

OFFICE: SITE ADDRESS:

DPA Planning Ltd Part of Plot 3
4100 Park Approach Delta Enterprise Park
Leeds Goole
LS15 8GB DN14 8JZ

Tel: 0113 3970 310
Mob: 07799 095 613

S.dewar@dpaplanning.co.uk

Notes:

Site to accommodate up to 92 self-storage containers.

Each container measures 6.09m long by 2.43m wide with a flat roof height of 2.62m.

N N

By Paper Scale Dwg no. Rev
2024 SD AO 1:250 003

Layout Plan over TOPO



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Andrew Moseley Associates, 15 St Paul's Street, Leeds, LS1 2JG

www.amatp.co.uk



## Appendix L

YORKSHIRE WATER PRE DEVELOPMENT ENQUIRY



Ms J Ellenor
Andrew Moseley Associates
15 St Pauls Street
Leeds
LS1 2JG
jasmine@amatp.co.uk

Yorkshire Water Services
Developer Services
Pre-Development Team
PO BOX 52
Bradford
BD3 7AY

Tel: 0345 120 8482

Fax:

**Your Ref:** 

Our Ref: A000075

Email:

technical.sewerage@yorkshirewater.co.uk

For telephone enquiries ring: Chris Roberts on 0345 120 8482

16th January 2024

Dear Ms J Ellenor,

#### Holt Business Park, East Riding, DN14 8JU - Pre-planning Enquiry V361452

Thank you for your recent enquiry and remittance. Our official VAT receipt has been sent to you under separate cover. Please find enclosed a complimentary extract from the Statutory Sewer Map which indicates the recorded position of the public sewers. Please note that as of October 2011 and the private to public sewer transfer, there are many uncharted Yorkshire Water assets currently not shown on our records.

The following comments reflect our view, with regard to the public sewer network only, based on a 'desk top' study of the site and are valid for a maximum period of twelve months:

#### **Foul Water**

The site is not within an area served by public sewers. The closest suitable public sewer is the 225 mm foul public sewer approximately 850 metres to the north east of the site in Airmyn Road.

Foul water from kitchens and/or food preparation areas of any restaurants and/or canteens etc. must pass through a fat and grease trap of adequate design before any discharge to the public sewer network.

#### **Surface Water**

The developer's attention is drawn to Requirement H3 of the Building Regulations 2010. This establishes a preferred hierarchy for surface water disposal. Consideration should firstly be given to discharge to soakaway, infiltration system and watercourse in that priority order.

Sustainable Drainage Systems (SuDS), for example the use of soakaways and/or permeable hardstanding etc, may be a suitable solution for surface water disposal appropriate in this situation. You are advised to seek comments on the suitability of SuDS in this instance from the appropriate authorities.







It is understood that watercourse are located around the site. This appears to be the obvious place for surface water disposal (if SuDS are not viable). Please note Yorkshire Water cannot provide plans of culverted watercourses or highway drains. To obtain plans please contact the Lead Local Flood Authority for more details.

Surface water run-off from communal parking (greater than 800 sq metres or more than 50 car parking spaces) and hardstanding must pass through an oil, petrol and grit interceptor/separator of adequate design before any discharge to the public sewer network. Roof water should not pass through the traditional 'stage' or full retention type of interceptor/separator. It is good drainage practice for any interceptor/separator to be located upstream of any on-site balancing, storage or other means of flow attenuation that may be required.

Surface water run-off from areas of vehicular parking and/or hardstanding etc. must pass through an oil, petrol and grit interceptor/separator of adequate design before any discharge to the public sewer network. Roof water should not pass through the traditional 'stage' or full retention type of interceptor/separator.

It is imperative, however that surface water run-off from the forecourt of petrol stations, areas used for the delivery of fuel, areas used for and immediately adjacent to vehicle washing facilities and/or other similar areas where detergent is likely to be used is not discharged to any public surface water sewer network. Surface water from such areas must pass through an oil, petrol and grit interceptor/separator of adequate design before discharge to the public foul or combined sewer network. A trade effluent consent - that may be conditional and, amongst other things, place a restriction on the rate of discharge to public sewer - may be required for such discharges. The developer is advised to contact Yorkshire Water's Industrial Waste Section (telephone 0345 1242424) about any such proposal.

