of the controller of H.M. Stationery Office, License no. 100019345 Crown Copyright Reserved.

Manhole Reference	Manhole Cover Level	Manhole Invert Level
371A	n/a	n/a
491A	n/a	n/a
491B	n/a	n/a
491C	n/a	n/a
491D	n/a	n/a
2501	85.663	82.463
3502	86.417	83.857
2502	86.68	83.23
251B	n/a	n/a
251A	n/a	n/a
3503	85.934	83.534
2503	87.59	83.54
3501	85.72	82.7
3601	86.59	n/a
3701	87.76	n/a
4801	88.8	n/a
4901	89.31	86.24

The position of the apparatus shown on this plan is given without obligation and warranty, and the accuracy cannot be guaranteed. Service pipes are not shown but their presence should be anticipated. No liability of any kind whatsoever is accepted by Thames Water for any error or omission. The actual position of mains and services must be verified and established on site before any works are undertaken.



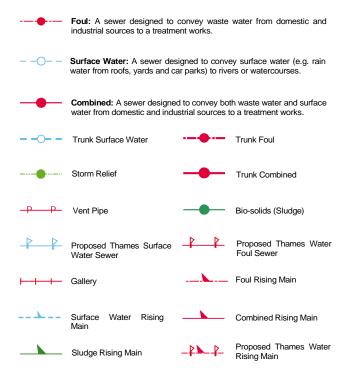
The position of the apparatus shown on this plan is given without obligation and warranty, and the accuracy cannot be guaranteed. Service pipes are not shown but their presence should be anticipated. No liability of any kind whatsoever is accepted by Thames Water for any error or omission. The actual position of mains and services must be verified before any works are undertaken. Crown copyright Reserved

Scale:	1:7159
Width:	2000m
Printed By:	Vkumar1
Print Date:	18/01/2018
Map Centre:	536590,218808
Grid Reference:	TL3618NE

Col	mm	en	ıts:



Public Sewer Types (Operated & Maintained by Thames Water)



Sewer Fittings

A feature in a sewer that does not affect the flow in the pipe. Example: a vent is a fitting as the function of a vent is to release excess gas.

Air Valve

Dam Chase

Fitting

Meter

♦ Vent Column

Operational Controls

A feature in a sewer that changes or diverts the flow in the sewer. Example: A hydrobrake limits the flow passing downstream.

Control Valve

Drop Pipe

Ancillary

✓ Weir

End Items

End symbols appear at the start or end of a sewer pipe. Examples: an Undefined End at the start of a sewer indicates that Thames Water has no knowledge of the position of the sewer upstream of that symbol, Outfall on a surface water sewer indicates that the pipe discharges into a stream or river.

Outfall

Undefined End

/ Inle

Notes:

----- Vacuum

- 1) All levels associated with the plans are to Ordnance Datum Newlyn.
- 2) All measurements on the plans are metric.
- Arrows (on gravity fed sewers) or flecks (on rising mains) indicate direction of flow.
- Most private pipes are not shown on our plans, as in the past, this information has not been recorded.
- 5) 'na' or '0' on a manhole level indicates that data is unavailable.

6) The text appearing alongside a sewer line indicates the internal diameter of the pipe in milimetres. Text next to a manhole indicates the manhole reference number and should not be taken as a measurement. If you are unsure about any text or symbology present on the plan, please contact a member of Property Insight on 0845 070 9148.

Other Symbols

Symbols used on maps which do not fall under other general categories

▲ / ▲ Public/Private Pumping Station

* Change of characteristic indicator (C.O.C.I.)

M Invert Level

<1 Summit

Areas

Lines denoting areas of underground surveys, etc.

Agreement

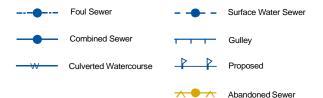
Operational Site

:::::: Chamber

Tunnel

Conduit Bridge

Other Sewer Types (Not Operated or Maintained by Thames Water)



Terms and Conditions

All sales are made in accordance with Thames Water Utilities Limited (TWUL) standard terms and conditions unless previously agreed in writing.

- 1. All goods remain in the property of Thames Water Utilities Ltd until full payment is received.
- 2. Provision of service will be in accordance with all legal requirements and published TWUL policies.
- 3. All invoices are strictly due for payment 14 days from due date of the invoice. Any other terms must be accepted/agreed in writing prior to provision of goods or service, or will be held to be invalid.
- 4. Thames Water does not accept post-dated cheques-any cheques received will be processed for payment on date of receipt.
- 5. In case of dispute TWUL's terms and conditions shall apply.
- 6. Penalty interest may be invoked by TWUL in the event of unjustifiable payment delay. Interest charges will be in line with UK Statute Law 'The Late Payment of Commercial Debts (Interest) Act 1998'.
- 7. Interest will be charged in line with current Court Interest Charges, if legal action is taken.
- 8. A charge may be made at the discretion of the company for increased administration costs.

A copy of Thames Water's standard terms and conditions are available from the Commercial Billing Team (cashoperations@thameswater.co.uk).

We publish several Codes of Practice including a guaranteed standards scheme. You can obtain copies of these leaflets by calling us on 0800 316 9800

If you are unhappy with our service you can speak to your original goods or customer service provider. If you are not satisfied with the response, your complaint will be reviewed by the Customer Services Director. You can write to her at: Thames Water Utilities Ltd. PO Box 492, Swindon, SN38 8TU.

If the Goods or Services covered by this invoice falls under the regulation of the 1991 Water Industry Act, and you remain dissatisfied you can refer your complaint to Consumer Council for Water on 0121 345 1000 or write to them at Consumer Council for Water, 1st Floor, Victoria Square House, Victoria Square, Birmingham, B2 4AJ.

Ways to pay your bill

Credit Card	BACS Payment	Telephone Banking	Cheque
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Cambridge House CB8 7PN

Infiltration SuDS GeoReport:

This report provides information on the suitability of the subsurface for the installation of infiltration sustainable drainage systems (SuDS). It provides information on the properties of the subsurface with respect to significant constraints, drainage, ground stability and groundwater quality protection.

Report Id: GR_216718/1

Client reference: U321 BGS





Search location

Search location indicated in Point centred at: 536614,218820 red

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Assessment for an infiltration sustainable drainage system

Introduction

Sustainable drainage systems (SuDS) are drainage solutions that manage the volume and quality of surface water close to where it falls as rain. They aim to reduce flow rates to rivers, increase local water storage capacity and reduce the transport of pollutants to the water environment. There are four main types of SuDS, which are often designed to be used in sequence. They comprise:

- source control: systems that control the rate of runoff
- pre-treatment: systems that remove sediments and pollutants
- retention: systems that delay the discharge of water by providing surface storage
- infiltration: systems that mimic natural recharge to the ground.

This report focuses on infiltration SuDS. It provides subsurface information on the quality protection. It is intended principally for those involved in the preliminary assessment of the suitability of the ground for infiltration SuDS, and those involved in assessing proposals from others for sustainable drainage, but it may also be useful to help house-holders judge whether or not further professional advice should be sought. If in doubt, users should consult a suitably-qualified professional about the properties of the ground with respect to drainage, ground stability and groundwater results in this report before making any decisions based upon it.

This GeoReport is structured in two parts:

Part 1. Summary data.

Comprises three maps that summarise the data contained within Part 2.

Part 2. Detailed data.

Comprises a further 24 maps in four thematic sections:

- (soluble rocks, non-coal shallow mining and landslide hazards); persistent shallow groundwater, or the presence of made ground, which may result in adverse impacts due to factors including: ground instability Very significant constraints. Maps highlight areas where infiltration may represent a ground stability or contamination hazard.
- Drainage potential. Maps indicate the drainage potential of the ground, by considering subsurface permeability, depth to groundwater and the presence of floodplain deposits.
- Ground stability. Maps indicate the presence of hazards that have the potential to cause ground instability resulting in damage to some buildings and structures, if water is infiltrated to the ground.
- Groundwater protection. Maps provide key indicators to help determine whether the groundwater may be susceptible to deterioration in quality as a result of infiltration.

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SuDS, such as soakaways, infiltration basins or permeable pavements. It provides subsurface data to indicate whether, and which type of infiltration system may be appropriate. It does not state that infiltration SuDS are, or are not, appropriate as this is highly dependent on the design of the individual system. This report therefore describes the subsurface conditions at the site, allowing the reader to determine the This report considers the suitability of the subsurface for the installation of infiltration suitability of the site for infiltration SuDS.

The map and text data in this report is similar to that provided in the 'Infiltration SuDS Map: Detailed' national map product. For further information about the data, consult the 'User Guide for the Infiltration SuDS Map: Detailed', available from nttp://nora.nerc.ac.uk/16618/.

