

Hydrogeological Report

Land at the Rear of Sturt Avenue, Haslemere, Surrey GU27 3SJ

On behalf of Casa Coevo Group Limited

Report Reference: GWPR5705/HGR/November 2023			Status: Final
Issue	Prepared By	Checked By	Verified By
V1.01			
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Hydrogeological Report

REPORT REFERENCE	<p>GWPR5705/HGR/November 2023. The techniques adopted for the investigation were chosen considering the requirements of the client, anticipated ground conditions, the nature of the site and logistical limitations. This hydrogeological report is not a Flood Risk Assessment and should be read in conjunction with a Flood Risk Assessment. The conditions and limitations of this report can be viewed within Appendix A. A technical glossary has also been provided within Appendix B.</p>																								
SITE DETAILS	<p>The site comprised a ~0.69ha plot of land, situated to the rear (north) of the properties along the northern side of Sturt Avenue, along the southern side of a watercourse. A site location plan has been provided within Figure 1. A view of the site development area has been provided within Figure 2, with an aerial view of the site provided in Figure 3.</p>																								
PROPOSED DEVELOPMENT	<p>At the time of reporting, November 2023, the proposed development was understood to comprise the development of 9No. residential plots, each with car parking, soft landscaped areas and areas of hardstanding, all accessed by a new access route. A proposed development plan is displayed within Figure 4.</p>																								
ANTICIPATED GEOLOGY AND HYDROGEOLOGY	<p>The British Geological Survey (BGS) maps and DEFRA online maps for the area suggest that the site was located on Head Deposits, classified as a Secondary Undifferentiated Bedrock Aquifer, overlying the Hythe Formation, classified as a Principal Bedrock Aquifer, underlain by the Atherfield Clay Formation, classified as Unproductive Bedrock Strata. Alluvium was noted along the banks of the adjacent watercourse, classified as a Secondary A Superficial Aquifer.</p> <p>Based on this, it was anticipated that groundwater was perched on top of the Atherfield Clay Formation within the Hythe Formation; however, perched water may be encountered within the superficial deposits, especially when they are mainly cohesive and have granular bands (the granular bands will become saturated with perched water, especially after periods of prolonged or intense rainfall).</p>																								
SITE WORKS	<p>Site works were undertaken on the 27th November 2023 and comprised the hand auguring of 9No. Trial Pits (HA1 – HA9) to 2.00m bgl. A trial hole location plan can be viewed within Figure 5.</p>																								
GROUND CONDITIONS ENCOUNTERED	<p>A summary of the ground conditions encountered has been summarised in the following table. The trial hole logs can be seen within Appendix C.</p> <table border="1" data-bbox="376 1579 1489 2018"> <thead> <tr> <th colspan="4">Summary of Strata Encountered</th> </tr> <tr> <th>Strata</th> <th>Top Depth (m bgl)</th> <th>Base Depth (m bgl)</th> <th>Thickness (m)</th> </tr> </thead> <tbody> <tr> <td>TOPSOIL: Brown gravelly silty/clayey SAND. Sand was fine to coarse. Gravel was fine and medium, sub-angular to sub-rounded flint. <i>(HA1 – HA5 and HA7 – HA9 only)</i></td> <td>GL</td> <td>0.20 – 0.60</td> <td>0.20 – 0.60</td> </tr> <tr> <td>MADE GROUND: Brown gravelly silty/clayey SAND. Sand was fine to coarse. Gravel was fine and medium, sub-angular to sub-rounded flint (80%) and brick (20%). <i>(HA6 only)</i></td> <td>GL</td> <td>0.40</td> <td>0.40</td> </tr> <tr> <td>HEAD DEPOSITS: Brown sandy silty CLAY. Sand is fine. <i>(All trial holes)</i></td> <td>0.20 – 0.60</td> <td>1.80 – >2.00</td> <td>1.20 – >1.80</td> </tr> <tr> <td>HEAD DEPOSITS: Brown silty/clayey SAND. Sand is fine. <i>(HA7 only)</i></td> <td>1.80</td> <td>>2.00</td> <td>>0.20</td> </tr> </tbody> </table>	Summary of Strata Encountered				Strata	Top Depth (m bgl)	Base Depth (m bgl)	Thickness (m)	TOPSOIL: Brown gravelly silty/clayey SAND. Sand was fine to coarse. Gravel was fine and medium, sub-angular to sub-rounded flint. <i>(HA1 – HA5 and HA7 – HA9 only)</i>	GL	0.20 – 0.60	0.20 – 0.60	MADE GROUND: Brown gravelly silty/clayey SAND. Sand was fine to coarse. Gravel was fine and medium, sub-angular to sub-rounded flint (80%) and brick (20%). <i>(HA6 only)</i>	GL	0.40	0.40	HEAD DEPOSITS: Brown sandy silty CLAY. Sand is fine. <i>(All trial holes)</i>	0.20 – 0.60	1.80 – >2.00	1.20 – >1.80	HEAD DEPOSITS: Brown silty/clayey SAND. Sand is fine. <i>(HA7 only)</i>	1.80	>2.00	>0.20
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Hydrogeological Report