It is good drainage practice for any interceptor/separator to be located upstream of any on-site balancing, storage or other means of flow attenuation that may be required.

#### **Other Observations**

Any new connection to an existing public sewer will require the prior approval of Yorkshire Water. You may apply on line or obtain an application form from our website - https://www.yorkshirewater.com/developers/sewerage/sewerage-connections/

Yorkshire Water's Trade Effluent team must be consulted in respect of any proposed trade effluent discharge to the public sewer. Please visit - https://www.yorkshirewater.com/business/trade-effluent/

All the above comments are based upon the information and records available at the present time and is subject to formal planning approval agreement. The information contained in this letter together with that shown on any extract from the Statutory Sewer Map that may be enclosed is believed to be correct and is supplied in good faith.







Please note that capacity in the public sewer network is not reserved for specific future development. It is used up on a 'first come, first served' basis. You should visit the site and establish the line and level of any public sewers affecting your proposals before the commencement of any design work.

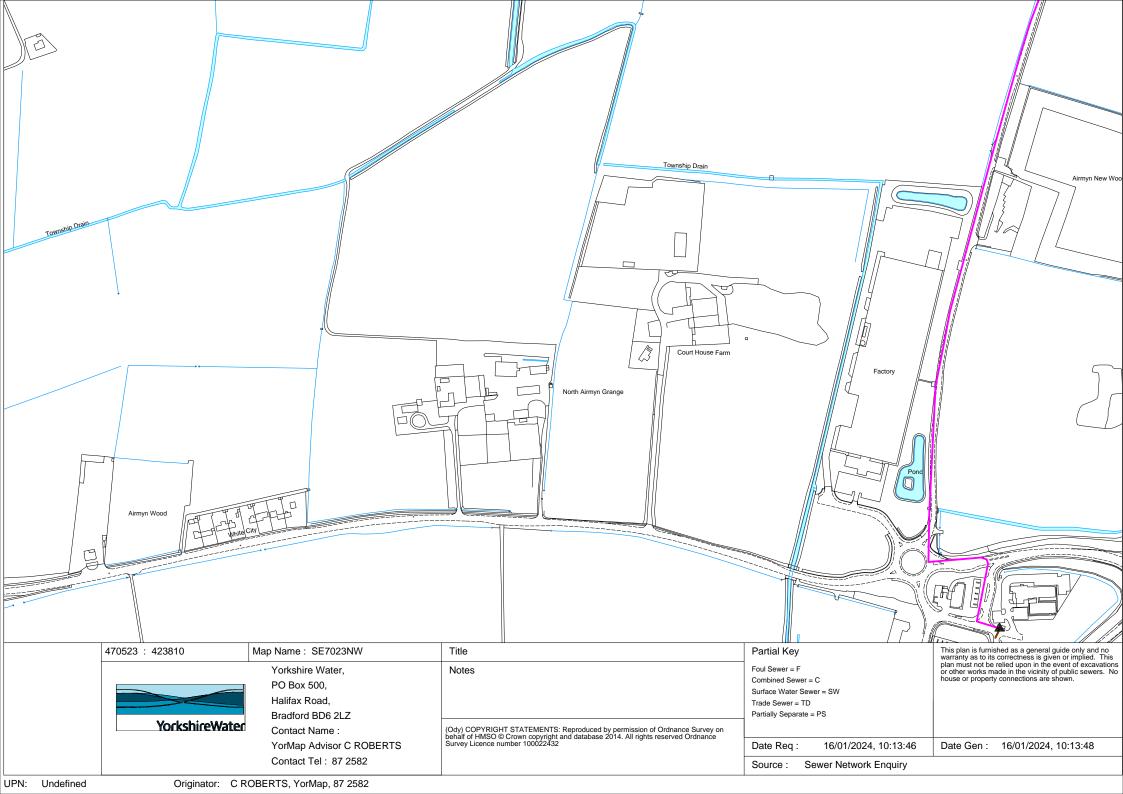
Yours sincerely

Chris Roberts

Development Services Technician









## Appendix M

**UK SUDS GREENFIELD RUN OFF RATES** 



### Greenfield runoff rate estimation for sites

www.uksuds.com | Greenfield runoff tool

Calculated by:	Aaron Yesudian
Site name:	Delta Enterprise Park
Site location:	Goole

Site Details

53.70803° N Latitude: 0.91951° W

Longitude:

608874908

This is an estimation of the greenfield runoff rates that are used to meet normal best practice Reference: criteria in line with Environment Agency guidance "Rainfall runoff management for developments", SC030219 (2013), the SuDS Manual C753 (Ciria, 2015) and the non-statutory standards for SuDS (Defra, 2015). This information on greenfield runoff rates may be the basis for setting consents for the drainage of surface water runoff from sites.

Jan 24 2024 15:19

### Runoff estimation approach

IH124

Site characteristics

Total site area (ha):

Methodology

**QBAR** estimation method:

Calculate from SPR and SAAR

SPR estimation method:

Calculate from SOIL type