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PART 1: SUMMARY DATA

his section provides a summary of the data on the following pages.

In terms of the drainage potential, is the ground suitable for infiltration SuDS?



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Highly compatible for infiltration SuDS.

The subsurface is likely to be suitable for free-draining infiltration SuDS.

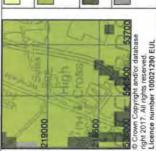
The subsurface is probably suitable although the design Probably compatible for infiltration SuDS

Opportunities for bespoke infiltration SuDS. may be influenced by the ground conditions.

The subsurface is potentially suitable although the design will be influenced by the ground conditions.

There is a very significant potential for one or more hazards Very significant constraints are indicated associated with infiltration.

Is ground instability likely to be a problem?



Increased infiltration is very unlikely to result in ground instability.

anticipated, but increased infiltration is unlikely to result Ground instability problems may be present or in ground instability

Increased infiltration may result in ground instability, Ground instability problems are probably present.

There is a very significant potential for one or more geohazards associated with infiltration.

Is the groundwater susceptible to deterioration in quality?



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The groundwater is not expected to be especially

vulnerable to contamination.

The groundwater may be vulnerable to contamination

The groundwater is likely to be vulnerable to contaminants. Made ground is present at the surface. Infiltration may increase the possibility of remobilising pollutants.





PART 2: DETAILED DATA

This section provides further information about the properties of the ground and will help assess the suitability of the ground for infiltration SuDS.

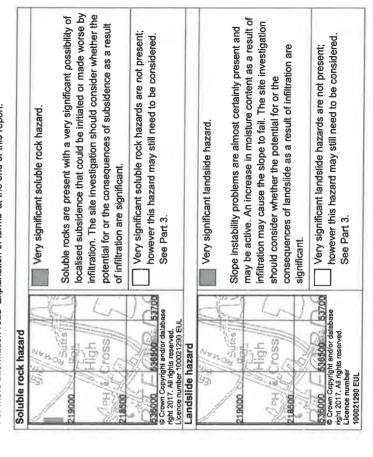
Section 1. Very significant constraints

Where maps are overlain by grey polygons, geological or hydrogeological hazards

may exist that could be made worse by infiltration. The following hazards are considered:

- soluble rocks
- landslides
- shallow mining
- shallow groundwater
 - made ground

For more information read 'Explanation of terms' at the end of this report.



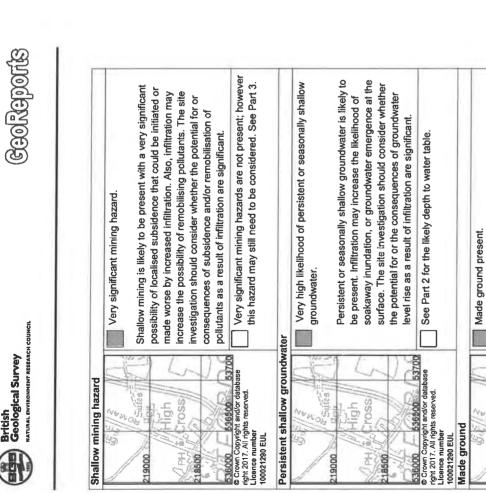
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18500

219000

3

219000

British Geological Survey



Section 2. Drainage potential

The following pages contain maps that will help you assess the drainage potential of the ground by considering the:

- depth to water table
- permeability of the superficial deposits
- thickness of the superficial deposits
- permeability of the bedrock
- presence of floodplains

superficial deposit permeability map may not be coloured. Where this is the case, the are thin (< 3 m thick) or thick (>3 m). Where they are over 3 m thick, the permeability consider both the permeability of the superficial deposits and the permeability of the bedrock. The superficial thickness map will tell you whether the superficial deposits deposits in some places are very thin and hence in these places you may wish to Superficial deposits are not present everywhere and therefore some areas of the bedrock permeability map shows the likely permeability of the ground. Superficial of the bedrock may not be relevant.

For more information read 'Explanation of terms' at the end of this report.

Depth to groundwater table 19000 8500

Groundwater is likely to be more than 5 m below the ground surface throughout the year. Groundwater is likely to be between 3 and 5 m below the ground surface for at least part of the year.

Groundwater is likely to be less than 3 m below the ground surface for at least part of the year.

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pollutants. The site investigation should consider whether the

5

219000

8500

Made ground

18500

None recorded

536500 53700

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ground stability or increase the possibility of remobilising potential for or consequences of ground instability and/or pollutant leaching as a result of infiltration are significant.

Made ground is present at the surface. Infiltration may affect

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Superficial deposits are likely to be poorly draining.

Maximum

Minimum

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These maps show the permeability range that is summarised above. 19000

219000

Very Low Low Moderate High

Very High High

18500

variable, but likely to permit moderate infiltration.

The superficial deposit permeability is spatially

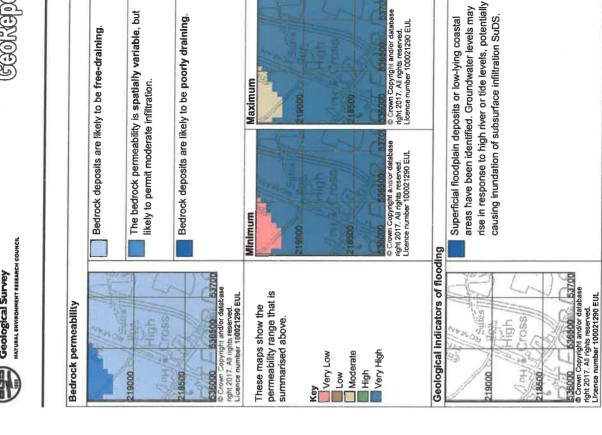
Superficial deposits are likely to be free-draining.

Superficial deposit permeability









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Superficial deposit thickness

hence the permeability of the superficial deposits is

likely to determine the permeability of the ground.

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The thickness of superficial deposits is > 3 m and

dependent on both the superficial deposits (where

hence the permeability of the ground may be present) and underlying bedrock (see below).

The thickness of superficial deposits is < 3 m and

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Section 3. Ground stability

The following pages contain maps that will help you assess whether infiltration may impact the stability of the ground. They consider hazards associated with:

- soluble rocks
- landslides
- shallow mining
- running sands
- swelling clays
- compressible ground, and
- collapsible ground

In the following maps, geohazards that are identified in green are unlikely to prevent infiltration SuDS from being installed, but they should be considered during design. For more information read 'Explanation of terms' at the end of this report.

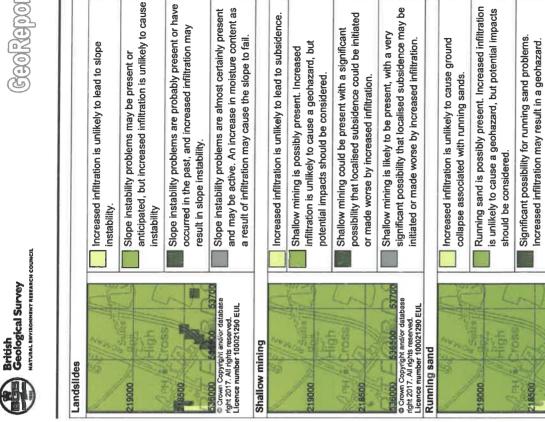
Soluble rocks



Increased infiltration is unlikely to cause localised subsidence, but potential impacts should be considered.

subsidence. The potential for or the consequences of subsidence associated with soluble rocks should be Increased infiltration may result in localised considered. Very significant possibility of localised subsidence that could be initiated or made worse by infiltration.





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Swelling clays

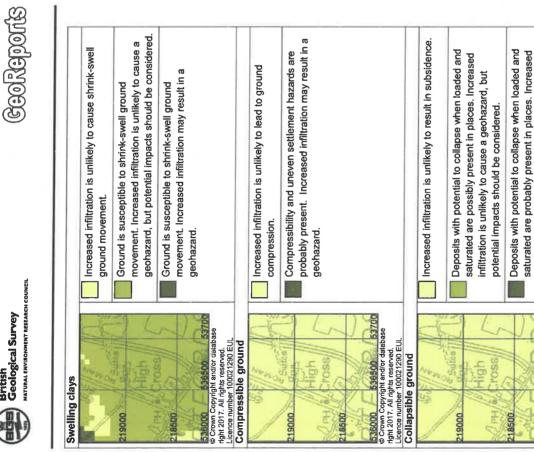
219000

536000

218600







19000

18500

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infiltration may result in a geohazard.