GROUNDWATER

Groundwater was encountered at depths of between 0.80m – 1.50m bgl within HA1 – HA7. No groundwater strikes were recorded within HA8 or HA9. Groundwater strikes may be obscured by the auguring process. A summary can be seen in the following table.

Summary of Groundwater Strikes	
Trial Hole	Groundwater Struck (m bgl)
HA1	1.30
HA2	1.30
HA3	1.50
HA4	0.80
HA5	0.80
HA6	0.80
HA7	0.80
HA8	No groundwater noted.
HA9	No groundwater noted.

Changes in groundwater level occur for a number of reasons including seasonal effects and variations in drainage. The investigation was undertaken in November 2023 when groundwater levels are likely to be approaching their annual maximum (highest elevation). Exact groundwater levels may only be determined through long term measurements from monitoring wells installed on-site.

CONCLUSIONS AND RECOMMENDATIONS

Based on the aquifer classification, the site itself has the potential to flood from groundwater, due to the site being located on a Secondary Undifferentiated Superficial Aquifer, underlain by a Principal Bedrock Aquifer, underlain by Unproductive Strata.

Due to the relatively low permeability rates of the cohesive soils encountered, the amount of groundwater was likely limited, with limited mobility horizontally and vertically through the cohesive soils. Limited sub-surface structures (i.e. foundations, services etc) were noted, which are not likely obstruct groundwater flow and cause ponding issues upstream.

Perched water may be encountered on top of the cohesive Head Deposits, within the shallow surface soils, especially after periods of prolonged or intense rainfall. This may cause localised surface water flooding from pluvial (rainfall) sources. This would be able to be mitigated by SUDS and/or drainage infrastructure.

All points above should be considered in final design with the help of a civil engineer/drainage designer.

FIGURES	4
APPENDIX A: Conditions and Limitations	5
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APPENDIX C: Trial Hole Logs	9