Notes

(1) Is  $Q_{BAR} < 2.0 \text{ I/s/ha}$ ?

When QBAR is < 2.0 l/s/ha then limiting discharge rates are set at 2.0 l/s/ha.

#### Soil characteristics

Dofault

Fditad

SOIL type:

**HOST class:** 

SPR/SPRHOST:

Derault	Luiteu
4	4
N/A	N/A
0.47	0.47

(2) Are flow rates < 5.0 l/s?

Where flow rates are less than 5.0 l/s consent for discharge is usually set at 5.0 l/s if blockage from vegetation and other materials is possible. Lower consent flow rates may be set where the blockage risk is addressed by using appropriate drainage elements.

#### Hydrological characteristics

SAAR (mm):

Hydrological region:

Growth curve factor 1 year.

Growth curve factor 30 years:

Growth curve factor 100 years:

Growth curve factor 200 years:

Default Edited 596 596

3 3

0.86 0.86

1.75 1.75

2.08 2.08

2.37 2.37 (3) Is  $SPR/SPRHOST \le 0.3$ ?

Where groundwater levels are low enough the use of soakaways to avoid discharge offsite would normally be preferred for disposal of surface water runoff.

Q <sub>BAR</sub> (I/s):	1.6	1.6
1 in 1 year (l/s):	1.38	1.38
1 in 30 years (I/s):	2.8	2.8
1 in 100 year (I/s):	3.33	3.33
1 in 200 years (l/s):	3.79	3.79

This report was produced using the greenfield runoff tool developed by HR Wallingford and available at www.uksuds.com. The use of this tool is subject to the UK SuDS terms and conditions and licence agreement, which can both be found at www.uksuds.com/terms-and-conditions.htm. The outputs from this tool are estimates of greenfield runoff rates. The use of these results is the responsibility of the users of this tool. No liability will be accepted by HR Wallingford, the Environment Agency, CEH, Hydrosolutions or any other organisation for the use of this data in the design or operational characteristics of any drainage scheme.



# Appendix N

**CAUSEWAY FLOW CALCULATIONS** 

**CAUSEWAY** 

Jasmine Ellenor 24/01/2024

Page 1

#### **Design Settings**

Rainfall Methodology	FSR	Maximum Time of Concentration (mins)	30.00
Return Period (years)	100	Maximum Rainfall (mm/hr)	50.0
Additional Flow (%)	0	Minimum Velocity (m/s)	1.00
FSR Region	<b>England and Wales</b>	Connection Type	<b>Level Soffits</b>
M5-60 (mm)	20.000	Minimum Backdrop Height (m)	0.200
Ratio-R	0.400	Preferred Cover Depth (m)	1.200
CV	1.000	Include Intermediate Ground	$\checkmark$
Time of Entry (mins)	5.00	Enforce best practice design rules	$\checkmark$