535000 536500 53700 © Crown Copyright and/or database right 2017. All rights reserved. Licence number 100021290 EUL

18500

19000





Section 4. Groundwater quality protection

The following pages contain maps showing some of the information required to ensure the protection of groundwater quality. Data presented includes:

- groundwater source protection zones (Environment Agency data)
- predorninant flow mechanism

For more information read 'Explanation of terms' at the end of this report.

ion zones	Groundwater is not within a source protection zone.	Source protection zone IV	Source protection zone III	Source protection zone II	Source protection zone I.			E	Water is likely to percolate through the unsaturated zone to the groundwater through either the pore space in granular media or through porespace and fractures; these processes have some potential for contaminant removal and breakdown. Water is likely to percolate through the unsaturated zone to the groundwater through fractures, a process which has little potential for contaminant removal and breakdown.
Groundwater source protection zones	THE SAME "N	219000		218500	建筑是	© Crown Copyright and or database right 2017. All rights reserved. Licence number 100021290 EUL	Derived in part from Scurce Protection Zone data provided under licence from the Euvironment Agency © Environment Agency 2017.	Predominant flow mechanism	215000 21500 21500 526000 536000





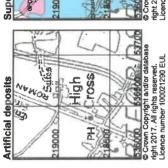
	Made ground is present at the surface. Infiltration may	increase the possibility of remobilising pollutants.								
tade ground	1 5//2	10	19000 / Cautes 7 /	1 = 3/4/	dgiH//	PH/# LOSS/	18500	38000 - 636500 - 53700	Crown Copyright and/or database	cence number 100021290 EUL

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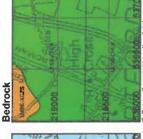
Section 5. Geological Maps

The following maps show the artificial, superficial and bedrock geology within the area of interest.









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Coal, ironstone or mineral vein

Fault

Note: Faults and Coals, ironstone & mineral veins are shown for illustration and to aid interpretation of the map. Not all such features are shown and their absence on the map face does not necessarily mean that none are present

Key to Artificial deposits; No deposits recorded by BGS in the search area

Key to Superficial deposits:

,			
Map colour	Computer Code	Rock name	Rock type
	LOFT-DMTN	LOWESTOFT FORMATION	DIAMICTON
	GFDMP-XSV	GLACIOFLUVIAL DEPOSITS, MID PLEISTOCENE	SAND AND GRAVEL
	HEAD-XCZSV	HEAD	CLAY, SILT, SAND AND GRAVEL





Key to Bedrock geology:

flap colour	Computer	Rock name	Rock type
	LMBE-XCZS	LAMBETH GROUP	CLAY, SILT AND SAND
	LESE-CHLK	LEWES NODULAR CHALK FORMATION AND SEAFORD CHALK FORMATION (UNDIFFERENTIATED)	CHALK

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Limitations of this report:

- This report is concerned with the potential for infiltration-to-the-ground to be used as a SuDS technique at the site described. It only considers the subsurface beneath the search area and does NOT consider potential surface or subsurface impacts outside of that area.
- This report is NOT an alternative for an on-site investigation or soakaway test, which might reach a different conclusion.
- This report must NOT be used to justify disposal of foul waste or grey water.
- datasets used (with the exception of that showing depth to water table) are based This report is based on and limited to an interpretation of the records held by the British Geological Survey (BGS) at the time the search is performed. The on 1:50 000 digital geological maps and not site-specific data.
 - Other more specific and detailed ground instability information for the site may be held by BGS, and an assessment of this could result in a modified assessment.
 - To interpret the maps correctly, the report must be viewed and printed in colour. The search does NOT consider the suitability of sites with regard to:
 - previous land use,
- potential for, or presence of contaminated land
- presence of perched water tables
- shallow mining hazards relating to coal mining. Searches of coal mining should be carried out via The Coal Authority Mine Reports Service: www.coalminingreports.co.uk.
 - made ground, where not recorded
- proximity to landfill sites (searches for landfill sites or contaminated land should be carried out through consultation with local authorities/Environment Agency) 0 0
 - zones around private water supply boreholes that are susceptible to groundwater contamination.
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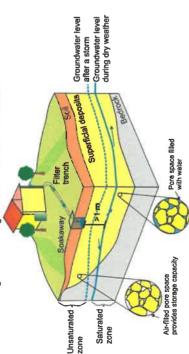




Explanation of terms

Depth to groundwater

In the shallow subsurface, the ground is commonly unsaturated with respect to water. Air fills the spaces within the soil and the underlying superficial deposits and bedrock. At some depth below the ground surface, there is a level below which these spaces are full of water. This level is known as the groundwater level, and the water below it is termed the groundwater. When water is infiltrated, the groundwater level may rise temporarily. To ensure that there is space in the unsaturated zone to accommodate this, there should be a minimum thickness of 1 m between the <u>base</u> of the infiltration system and the <u>water table</u>. An estimate of the depth to groundwater is therefore useful in determining whether the ground is suitable for infiltration.



Groundwater flooding

Groundwater flooding occurs when a rise in groundwater level results in very shallow groundwater or the emergence of groundwater at the surface. If infiltration systems are installed in areas that are susceptible to groundwater flooding, it is possible that the system could become inundated. The susceptibility map seeks to identify areas where the geological conditions and water tables indicate that groundwater level rise could occur under certain circumstances. A high susceptibility to groundwater flooding classification does not mean that groundwater flooding has ever occurred in the past, or will do so in the future as the susceptibility maps do not contain information on how often flooding may occur. The susceptibility maps are designed for planning; identifying areas where groundwater flooding might be an issue that needs to be taken into account.

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Geological indicators of flooding

In floodplain deposits, groundwater level can be influenced by the water level in the adjacent river. Groundwater level may increase during periods of fluvial flood and therefore this should be taken into account when designing infiltration systems on such deposits. The geological indicators of flooding dataset shows where there is geological evidence (floodplain deposits) that flooding has occurred in the past.

For further information on flood-risk, the likely frequency of its recurrence in relation to any proposed development of the site, and the status of any flood prevention measures in place, you are advised to contact the local office of the Environment Agency (England and Wales) at www.environment-agency.gov.uk or the Scottish Environment Protection Agency (Scotland) at www.sepa.org.uk.

Artificial ground

Artificial ground comprises deposits and excavations that have been created or modified by human activity. It includes ground that is worked (quarries and road cuttings), infilled (back-filled quarries), landscaped (surface re-shaping), disturbed (near surface mineral workings) or classified as made ground (embankments and spoil heaps). The composition and properties of artificial ground are often unknown. In particular, the permeability and chemical composition of the artificial ground should be determined to ensure that the ground will drain and that any contaminants present will not be remobilised.

Superficial permeability

Superficial deposits are those geological deposits that were formed during the most recent period of geological time (as old as 2.6 million years before present). They generally comprise relatively thin deposits of gravel, sand, sitt and clay and are present beneath the pedological soil in patches or larger spreads over much of Britain. The ease with which water can percolate through these deposits is controlled by their permeability and varies widely depending on their composition. Those deposits comprising clays and silts are less permeable and thus infiltration is likely to be slow, such that water may pool on the surface. In comparison, deposits comprising sands and gravels are more permeable allowing water to percolate freely.

Bedrock permeability

Bedrock forms the main mass of rock forming the Earth. It is present everywhere, commonly beneath superficial deposits. Where the superficial deposits are thin or absent, the ease with which water will percolate into the ground depends on the permeability of the bedrock.





Natural ground instability

Natural ground instability refers to the propensity for upward, lateral or downward movement of the ground that can be caused by a number of natural geological hazards (e.g. ground dissolution/compressible ground). Some movements associated with particular hazards may be gradual and of millimetre or centimetre scale, whilst others may be sudden and of metre or tens of metres scale. Significant natural ground instability has the potential to cause damage to buildings and structures, especially when the drainage characteristics of a site are altered. It should be noted, however, that many buildings, particularly more modern ones, are built to such a standard that they can remain unaffected in areas of significant ground movement.

hrink-owell

A shrinking and swelling clay changes volume significantly according to how much water it contains. All clay deposits change volume as their water content varies, typically swelling in winter and shrinking in summer, but some do so to a greater extent than others. Contributory circumstances could include drought, leaking service pipes, tree roots drying-out the ground or changes to local drainage patterns, such as the creation of soakaways. Shrinkage may remove support from the foundations of buildings and structures, whereas clay expansion may lead to uplift (heave) or lateral stress on part or all of a structure; any such movements may cause cracking and distortion.

Landslides (slope stability)

A landslide is a relatively rapid outward and downward movement of a mass of ground on a slope, due to the force of gravity. A slope is under stress from gravity but ground on a slope, due to the force of gravity. A slope is under stress from gravity but will not move if its strength is greater than this stress. If the balance is altered so that the stress exceeds the strength, then movement will occur. The stability of a slope can be reduced by removing ground at the base of the slope, by placing material on the slope, especially at the top, or by increasing the water content of the materials forming the slope. Increase in subsurface water content beneath a soakaway could increase susceptibility to landslide hazards. The assessment of landslide hazard refers to the stability of the present land surface. It does not encompass a consideration of the stability of excavations.