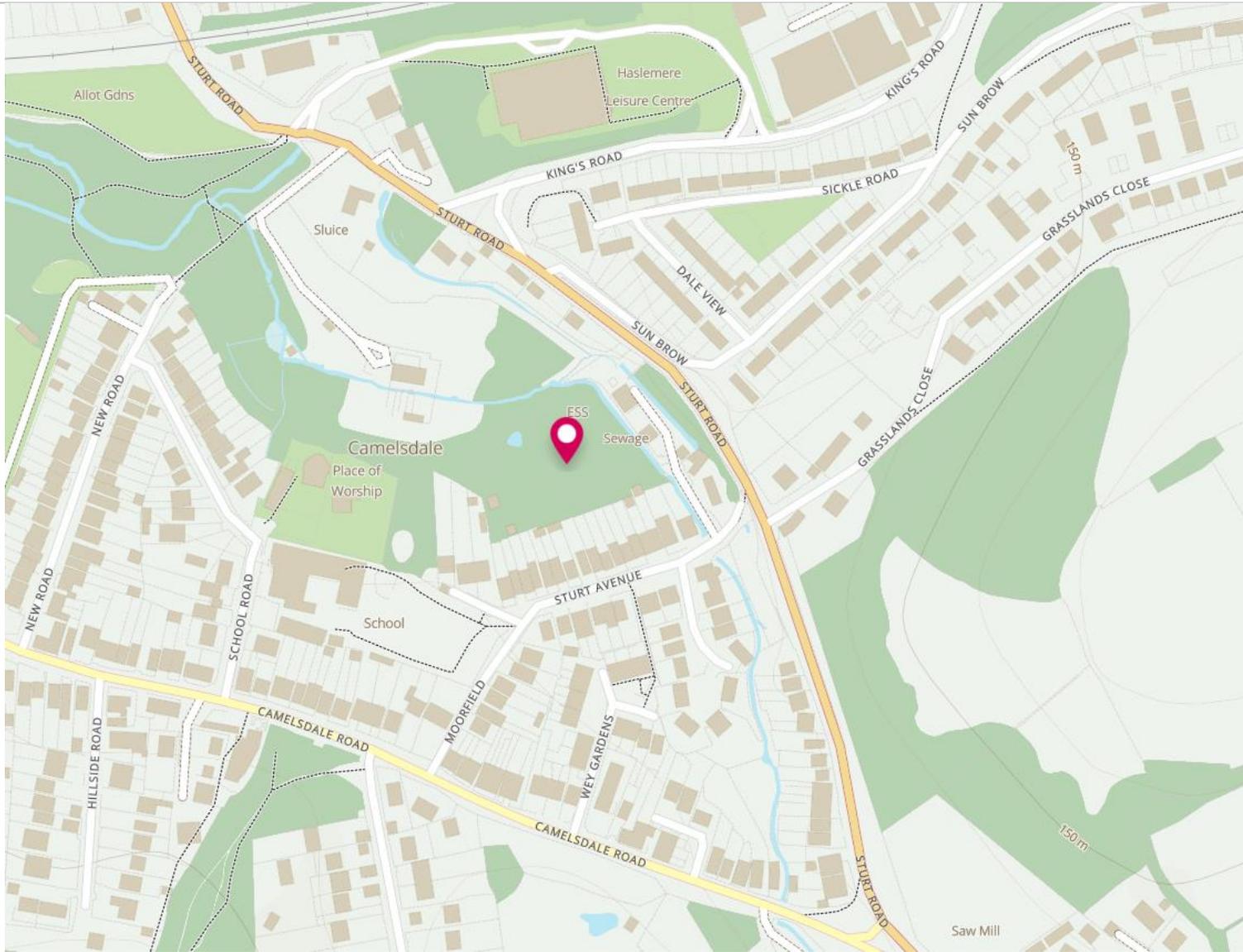


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FIGURES



 Site Location

Not to Scale

Land at the Rear of Sturt Avenue, Haslemere, Surrey GU27 3SJ

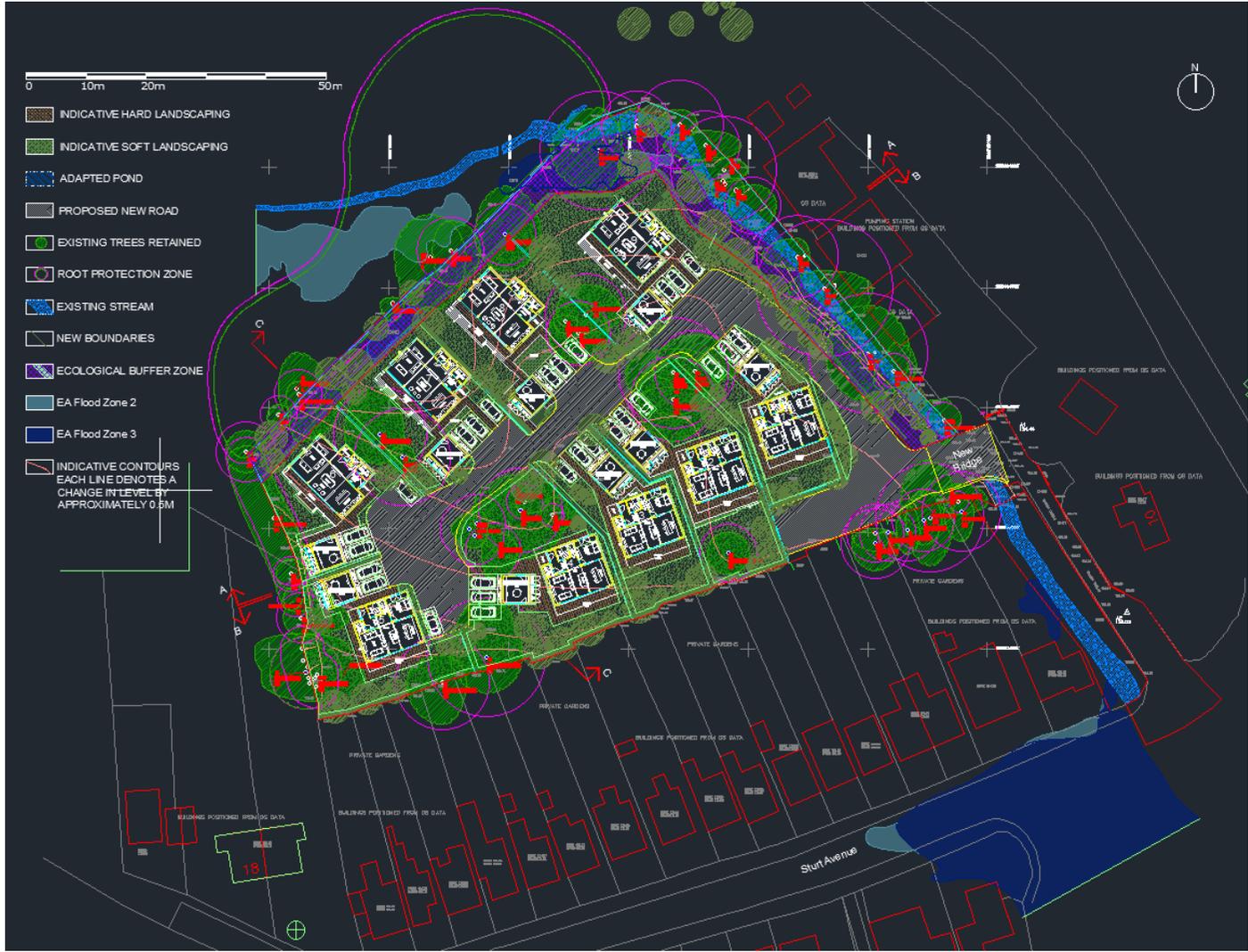
Casa Coevo Group Limited

November 2023

Figure 1: Site Location Plan

GWPR5705





Not to Scale

Land at the Rear of Sturt Avenue, Haslemere, Surrey GU27 3SJ

Casa Coevo Group Limited

November 2023

Figure 2: Proposed Development Plan

GWPR5705





-  Site boundary
-  Hand Auger

Not to Scale

Land at the Rear of Sturt Avenue, Haslemere, Surrey GU27 3SJ

Casa Coevo Group Limited

November 2023

Figure 3: Trial Hole Location Plan

GWPR5705



APPENDIX A: Conditions and Limitations

The ground is a product of continuing natural and artificial processes. As a result, the ground will exhibit a variety of characteristics that vary from place to place across a site, and also with time. Whilst a ground investigation will mitigate to a greater or lesser degree against the resulting risk from variation, the risks cannot be eliminated.

The report has been prepared on the basis of information, data and materials which were available at the time of writing. Accordingly any conclusions, opinions or judgements made in the report should not be regarded as definitive or relied upon to the exclusion of other information, opinions and judgements.

The investigation, interpretations, and recommendations given in this report were prepared for the sole benefit of the client in accordance with their brief; as such these do not necessarily address all aspects of ground behaviour at the site. No liability is accepted for any reliance placed on it by others unless specifically agreed in writing.

Any decisions made by you, or by any organisation, agency or person who has read, received or been provided with information contained in the report (“you” or “the Recipient”) are decisions of the Recipient and we will not make, or be deemed to make, any decisions on behalf of any Recipient. We will not be liable for the consequences of any such decisions.

Current regulations and good practice were used in the preparation of this report. An appropriately qualified person must review the recommendations given in this report at the time of preparation of the scheme design to ensure that any recommendations given remain valid in light of changes in regulation and practice, or additional information obtained regarding the site.

Any Recipient must take into account any other factors apart from the Report of which they and their experts and advisers are or should be aware. The information, data, conclusions, opinions and judgements set out in the report may relate to certain contexts and may not be suitable in other contexts. It is your responsibility to ensure that you do not use the information we provide in the wrong context.