#### **Nodes**

Name			Cover Level (m)	Diameter (mm)	Easting (m)	Northing (m)	Depth (m)
AT	0.400	5.00	100.000	1200	100.000	100.000	1.500

#### **Simulation Settings**

Rainfall Methodology	FSR	Analysis Speed	Normal
FSR Region	<b>England and Wales</b>	Skip Steady State	Χ
M5-60 (mm)	20.000	Drain Down Time (mins)	240
Ratio-R	0.400	Additional Storage (m³/ha)	20.0
Summer CV	1.000	Check Discharge Rate(s)	Χ
Winter CV	1.000	Check Discharge Volume	Χ

#### **Storm Durations**

15	30	60   120	180 240	360 480	600 720 960 14	140
		Return Period	Climate Change	Additional Area	Additional Flow	

Return Period (years)	Climate Change (CC %)	Additional Area (A %)	Additional Flow (Q %)
1	0	0	0
20	0	0	0
100	30	0	0

#### Node AT Online Hydro-Brake® Control

Flap Valve	X	Objective	(HE) Minimise upstream storage
Replaces Downstream Link	X	Sump Available	$\checkmark$
Invert Level (m)	98.500	Product Number	CTL-SHE-0054-1600-1500-1600
Design Depth (m)	1.500	Min Outlet Diameter (m)	0.075
Design Flow (I/s)	1.6	Min Node Diameter (mm)	1200

#### **Node AT Carpark Storage Structure**

Base Inf Coefficient (m/hr)	0.00000	Invert Level (m)	98.500	Slope (1:X)	500.0
Side Inf Coefficient (m/hr)	0.00000	Time to half empty (mins)		Depth (m)	1.000
Safety Factor	2.0	Width (m)	66.500	Inf Depth (m)	
Porosity	1.00	Length (m)	5.000		



#### Andrew Moseley Associates Ltc

File: attenuation.pfd Network: Storm Network Jasmine Ellenor 24/01/2024 Page 2

#### Results for 1 year Critical Storm Duration. Lowest mass balance: 99.99%

Node Event	US	Peak	Level	Depth	Inflow	Node	Flood	Status
	Node	(mins)	(m)	(m)	(l/s)	Vol (m³)	(m³)	
960 minute summer	AT	690	98.721	0.221	7.5	73.3450	0.0000	OK

Link Event	US	Link	Outflow	Discharge
(Upstream Depth)	Node		(I/s)	Vol (m³)
960 minute summer	AT	Hydro-Brake®	1.2	62.2



#### Andrew Moseley Associates Ltc

File: attenuation.pfd Network: Storm Network Jasmine Ellenor 24/01/2024 Page 3

#### Results for 20 year Critical Storm Duration. Lowest mass balance: 99.99%

**Node Event** US Peak Level Depth Inflow Node Flood **Status** Node (mins) (m) (m) (I/s) Vol (m³) (m³) 960 minute summer ΑT 960 99.011 0.511 14.5 171.5206 0.0000 OK

Link EventUSLinkOutflowDischarge(Upstream Depth)Node(I/s)Vol (m³)960 minute summerATHydro-Brake®1.262.2



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File: attenuation.pfd Network: Storm Network Jasmine Ellenor 24/01/2024 Page 4