Soluble rocks (dissolution)

Some rocks are soluble in water and can be progressively removed by the flow of water through the ground. This process tends to create cavities, potentially leading to the collapse of overlying materials and possibly subsidence at the surface. The release of water into the subsurface from infiltration systems may increase the dissolution of rock or destabilise material above or within a cavity. Dissolution cavities may create a pathway for rapid transport of contaminated water to an aquifer or water course.

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Compressible ground

Many ground materials contain water-filled pores (the spaces between solid particles). Ground is compressible if a building (or other load) can cause the water in the pore space to be squeezed out, causing the ground to decrease in thickness. If ground is extremely compressible the building may sink. If the ground is not uniformly compressible, different parts of the building may sink by different amounts, possibly causing tilting, cracking or distortion. The compressibility of the ground may alter as a result of changes in subsurface water content caused by the release of water from soakaways.

Collapsible deposits

Collapsible ground comprises certain fine-grained materials with large pore spaces (the spaces between solid particles). It can collapse when it becomes saturated by water and/or a building (or other structure) places too great a load on it. If the material below a building collapses it may cause the building to sink. If the collapsible ground is variable in thickness or distribution, different parts of the building may sink by different amounts, possibly causing tilting, cracking or distortion. The subsurface underlying a soakaway will experience an increase in water content that may affect the stability of the ground. This hazard is most likely to be encountered only in parts of southern England.

Running sand

Running sand conditions occur when loosely-packed sand, saturated with water, flows into an excavation, borehole or other type of void. The pressure of the water filling the spaces between the sand grains reduces the contact between the grains and they are carried along by the flow. This can lead to subsidence of the surrounding ground. Running sand is potentially hazardous during the drainage system installation. During installation, excavation of the ground may create a space into which sand can flow, potentially causing subsidence of surrounding ground.

Shallow mining hazards (non coal)

Current or past underground mining for coal or for other commodities can give rise to cavities at shallow or intermediate depths, which may cause fracturing, general settlement, or the formation of crown-holes in the ground above. Spoil from mineral workings may also present a pollution hazard. The release of water into the subsurface from soakaways may destabilise material above or within a cavity. Cavities arising as a consequence of mining may also create a pathway for rapid transport of contaminated water to an aquifer or watercourse. The mining hazards map is derived from the geological map and considers the potential for subsidence associated with mining on the basis of geology type. Therefore if mining is known to occur within a certain rock, the map will highlight the potential for a hazard within the area covered by that geology.







For more information regarding underground and opencast coal mining, the location of 200 Lichfield Lane, Mansfield, Nottinghamshire, NG18 4RG; telephone 0845 762 6848 movement induced by coal mining please contact the Coal Authority, Mining Reports, or at www.coal.gov.uk. For more information regarding other types of mining (i.e. nonmine entries (shafts and adits) and matters relating to subsidence or other ground coal), please contact the British Geological Survey.

Groundwater source protection zones

source protection zones. In conjunction with Groundwater Protection Policy the zones are used to restrict activities that may impact groundwater quality, thereby preventing Environment Agency can provide advice on the location and implications of source boreholes and springs that are used for the abstraction of public drinking water as In England and Wales, the Environment Agency has defined areas around wells, pollution of underlying aquifers, such that drinking water quality is upheld. The protection zones in your area (www.environment-agency.gov.uk/)

NATURAL ENVIRONMENT RESEARCH COUNCIL British Geological Survey



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Email: enquiry@bgs.ac.uk British Geological Survey Research Avenue South Tel: 0131 6671000 Lyell Centre EH14 4AP Edinburgh

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Page: 25 of 25 BGS Report No: GR_216718/1



TRIAL PIT LOG

Project			Client		TRIAL PIT No
Land at Sutes	Farm, High Cross		M Scott	Properties Ltd	TP1
Job No 2895,SK	Date 25-01-18 25-01-18	Groun	d Level (m)	Grid Reference ()	IPI
Fieldwork By		•	Logged By		Sheet
GEL			EP		1 of 1

Depth	DESCRIPTION	Legend	Depth	No	Remarks/Tests
0.00-0.30	TOPSOIL (Dark brown clay with occasional gravel of fine and medium subangular and subrounded flint, chalk and quartzite)	_	Берин	110	Nemansy rests
0.30-1.50	Orange/pale brown gravelly CLAY. Gravel is fine to coarse subangular and subrounded flint and chalk (LOWESTOFT FORMATION)				
	1.10 Becoming sandy and friable with depth				
	- - 	- 0 -			

VIBULE VILLE TOOL TOOL TOOL TO THE VILLE TO



GEL AGS TP BETA 2895,SK HIGH CROSS, 29-01-18.GPJ GINT STD AGS 3_1.GDT 31/1/18

TRIAL PIT LOG

Project			Client		TRIAL PIT No	
Land at Sutes	Farm, High Cross		M Scott		TP2	
Job No 2895,SK	Date 25-01-18 25-01-18	Groun	d Level (m)	Grid Reference ()		IPZ
Fieldwork By		•	Logged By		Sh	eet
GEL			EP			1 of 1

Depth	DESCRIPTION	Legend	Depth	No	Remarks/Tests
0.00-0.35	TOPSOIL (Dark brown slightly gravelly clay. Gravel is fine and medium subangular flint and chalk)	-			
0.35-1.50	Orange/pale brown gravelly CLAY. Gravel is fine to coarse subangular and subrounded chalk and flint (LOWESTOFT FORMATION)				
-	- - -				
-	1.00 Becoming friable with depth -				
-	- 	-			
_		-			

- 1.6 -₹ 0.3 Shoring/Support: None Stability: Stable Plant UsedMECHANICAL Checked By

All dimensions in metres Scale 1:15 Method Trial Pit/trench **EXCAVATOR**



TRIAL PIT LOG

Project			Client			TRIAL PIT No
Land at Sutes	Farm, High Cross		M Scott	TP3		
Job No Date 25-01-18 Ground			d Level (m)	Grid Reference ()		1175
2895,SK	25-01-18					
Fieldwork By			Logged By			Sheet
GEL			EP			1 of 1

Depth	DESCRIPTION	Legend	Depth	No	Remarks/Tests
0.00-0.30	TOPSOIL (Dark brown slightly gravelly clay. Gravel is fine to medium subangular flint and chalk)	-			
0.30-1.50	Pale brown/orange/grey gravelly friable CLAY. Gravel is fine to coarse subangular flint and subrounded chalk (LOWESTOFT FORMATION)				
	- - -				
	- - 				
	-	-			

GEL AGS TP BETA 2895,SK HIGH CROSS, 29-01-18.GPJ GINT STD AGS 3_1.GDT 31/1/18 - 1.7 -₹ 0.3 Shoring/Support: None Stability: Stable All dimensions in metres Scale 1:15 Method Trial Pit/trench Plant UsedMECHANICAL Checked By

EXCAVATOR



TRIAL PIT LOG

Project			Client	TR	TRIAL PIT No	
Land at Sutes	Farm, High Cross		M Scott		TP4	
Job No 2895,SK	Date 25-01-18 25-01-18	Groun	d Level (m)	Grid Reference ()		174
Fieldwork By		•	Logged By		Sheet	
GEL			EP			1 of 1

Depth	DESCRIPTION	Legend	Depth	No	Remarks/Tests
0.00-0.30	TOPSOIL (Dark brown slightly gravelly clay. Gravel is fine and medium subangular flint and chalk)	-			
0.30-1.50	Pale brown/orange/grey gravelly CLAY. Gravel is fine to coarse subangular and subrounded chalk and flint (LOWESTOFT FORMATION)	_ _			
_	-				
	- -				
_	- -				
_	- 				
	-				

GEL AGS TP BETA 2895,SK HIGH CROSS, 29-01-18.GPJ GINT STD AGS 3_1.GDT 31/1/18 - 1.6 -₹ 0.3 Shoring/Support: None Stability: Stable Plant UsedMECHANICAL Checked By