This report is based on readily available geological records, the recorded physical investigation, the strata observed in the works, together with the results of completed site and laboratory tests. Whilst skill and care has been taken to interpret these conditions likely between or below investigation points, the possibility of other characteristics not revealed cannot be discounted, for which no liability can be accepted. The impact of our assessment on other aspects of the development required evaluation by other involved parties.

The opinions expressed cannot be absolute due to the limitations of time and resources within the context of the agreed brief and the possibility of unrecorded previous in ground activities. The ground conditions have been sampled or monitored in recorded locations and tests for some of the more common chemicals generally expected. Other concentrations of types of chemicals may exist. It was not part of the scope of this report to comment on environment/contaminated land considerations.

The conclusions and recommendations relate to the Rear of Sturt Avenue, Haslemere, Surrey GU27 3SJ.

Trial hole is a generic term used to describe a method of direct investigation. The term trial pit, borehole or window sampler borehole implies the specific technique used to produce a trial hole.

The depth to roots and/or of desiccation may vary from that found during the investigation. The client is responsible for establishing the depth to roots and/or of desiccation on a plot-by-plot basis prior to the construction of foundations. Where trees are mentioned in the text this means existing trees, recently removed trees (approximately 15 years to full recovery on cohesive soils) and those planned as part of the site landscaping.

Ownership of copyright of all printed material including reports, laboratory test results, trial pit and borehole log sheets, including drillers log sheets, remain with Ground and Water Limited. Licence is for the sole use of the client and may not be assigned, transferred or given to a third party.

Only our client may rely on this report and should this report or any information contained in it be provided to any third party we accept no responsibility to the third party for the contents of this report save to the extent expressly outlined by us in writing in a reliance letter addressed from us to the third party.

Recipients are not permitted to publish this report outside of their organisation without our express written consent.

The aim of the investigation was understood to be to supply the client and their designers with information regarding the ground conditions underlying the site to assist them in preparing an appropriate scheme for development.

APPENDIX B: Technical Glossary

TECHNICAL GLOSSARY

The list of possible definitions within the report may be seen below. Please note that some definitions may not be relevant to this report.

HYDROGEOLOGY:

A **Principal Aquifer** is a layer of rock or drift deposits that have high intergranular and/or fracture permeability - meaning they usually provide a high level of water storage. They may support water supply and/or river base flow on a strategic scale. In most cases, principal aquifers are aquifers previously designated as major aquifer.

Secondary (A) Aquifers consist of deposits with permeable layers capable of supporting water supplies at a local rather than strategic scale, and in some cases forming an important source of base flow to rivers. These are generally aquifers formerly classified as Minor Aquifers.

Secondary (B) Aquifers consist of deposits with predominantly lower permeability layers with may stoke and yield limited amounts of groundwater due to localised features such as fissures, think permeable horizons and weathering. These are generally the water-bearing parts of the former non-aquifers.

Secondary Aquifers (Undifferentiated) are assigned in cases where it has not been possible to attribute either category A or B to a rock type. In most cases, this means that the layer in question has previously been designated as both a minor aquifer and non-aquifer in different locations due to the variable characteristics of the rock type.

Unproductive Strata are rock layers with low permeability that have negligible significance for water supply or river base flow. These were formerly classified as non-aquifers.

FLOOD ZONES:

Environment Agency Flood Zone 2, defined as; land having between a 1 in 100 and 1 in 1,000 annual probability of river flooding; or land having between a 1 in 200 and 1 in 1,000 annual probability of sea flooding.

Environment Agency Flood Zone 3 shows the extent of a river flood with a 1 in 100 (1% or greater chance of occurring in any year or a sea flood with a 1 in 200 (0.5%) or greater chance of occurring in any year.

Environment Agency Flood Zone 3 area that benefits from flood defences, defined as; land and property in this flood zone would have a high probability of flooding without the local flood defences. These protect the area against a river flood with a 1% chance of happening each year, or a flood from the sea with a 0.5% chance of happening each year.

GROUNDWATER SOURCE PROTECTION ZONES (SPZS):

Inner Zone (SPZ1): This zone is 50 day travel time of pollutant to source with a 50 metres default minimum radius.