#### Results for 100 year +30% CC Critical Storm Duration. Lowest mass balance: 99.99%

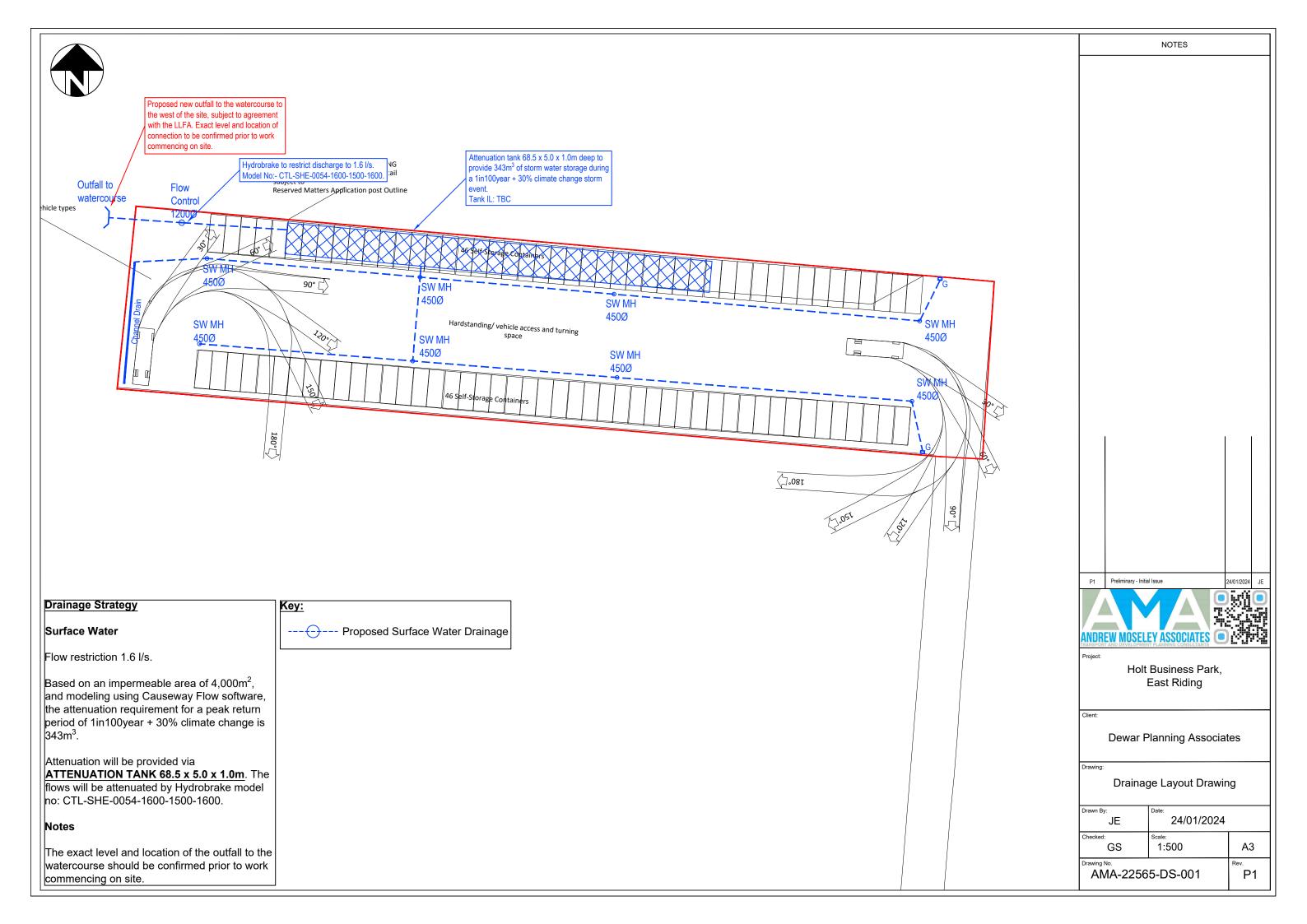
**Node Event** US Peak Level Depth Inflow Node Flood Status Node (mins) (m) (I/s) Vol (m³) (m³) (m) 1440 minute winter ΑT 1380 99.998 1.498 12.5 340.6844 0.0000 OK

Link EventUSLinkOutflowDischarge(Upstream Depth)Node(I/s)Vol (m³)1440 minute winterATHydro-Brake®1.6110.5



# Appendix O

DRAINAGE LAYOUT DRAWING





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