All dimensions in metres Scale 1:15 Method Trial Pit/trench **EXCAVATOR**

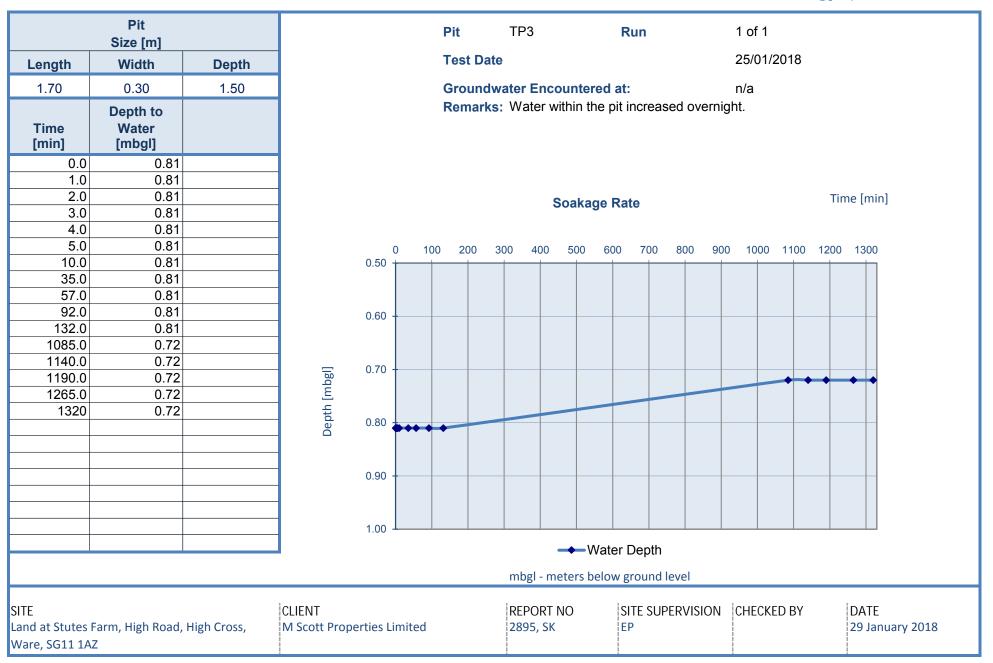
geosphere environmental ltd

Geosphere Environmental Ltd, Brightwell Barns, Ipswich Road, Brightwell, Suffolk, IP10 0BJ T 01603 298 076 E info@geosphere-environmental.co.uk

	Pit Size [m]					Pit		TP2			Rur	1		1 c	of 1					
Length	Width	Depth	1			Test I	Date							25	/01/20	18				
1.60	0.30	1.50				Grou	ndwa	ter Eı	ncour	nterec	l at:			n/a	ì					
Time [min]	Depth to Water [mbgl]					Rema	ırks:	Maxir	num v	water	depth	achie\	ved in	the tes	st = 0.6	35mbg	l .			
0.0	0.65																			
1.0	0.65		-																	
2.0 3.0	0.65 0.65		4						Soa	kage	Rate						Time	[min]	
4.0	0.65		1																	
5.0	0.65		1	C	100	200	300	0 40)O 5	00 6	800 7	'00 E	800	900 1	000 1	100 1	200 1	1300		
10.0	0.65		1	0.50 +	, 100	200	300		,0 0		100 /	1	-	J00 I	000 1	100 1	200	1300		
37.0	0.65]																	
67.0	0.65																			
88.0	0.65		1	0.60							_	-				-				
148.0	0.65		4																	
1077.0	0.65		4																	
1137.0 1190.0	0.65 0.65		<u> </u>	0.70																
1275.0	0.65		ig m																	
			Depth [mbgl]	0.80 -	-															
			-	0.90 -																
				1.00																
			1						-	- Wa	ter De	pth								
								mbgl -	mete	rs belo	ow grou	und lev	/el							
SITE and at Stutes Vare, SG11 1A	Farm, High Road, Z	High Cross,	CLIENT M Scott Prop	oerties Lii	mited			REPOI 2895,	RT NO SK		SITE	SUPER	RVISIO	N CH	ECKED	ВҮ		ATE 9 Jani	uary 20)18

geosphere environmental ltd

Geosphere Environmental Ltd, Brightwell Barns, Ipswich Road, Brightwell, Suffolk, IP10 0BJ T 01603 298 076 E info@geosphere-environmental.co.uk



geosphere environmental ltd

Geosphere Environmental Ltd, Brightwell Barns, Ipswich Road, Brightwell, Suffolk, IP10 0BJ T 01603 298 076 E info@geosphere-environmental.co.uk

	Pit Size [m]				Pit	TP4	Run	1 of 1		
Length	Width	Depth			Test Da	te		26/01	/2018	
1.60	0.30	1.50	1		Ground	water End	ountered at:	n/a		
Time	Depth to Water		1		Remark	s:				
[min]	[mbgl]							ull-depth soakawa	y test. Maximur	n water depth
0.0	0.71				achieve	d in the te	est = 0.71mbgl			
1.0	0.71									
2.0	0.71		-				Soakage Rate		Time [min]
3.0	0.71		-				_			
4.0 5.0	0.71 0.71		-							
10.0	0.71		1	0.50		50	100	150	200	_
21.0	0.71		1	0.50						
30.0	0.71		1							
51.0	0.71		1	0.60						
103.0	0.71		1	0.00						
163.0	0.71		1							
223.0	0.71] _	0.70						
			lgd	0.70			•	•		•
			Depth [mbgl]							
			bth	0.80						
				0.00						
			-							
			-	0.90						
			-	0.90						
			-							
			1	1.00						
			1	1.00			→ Water Dept	th.		
			_							
						mbgl - n	neters below groun	nd level		
SITE			CLIENT			REPORT		UPERVISION CHECK		
	Farm, High Road,	High Cross,	M Scott Prop	perties Limite	d	2895, SI	EP		29	January 2018
Vare, SG11 1A							I I			



LEGEND:

Trial pit locations





geosphere environmental ltd

Brightwell Barn, Ipswich Road, Brightwell, Suffolk, IP10 0BJ T 01603 298 F 01603 289 075 info@geosphere-environmental.co.uk

Land at Stutes Farm, High Road, High Cross, Ware, SG11 1AZ.

TITLE **Exploratory Hole Location Plan** CLIENT M Scott Properties Limited

REPORT NO. 2895, SK DRAWN BY FS

DRAWING NO. 001 / Rev 0 CHECKED

DATE January 2018 SCALE Not to scale



Photograph 1

Saturated ground conditions -Centre of the site.



Photograph 2

Saturated ground conditions in the centre of the site.



Photograph 3

Saturated ground conditions to the south of the site, alongside the footpath.







Photograph 4

Perched water infiltration into TP1

Photograph 5

Saturated conditions located near to TP1.

Photograph 6

Saturated conditions located south of the site.

Appendix B – Proposed Site
Illustrative development layout
Surface water management plan
Impermeable catchment plan
Surface water maintenance plan
Surface water calculations