Outer Zone (SPZ2): This zone is 400 day travel time of pollutant to source. This has a 250 or 500 metres minimum radius around the source depending on the amount of water taken.

Total Catchment (SPZ3): This is the area around a supply source within which all the groundwater ends up at the abstraction point. This is the point from where the water is taken. This could extend some distance from the source point.

Zone of Special Interest (SPZ4): This zone is where local conditions require additional protection.

IN-SITU STRENGTH GEOTECHNICAL TESTING:

Windowless Sample and/or Cable Percussion and/or Rotary Boreholes provide samples of the ground for assessment but they do not give any engineering data. The standard penetration test (SPT) is an in-situ dynamic penetration test designed to provide information on the geotechnical engineering properties of soil. The test uses a thick-walled sample tube, with an outside diameter of 50mm and an inside diameter of 35mm, and a length of around 650mm. This is driven into the ground at the bottom of a borehole by blows from a slide hammer with a weight of 63.5kg falling through a distance of 760mm. The sample tube is driven 150mm into the ground and then the number of blows needed for the tube to penetrate each 75mm up to a depth of 450mm is recorded. The sum of the number of blows is termed the "standard penetration resistance" or the "N-value".

Dynamic Probing involves the driving of a metal cone into the ground via a series of steel rods. These rods are driven from the surface by a hammer system that lifts and drops a 63.5kg (SHDP) hammer onto the top of the rods through a set height, thus ensuring a consistent energy input. The number of hammer blows that are required to drive the cone down by each 100mm increment are recorded. These blow counts then provide a comparative assessment from which correlations have been published, based on dynamic energy, which permits engineering parameters to be generated. (The Dynamic Probe 'Super Heavy' (SHDP) Tests were conducted in accordance with BS 1377; 1990; Part 9, Clause 3.2).



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APPENDIX C: Trial Hole Logs



Trial Pit Log

Project Name: Land at the Rear of Sturt Avenue	Client: Casa Coevo Group Limited	Date: 27/11/2023
Location: Haslemere, Surrey GU27 3SJ	Contractor:	
Project No. : GWPR5705	Crew Name:	Equipment:

Location Number HA1	Location Type TP	Level	Logged By	Scale 1:25	Page Number Sheet 1 of 1
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Well	Water Strikes	Sample and In Situ Testing			Depth (m)	Level (m)	Legend	Stratum Description	
		Depth (m)	Type	Results					
▼					0.50		TOPSOIL: Brown gravelly silty/clayey SAND. Sand was fine to coarse. Gravel was fine and medium, sub-angular to sub-rounded flint.	1	
					2.00		HEAD DEPOSITS: Brown sandy silty CLAY. Sand is fine.	2	
							End of Borehole at 2.000m	3	
								4	
								5	

Dimensions		Trench Support and Comment			Pumping Data		
Pit Length	Pit Width	Pit Stability	Shoring Used	Remarks	Date	Rate	Remarks

Remarks
Groundwater standing at 1.30m bgl after the conclusion of hand auguring.





Trial Pit Log

Project Name: Land at the Rear of Sturt Avenue	Client: Casa Coevo Group Limited	Date: 27/11/2023
Location: Haslemere, Surrey GU27 3SJ	Contractor:	
Project No. : GWPR5705	Crew Name:	Equipment:

Location Number HA2	Location Type TP	Level	Logged By	Scale 1:25	Page Number Sheet 1 of 1
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Well	Water Strikes	Sample and In Situ Testing			Depth (m)	Level (m)	Legend	Stratum Description	
		Depth (m)	Type	Results					
					0.20		TOPSOIL: Brown gravelly silty/clayey SAND. Sand was fine to coarse. Gravel was fine and medium, sub-angular to sub-rounded flint. HEAD DEPOSITS: Brown sandy silty CLAY. Sand is fine.	1	
	▼				2.00		End of Borehole at 2.000m	2	
								3	
								4	
								5	

Dimensions		Trench Support and Comment			Pumping Data		
Pit Length	Pit Width	Pit Stability	Shoring Used	Remarks	Date	Rate	Remarks

Remarks
Groundwater standing at 1.30m bgl after the conclusion of hand auguring.





Trial Pit Log

Project Name: Land at the Rear of Sturt Avenue
 Client: Casa Coevo Group Limited
 Date: 27/11/2023
 Location: Haslemere, Surrey GU27 3SJ
 Contractor:
 Project No. : GWPR5705
 Crew Name:
 Equipment:

Location Number: HA3
 Location Type: TP
 Level:
 Logged By:
 Scale: 1:25
 Page Number: Sheet 1 of 1

Well	Water Strikes	Sample and In Situ Testing			Depth (m)	Level (m)	Legend	Stratum Description
		Depth (m)	Type	Results				
▼					0.40		TOPSOIL: Brown gravelly silty/clayey SAND. Sand was fine to coarse. Gravel was fine and medium, sub-angular to sub-rounded flint.	
					2.00		HEAD DEPOSITS: Brown sandy silty CLAY. Sand is fine.	
							End of Borehole at 2.000m	

Dimensions		Trench Support and Comment			Pumping Data		
Pit Length	Pit Width	Pit Stability	Shoring Used	Remarks	Date	Rate	Remarks

Remarks
 Groundwater standing at 1.50m bgl after the conclusion of hand auguring.





Trial Pit Log

Project Name: Land at the Rear of Sturt Avenue	Client: Casa Coevo Group Limited	Date: 27/11/2023
Location: Haslemere, Surrey GU27 3SJ	Contractor:	
Project No. : GWPR5705	Crew Name:	Equipment:

Location Number HA4	Location Type TP	Level	Logged By	Scale 1:25	Page Number Sheet 1 of 1
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Well	Water Strikes	Sample and In Situ Testing			Depth (m)	Level (m)	Legend	Stratum Description	
		Depth (m)	Type	Results					
▼					0.50		TOPSOIL: Brown gravelly silty/clayey SAND. Sand was fine to coarse. Gravel was fine and medium, sub-angular to sub-rounded flint.	1	
					2.00		HEAD DEPOSITS: Brown sandy silty CLAY. Sand is fine.	2	
							End of Borehole at 2.000m	3	
								4	
								5	

Dimensions		Trench Support and Comment			Pumping Data		
Pit Length	Pit Width	Pit Stability	Shoring Used	Remarks	Date	Rate	Remarks

Remarks
Groundwater standing at 0.80m bgl after the conclusion of hand auguring.

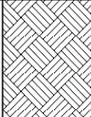
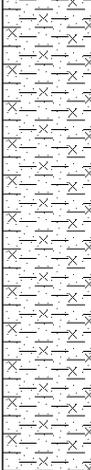




Trial Pit Log

Project Name: Land at the Rear of Sturt Avenue	Client: Casa Coevo Group Limited	Date: 27/11/2023
Location: Haslemere, Surrey GU27 3SJ	Contractor:	
Project No. : GWPR5705	Crew Name:	Equipment:

Location Number HA5	Location Type TP	Level	Logged By	Scale 1:25	Page Number Sheet 1 of 1
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Well	Water Strikes	Sample and In Situ Testing			Depth (m)	Level (m)	Legend	Stratum Description	
		Depth (m)	Type	Results					
	▼				0.40		 TOPSOIL: Brown gravelly silty/clayey SAND. Sand was fine to coarse. Gravel was fine and medium, sub-angular to sub-rounded flint.		
					2.00		 HEAD DEPOSITS: Brown sandy silty CLAY. Sand is fine.	1	
							End of Borehole at 2.000m	2	
								3	
								4	
								5	

Dimensions		Trench Support and Comment			Pumping Data		
Pit Length	Pit Width	Pit Stability	Shoring Used	Remarks	Date	Rate	Remarks

Remarks
Groundwater standing at 0.80m bgl after the conclusion of hand auguring.