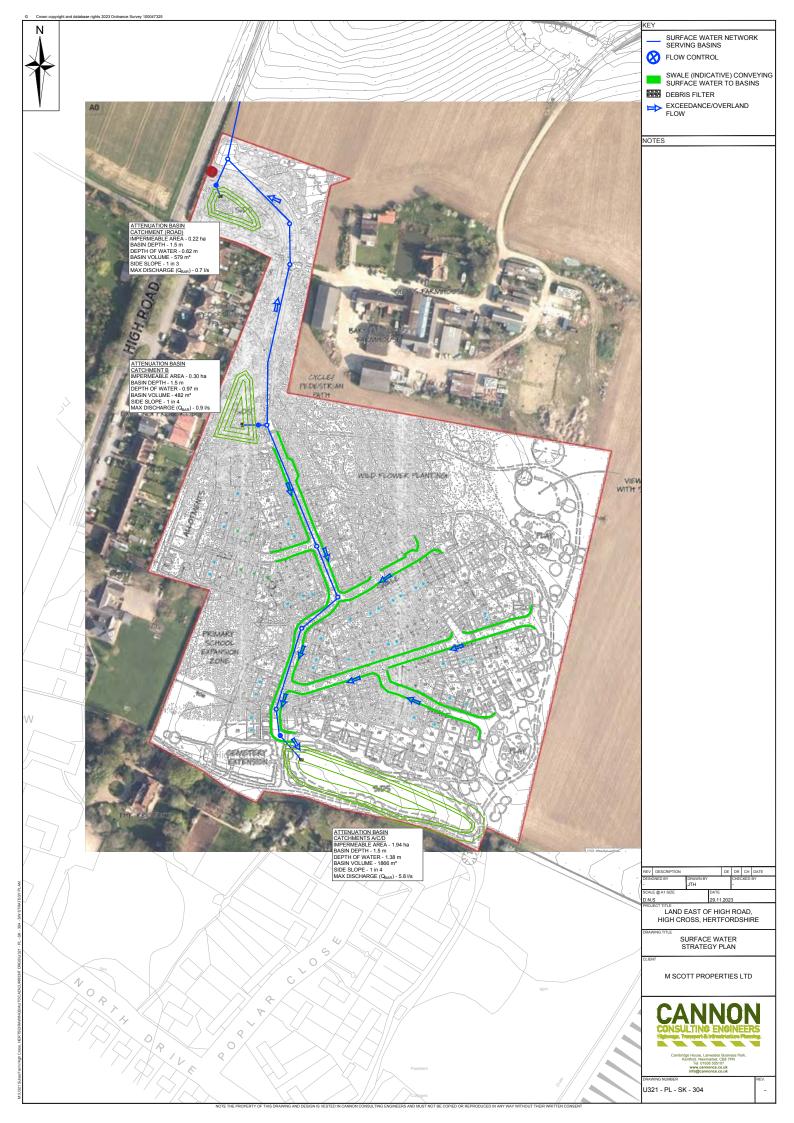


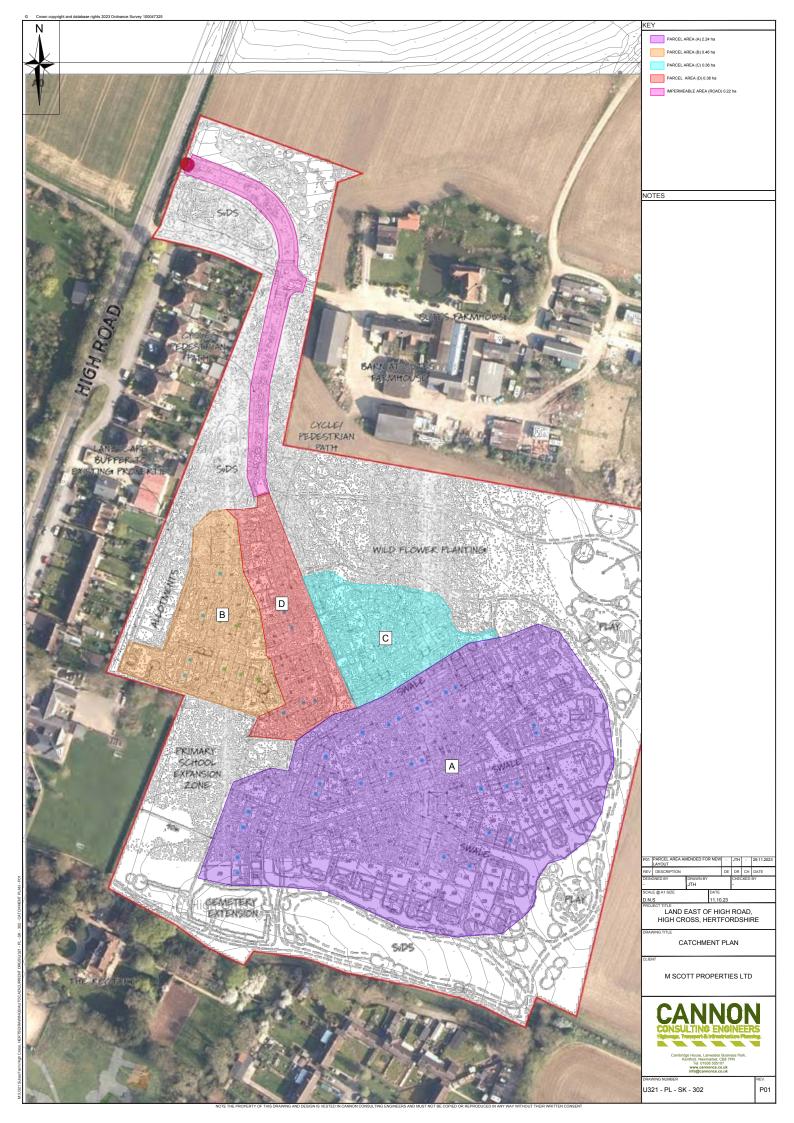
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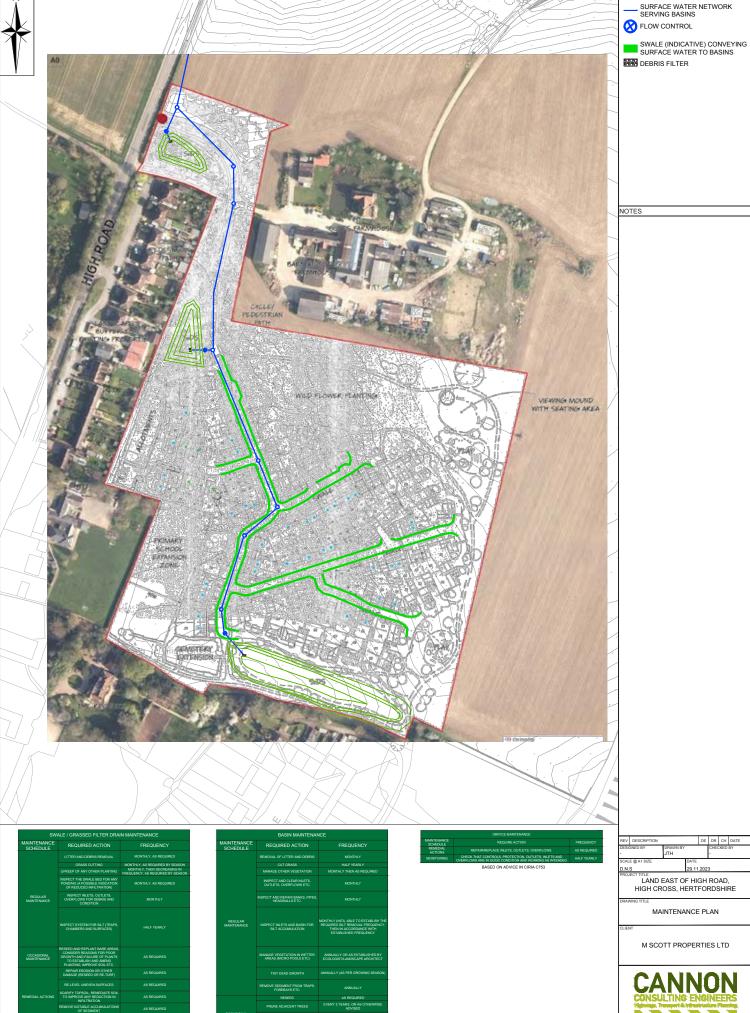
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atus.	PRELIMINA	RY	Office	Romsey

thrive.







SURFACE WATER NETWORK SERVING BASINS

29.11.2023 LAND EAST OF HIGH ROAD, HIGH CROSS, HERTFORDSHIRE

M SCOTT PROPERTIES LTD



U321 - PL - SK - 303

Network: Storm Network

James Howard 29/11/2023

Design Settings

Rainfall Methodology	FEH-22	Minimum Velocity (m/s)	1.00
Return Period (years)	100	Connection Type	Level Soffits
Additional Flow (%)	0	Minimum Backdrop Height (m)	0.200
CV	0.950	Preferred Cover Depth (m)	1.200
Time of Entry (mins)		Include Intermediate Ground	\checkmark
Maximum Time of Concentration (mins)	30.00	Enforce best practice design rules	\checkmark
Maximum Rainfall (mm/hr)	500.0		

Nodes

Name	Area	Cover	Depth
	(ha)	Level	(m)
		(m)	
Basin A.C.D	1.940	89.000	1.500

Simulation Settings

Rainfall Methodology	FEH-22	Drain Down Time (mins)	240	30 year (l/s)	13.8
Summer CV	0.950	Additional Storage (m³/ha)	20.0	100 year (l/s)	18.4
Winter CV	0.950	Check Discharge Rate(s)	\checkmark	Check Discharge Volume	\checkmark
Analysis Speed	Normal	1 year (l/s)	4.9	100 year 360 minute (m³)	427
Skip Steady State	х	2 vear (l/s)	5.1		

Storm Durations

15	60	180	360	600	960	2160	4320	7200	10080
30	120	240	480	720	1440	2880	5760	8640	

Return Period	Climate Change	Additional Area	Additional Flow
(years)	(CC %)	(A %)	(Q %)
2	0	0	0
30	0	0	0
30	35	0	0
100	0	0	0
100	40	0	0
100	40	5	0

Pre-development Discharge Rate

Site Makeup	Greenfield	Growth Factor 30 year	2.40
Greenfield Method	IH124	Growth Factor 100 year	3.19
Positively Drained Area (ha)	1.940	Betterment (%)	0
SAAR (mm)	623	QBar	5.8
Soil Index	3	Q 1 year (I/s)	4.9
SPR	0.40	Q 2 year (I/s)	5.1
Region	6	Q 30 year (I/s)	13.8
Growth Factor 1 year	0.85	Q 100 year (I/s)	18.4
Growth Factor 2 year	0.88		

Pre-development Discharge Volume

Site Makeup	Greenfield	CWI	93.778
Greenfield Method	FSR/FEH	Return Period (years)	100
Positively Drained Area (ha)	1.940	Climate Change (%)	0
Soil Index	3	Storm Duration (mins)	360
SPR	0.40	Betterment (%)	0



File: U321 Catchment A-C-D.pf | Page 2 Network: Storm Network

James Howard 29/11/2023

Pre-development Discharge Volume

PR 0.360 Runoff Volume (m³) 427

Node Basin A,C,D Online Orifice Control

Flap Valve	Х	Design Depth (m)	1.300	Discharge Coefficient	0.600
Replaces Downstream Link	\checkmark	Design Flow (I/s)	4.8		
Invert Level (m)	87.500	Diameter (m)	0.045		

Node Basin A,C,D Depth/Area Storage Structure

Base Inf Coefficient (m/hr)	0.00000	Safety Factor	2.0	Invert Level (m)	87.500
Side Inf Coefficient (m/hr)	0.00000	Porosity	1.00	Time to half empty (mins)	

Depth	Area	Inf Area	Depth	Area	Inf Area
(m)	(m²)	(m²)	(m)	(m²)	(m²)
0.000	543.0	0.0	1.500	2045.0	0.0



File: U321 Catchment A-C-D.pf | Page 3 Network: Storm Network

James Howard 29/11/2023

Results for 2 year Critical Storm Duration. Lowest mass balance: 99.99%

Node Event	US	Peak	Level	Depth	Inflow	Node	Flood	Status
	Node	(mins)	(m)	(m)	(I/s)	Vol (m³)	(m³)	
960 minute winter	Basin A.C.D	930	88.028	0.528	24.0	439.6241	0.0000	OK

Link Event	US	Link	Outflow	Discharge
(Upstream Depth)	Node		(I/s)	Vol (m³)
960 minute winter	Basin A.C.D	Orifice	3.0	154.2



File: U321 Catchment A-C-D.pf
Network: Storm Network

James Howard 29/11/2023 Page 4

Results for 30 year Critical Storm Duration. Lowest mass balance: 99.99%

Node Event	US	Peak	Level	Depth	Inflow	Node	Flood	Status
	Node	(mins)	(m)	(m)	(I/s)	Vol (m³)	(m³)	
960 minute winter	Basin A.C.D	945	88.395	0.895	46.4	910.7439	0.0000	OK

Link EventUSLinkOutflowDischarge(Upstream Depth)Node(I/s)Vol (m³)960 minute winterBasin A,C,DOrifice4.0206.9



File: U321 Catchment A-C-D.pf Network: Storm Network James Howard

Page 5

Results for 30 year +35% CC Critical Storm Duration. Lowest mass balance: 99.99%

29/11/2023

Node Event US Peak Level Depth Inflow Node Flood **Status** (I/s) Node (mins) Vol (m³) (m³) (m) (m) 1440 minute winter Basin A,C,D 1410 44.1 1262.6960 0.0000 OK 88.619 1.119

Link EventUSLinkOutflowDischarge(Upstream Depth)Node(I/s)Vol (m³)1440 minute winterBasin A,C,DOrifice4.4319.8



File: U321 Catchment A-C-D.pf | Page 6 Network: Storm Network

James Howard 29/11/2023

Results for 100 year Critical Storm Duration. Lowest mass balance: 99.99%

Node Event	US	Peak	Level	Depth	Inflow	Node	Flood	Status
	Node	(mins)	(m)	(m)	(I/s)	Vol (m³)	(m³)	
960 minute winter	Basin A,C,D	945	88.548	1.048	57.2	1145.7350	0.0000	OK

Link Event	US	Link	Outflow	Discharge
(Upstream Depth)	Node		(I/s)	Vol (m³)
960 minute winter	Basin A.C.D	Orifice	4.3	225.7



File: U321 Catchment A-C-D.pf Page 7 Network: Storm Network

James Howard 29/11/2023

Results for 100 year +40% CC Critical Storm Duration. Lowest mass balance: 99.99%

Node Event	US	Peak	Level	Depth	Inflow	Node	Flood	Status
	Node	(mins)	(m)	(m)	(I/s)	Vol (m³)	(m³)	
1440 minute winter	Basin A,C,D	1410	88.835	1.335	56.2	1651.6410	0.0000	OK

Link Event	US	Link	Outflow	Discharge
(Upstream Depth)	Node		(I/s)	Vol (m³)
1440 minute winter	Basin A.C.D	Orifice	4.8	352.1



File: U321 Catchment A-C-D.pf Network: Storm Network James Howard Page 8

Results for 100 year +40% CC +5% A Critical Storm Duration. Lowest mass balance: 99.99%

29/11/2023

Node Event US Peak Level Depth Inflow Node Flood **Status** Node (mins) (I/s) Vol (m³) (m³) (m) (m) 1440 minute winter Basin A,C,D 1410 88.881 1.381 59.0 1742.5690 0.0000 OK

Link EventUSLinkOutflowDischarge(Upstream Depth)Node(I/s)Vol (m³)1440 minute winterBasin A,C,DOrifice4.9358.7

CAUSEWAY

Network: Storm Network James Howard

29/11/2023

Page 1

Design Settings

Rainfall Methodology	FEH-22	Minimum Velocity (m/s)	1.00
Return Period (years)	100	Connection Type	Level Soffits
Additional Flow (%)	0	Minimum Backdrop Height (m)	0.200
CV	0.950	Preferred Cover Depth (m)	1.200
Time of Entry (mins)	5.00	Include Intermediate Ground	\checkmark
Maximum Time of Concentration (mins)	30.00	Enforce best practice design rules	\checkmark
Maximum Rainfall (mm/hr)	500.0		

Nodes

Name	Area	Cover	Depth
	(ha)	Level	(m)
		(m)	
Basin B	0.300	90.300	1.500

Simulation Settings

Rainfall Methodology	FEH-22	Drain Down Time (mins)	240	30 year (l/s)	2.1
Summer CV	0.950	Additional Storage (m³/ha)	20.0	100 year (l/s)	2.8
Winter CV	0.950	Check Discharge Rate(s)	\checkmark	Check Discharge Volume	\checkmark
Analysis Speed	Normal	1 year (I/s)	8.0	100 year 360 minute (m³)	66
Skip Steady State	X	2 year (I/s)	8.0		

					Storm Dura					
600	720	960	1440	2160	2880	4320	5760	7200	8640	10080

Return Period (years)	Climate Change (CC %)	Additional Area (A %)	Additional Flow (Q %)
2	0	0	0
30	0	0	0
30	35	0	0
100	0	0	0
100	40	0	0
100	40	5	0

Pre-development Discharge Rate

Site Makeup	Greenfield	Growth Factor 30 year	2.40
•		,	_
Greenfield Method	IH124	Growth Factor 100 year	3.19
Positively Drained Area (ha)	0.300	Betterment (%)	0
SAAR (mm)	623	QBar	0.9
Soil Index	3	Q 1 year (I/s)	8.0
SPR	0.40	Q 2 year (I/s)	8.0
Region	6	Q 30 year (I/s)	2.1
Growth Factor 1 year	0.85	Q 100 year (I/s)	2.8
Growth Factor 2 year	0.88		

Pre-development Discharge Volume

Site Makeup	Greenfield	Return Period (years)	100
Greenfield Method	FSR/FEH	Climate Change (%)	0
Positively Drained Area (ha)	0.300	Storm Duration (mins)	360
Soil Index	3	Betterment (%)	0
SPR	0.40	PR	0.360
CWI	93.778	Runoff Volume (m³)	66



File: U321 Catchment B.pfd Network: Storm Network James Howard

Page 2

Node Basin B Online Orifice Control

Flap Valve x
Replaces Downstream Link √
Invert Level (m) 88.800

Design Depth (m) 1.300 Design Flow (l/s) 0.8 Diameter (m) 0.020

29/11/2023

Discharge Coefficient 0.600

Node Basin B Depth/Area Storage Structure

Base Inf Coefficient (m/hr) 0.00000 Side Inf Coefficient (m/hr) 0.00000 Safety Factor 2.0 Porosity 1.00 Invert Level (m) 88.800

Time to half empty (mins)

Inf Area Inf Area Depth Area Depth Area (m) (m²) (m²) (m) (m²) (m²) 0.000 100.0 0.0 1.500 628.0 0.0



File: U321 Catchment B.pfd Network: Storm Network James Howard 29/11/2023 Page 3

Results for 2 year Critical Storm Duration. Lowest mass balance: 100.00%

Node Event US Peak Level Depth Inflow Node Flood Status Node (mins) (m) (m) (I/s) Vol (m³) (m³) 720 minute winter Basin B 705 89.184 0.384 4.7 65.8927 0.0000 OK