Trial Pit Log

Project Name: Land at the Rear of Sturt Avenue	Client: Casa Coevo Group Limited	Date: 27/11/2023
Location: Haslemere, Surrey GU27 3SJ	Contractor:	
Project No. : GWPR5705	Crew Name:	Equipment:

Location Number HA6	Location Type TP	Level	Logged By	Scale 1:25	Page Number Sheet 1 of 1
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Well	Water Strikes	Sample and In Situ Testing			Depth (m)	Level (m)	Legend	Stratum Description	
		Depth (m)	Type	Results					
▼					0.40		MADE GROUND: Brown gravelly silty/clayey SAND. Sand was fine to coarse. Gravel was fine and medium, sub-angular to sub-rounded flint (80%) and brick (20%).	1	
					2.00		HEAD DEPOSITS: Brown sandy silty CLAY with grey mottling. Sand is fine.	2	
							End of Borehole at 2.000m	3	
								4	
								5	

Dimensions		Trench Support and Comment			Pumping Data		
Pit Length	Pit Width	Pit Stability	Shoring Used	Remarks	Date	Rate	Remarks

Remarks
Groundwater standing at 0.80m bgl after the conclusion of hand auguring.





Trial Pit Log

Project Name: Land at the Rear of Sturt Avenue
 Client: Casa Coevo Group Limited
 Date: 27/11/2023
 Location: Haslemere, Surrey GU27 3SJ
 Contractor:
 Project No. : GWPR5705
 Crew Name:
 Equipment:

Location Number: HA7
 Location Type: TP
 Level:
 Logged By:
 Scale: 1:25
 Page Number: Sheet 1 of 1

Well	Water Strikes	Sample and In Situ Testing			Depth (m)	Level (m)	Legend	Stratum Description
		Depth (m)	Type	Results				
▼					0.60		TOPSOIL: Brown gravelly silty/clayey SAND. Sand was fine to coarse. Gravel was fine and medium, sub-angular to sub-rounded flint.	
					1.80		HEAD DEPOSITS: Brown sandy silty CLAY. Sand is fine.	
					2.00		HEAD DEPOSITS: Brown silty/clayey SAND. Sand was fine.	
							End of Borehole at 2.000m	

Dimensions		Trench Support and Comment			Pumping Data		
Pit Length	Pit Width	Pit Stability	Shoring Used	Remarks	Date	Rate	Remarks

Remarks
 Groundwater standing at 0.80m bgl after the conclusion of hand auguring.





Trial Pit Log

Project Name: Land at the Rear of Sturt Avenue	Client: Casa Coevo Group Limited	Date: 27/11/2023
Location: Haslemere, Surrey GU27 3SJ	Contractor:	
Project No. : GWPR5705	Crew Name:	Equipment:

Location Number HA8	Location Type TP	Level	Logged By	Scale 1:25	Page Number Sheet 1 of 1
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Well	Water Strikes	Sample and In Situ Testing			Depth (m)	Level (m)	Legend	Stratum Description	
		Depth (m)	Type	Results					
					0.50		TOPSOIL: Brown gravelly silty/clayey SAND. Sand was fine to coarse. Gravel was fine and medium, sub-angular to sub-rounded flint.	1	
					2.00		HEAD DEPOSITS: Brown sandy silty CLAY. Sand is fine.	2	
							End of Borehole at 2.000m	3	
								4	
								5	

Dimensions		Trench Support and Comment			Pumping Data		
Pit Length	Pit Width	Pit Stability	Shoring Used	Remarks	Date	Rate	Remarks

Remarks
No groundwater encountered.



Trial Pit Log

Project Name: Land at the Rear of Sturt Avenue	Client: Casa Coevo Group Limited	Date: 27/11/2023
Location: Haslemere, Surrey GU27 3SJ	Contractor:	
Project No. : GWPR5705	Crew Name:	Equipment:

Location Number HA9	Location Type TP	Level	Logged By	Scale 1:25	Page Number Sheet 1 of 1
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Well	Water Strikes	Sample and In Situ Testing			Depth (m)	Level (m)	Legend	Stratum Description	
		Depth (m)	Type	Results					
					0.40			TOPSOIL: Brown gravelly silty/clayey SAND. Sand was fine to coarse. Gravel was fine and medium, sub-angular to sub-rounded flint.	
					2.00			HEAD DEPOSITS: Brown sandy silty CLAY. Sand is fine.	1
								End of Borehole at 2.000m	2
									3
									4
									5

Dimensions		Trench Support and Comment			Pumping Data		
Pit Length	Pit Width	Pit Stability	Shoring Used	Remarks	Date	Rate	Remarks

Remarks
No groundwater encountered.