Link EventUSLinkOutflowDischarge(Upstream Depth)Node(I/s)Vol (m³)720 minute winterBasin BOrifice0.521.7



File: U321 Catchment B.pfd Network: Storm Network James Howard Page 4

Results for 30 year Critical Storm Duration. Lowest mass balance: 100.00%

29/11/2023

Node Event US Peak Level Depth Inflow Node Flood **Status** (m) Node (mins) (m) (I/s) Vol (m³) (m³) 960 minute winter Basin B 930 89.439 0.639 7.2 138.2475 0.0000 OK

Link EventUSLinkOutflowDischarge(Upstream Depth)Node(I/s)Vol (m³)960 minute winterBasin BOrifice0.735.3



File: U321 Catchment B.pfd Network: Storm Network James Howard 29/11/2023 Page 5

Results for 30 year +35% CC Critical Storm Duration. Lowest mass balance: 100.00%

Node Event	US	Peak	Level	Depth	Inflow	Node	Flood	Status
	Node	(mins)	(m)	(m)	(I/s)	Vol (m³)	(m³)	
960 minute winter	Basin B	945	89.592	0.792	9.7	192.6304	0.0000	OK

Link Event	US	Link	Outflow	Discharge
(Upstream Depth)	Node		(I/s)	Vol (m³)
960 minute winter	Basin B	Orifice	0.7	39.6



File: U321 Catchment B.pfd Network: Storm Network James Howard Page 6

Results for 100 year Critical Storm Duration. Lowest mass balance: 100.00%

Node Event	US	Peak	Level	Depth	Inflow	Node	Flood	Status
	Node	(mins)	(m)	(m)	(I/s)	Vol (m³)	(m³)	
960 minute winter	Basin B	945	89.543	0.743	8.8	174.4584	0.0000	OK

Link Event	US	Link	Outriow	Discharge
(Upstream Depth)	Node		(I/s)	Vol (m³)
960 minute winter	Basin B	Orifice	0.7	38.3



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Results for 100 year +40% CC Critical Storm Duration. Lowest mass balance: 100.00%

Node Event	US	Peak	Level	Depth	Inflow	Node	Flood	Status
	Node	(mins)	(m)	(m)	(I/s)	Vol (m³)	(m³)	
960 minute winter	Basin B	945	89.736	0.936	12.4	251.7195	0.0000	OK

Link Event	US	Link	Outflow	Discharge
(Upstream Depth)	Node		(I/s)	Vol (m³)
960 minute winter	Basin B	Orifice	0.8	43.4



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Results for 100 year +40% CC +5% A Critical Storm Duration. Lowest mass balance: 100.00%

Node Event	US	Peak	Level	Depth	Inflow	Node	Flood	Status
	Node	(mins)	(m)	(m)	(I/s)	Vol (m³)	(m³)	
1440 minute winter	Basin B	1410	89.768	0.968	9.1	265.7810	0.0000	OK

Link Event	US	Link	Outflow	Discharge
(Upstream Depth)	Node		(I/s)	Vol (m³)
1440 minute winter	Basin B	Orifice	0.8	60.4

CAUSEWAY

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Design Settings

/ \ - /		Minimum Velocity (m/s) Connection Type Minimum Backdrop Height (m) Preferred Cover Depth (m) Include Intermediate Ground	Level Soffits 0.200 1.200 √
Maximum Time of Concentration (mins) Maximum Rainfall (mm/hr)	30.00 500.0	Enforce best practice design rules	

Nodes

Name	Area	Cover	Depth
	(ha)	Level	(m)
		(m)	
Road	0.220	89.000	1.500

Simulation Settings

Rainfall Methodology	FEH-22	Drain Down Time (mins)	240	30 year (l/s)	1.6
Summer CV	0.950	Additional Storage (m³/ha)	20.0	100 year (l/s)	2.1
Winter CV	0.950	Check Discharge Rate(s)	\checkmark	Check Discharge Volume	\checkmark
Analysis Speed	Normal	1 year (l/s)	0.6	100 year 360 minute (m³)	66
Skip Steady State	Х	2 year (I/s)	0.6	, , , ,	

Storm Durations

600	720	960	1440	2160	2880	4320	5760	7200	8640	10080
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Return Period (years)	Climate Change (CC %)	Additional Area (A %)	Additional Flow (Q %)
2	0	0	0
30	0	0	0
30	35	0	0
100	0	0	0
100	40	0	0

Pre-development Discharge Rate

Site Makeup	Greenfield	Growth Factor 30 year	2.40
Greenfield Method	IH124	Growth Factor 100 year	3.19
Positively Drained Area (ha)	0.220	Betterment (%)	0
SAAR (mm)	623	QBar	0.7
Soil Index	3	Q 1 year (I/s)	0.6
SPR	0.40	Q 2 year (I/s)	0.6
Region	6	Q 30 year (I/s)	1.6
Growth Factor 1 year	0.85	Q 100 year (I/s)	2.1
Growth Factor 2 year	0.88		

Pre-development Discharge Volume

Site Makeup	Greenfield	Return Period (years)	100
Greenfield Method	FSR/FEH	Climate Change (%)	0
Positively Drained Area (ha)	0.220	Storm Duration (mins)	360
Soil Index	3	Betterment (%)	0
SPR	0.40	PR	0.360
CWI	93.778	Runoff Volume (m³)	48



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Node Road Online Orifice Control

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Flap Valve x Invert Level (m) 87.500 Discharge Coefficient 0.600 Replaces Downstream Link ✓ Diameter (m) 0.020

Node Road Depth/Area Storage Structure

Base Inf Coefficient (m/hr) 0.00000 Safety Factor 2.0 Invert Level (m) 87.500 Side Inf Coefficient (m/hr) 0.00000 Porosity 1.00 Time to half empty (mins) 0

Depth Inf Area Depth Area Inf Area Area (m) (m²)(m²) (m) (m²)(m²)0.000 214.0 593.0 0.0 1.500 0.0



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Results for 2 year Critical Storm Duration. Lowest mass balance: 100.00%

Node Event	US	Peak	Level	Depth	Inflow	Node	Flood	Status
	Node	(mins)	(m)	(m)	(I/s)	Vol (m³)	(m³)	
960 minute winter	Road	915	87.704	0.204	2.7	49,4997	0.0000	OK

Link Event	US	Link	Outrlow	Discharge
(Upstream Depth)	Node		(I/s)	Vol (m³)
960 minute winter	Road	Orifice	0.4	18.3



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Results for 30 year Critical Storm Duration. Lowest mass balance: 100.00%

Node Event	US	Peak	Level	Depth	Inflow	Node	Flood	Status
	Node	(mins)	(m)	(m)	(I/s)	Vol (m³)	(m³)	
720 minute winter	Road	705	87.883	0.383	6.7	101.7468	0.0000	OK

Link Event	US	Link	Outflow	Discharge
(Upstream Depth)	Node		(I/s)	Vol (m³)
720 minute winter	Road	Orifice	0.5	21.1



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Results for 30 year +35% CC Critical Storm Duration. Lowest mass balance: 100.00%

Node Event	US	Peak	Level	Depth	Inflow	Node	Flood	Status
	Node	(mins)	(m)	(m)	(I/s)	Vol (m³)	(m³)	
960 minute winter	Road	945	88.003	0.503	7.1	140.9217	0.0000	OK

Link Event	US	Link	Outflow	Discharge
(Upstream Depth)	Node		(I/s)	Vol (m³)
960 minute winter	Road	Orifice	0.6	30.0



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Results for 100 year Critical Storm Duration. Lowest mass balance: 100.00%

Node Event	US	Peak	Level	Depth	Inflow	Node	Flood	Status
	Node	(mins)	(m)	(m)	(I/s)	Vol (m³)	(m³)	
960 minute winter	Road	945	87.965	0.465	6.5	128.1925	0.0000	OK

Link Event	US	Link	Outflow	Discharge
(Upstream Depth)	Node		(I/s)	Vol (m³)
960 minute winter	Road	Orifice	0.6	28.8



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Results for 100 year +40% CC Critical Storm Duration. Lowest mass balance: 100.00%

Node Event	US	Peak	Level	Depth	Inflow	Node	Flood	Status
	Node	(mins)	(m)	(m)	(I/s)	Vol (m³)	(m³)	
960 minute winter	Road	945	88.121	0.621	9.1	183.5923	0.0000	OK

Link Event	US	Link	Outflow	Discharge
(Upstream Depth)	Node		(I/s)	Vol (m³)
960 minute winter	Road	Orifice	0.7	33.6