



Foul and Surface Water Drainage Strategy

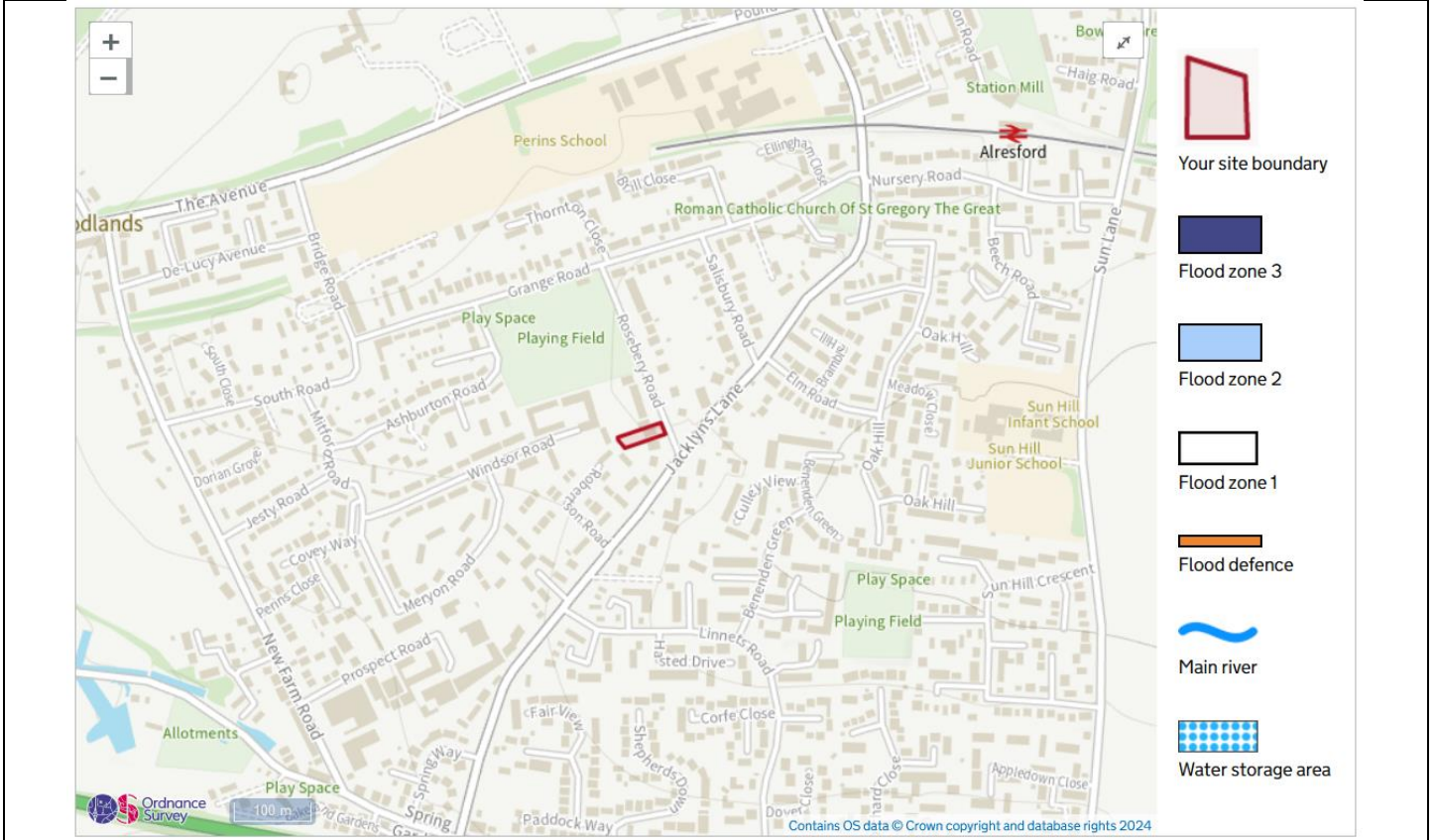
Project name:	30 Rosebery Road
Project number:	D2234
Client:	Mr. & Mrs. J Phare
Prepared by and date:	S Burnett, January 2024
Site address:	30 Rosebery Road, New Alresford, Winchester, Hampshire SO24 9HQ
Grid Reference:	SU 58398 32051 / SU583320
Eastings / Northings:	458398 / 132051
Local Authority:	Winchester City Council
Lead Local Flood Authority:	Hampshire County Council
Proposed Development:	<p>The proposed development involves the extension of an existing single-storey bungalow to provide a new first floor, in addition to the construction of a new single-storey garage.</p> <p>The architectural development proposal is contained within Appendix A.</p>
Site area:	930m ² (0.093Ha)
Topography	<p>J. Brotherton & Partners has undertaken a topographical survey at the site, dated April 2023.</p> <p>Levels are demonstrated as falling within the range of 90.63m AOD and 87.78m AOD, broadly falling in a south/south-westerly direction.</p> <p>Rosebery Road falls in a southerly direction, towards B3046.</p>
Greenfield run-off rate:	<p>The following greenfield run-off rates have been established in relation to the site area of 930m² (0.093Ha):</p> <p>$Q_{BAR} = 0.0186$ l/s 1 year = 0.0186 l/s 30 year = 0.0465 l/s 100 year = 0.0558 l/s</p> <p>The above figures are based on a pro-rata calculation utilising HR Wallingford's greenfield run-off tool, which considers a minimum site area of 0.1Ha.</p>
Brownfield run-off rate:	<p>A brownfield run-off rate has been established in relation to the existing impermeable area at the site, which totals 305m².</p> <p>Based on a 'flat rainfall rate' methodology, with a rainfall intensity of 50mm an hour, the existing run-off from the site in its current use is 4.236 l/s.</p>
Geology:	<p>Reference has been made to the British Geological Survey (BGS) website, and specifically the Geology of Britain Viewer. This tool confirms the following:</p> <p>Bedrock Geology: Newhaven Chalk Formation - Chalk Superficial Deposits: None</p> <p>Mapping extracts are contained within Appendix B.</p>
Site Specific Investigation:	The BGS provides records of historic onshore boreholes. Reference has been made to this tool in the absence of site-specific intrusive investigation.

	<p>The nearest borehole where records are available is located c. 260m north-east of the site (Ref: SU53SE79). The records for this location do not state the encountered geology, however.</p> <p>Attempts have also been made to review logs c. 430m north of the site (SU53SE86), although these records are confidential and not publicly available.</p> <p>Mapping extracts and records are contained within Appendix C.</p>
Aquifer Designation:	<p>For the following four sections, reference has been made to the MAGIC website (www.magic.defra.gov.uk) which provides authoritative geographic information about the natural environment from across government, in an interactive mapping tool format. Mapping extracts are contained within Appendix D.</p> <p>Bedrock: Principal Superficial Drift: Unproductive</p>
Groundwater Vulnerability Zone:	'High' with a 'Soluble Rock Risk' identified
Groundwater Source Protection Zone:	None
Drinking Water Safeguarding Zone (Surface Water and/or Groundwater):	None
Groundwater depth:	<p>Groundwater investigation has not been undertaken at the site.</p> <p>The BGS borehole records reviewed in relation to SU53SE79 confirms a ground level of 314.15ft AOD (95.75m AOD), with a rest water level of 307ft AOD (93.57m AOD).</p> <p>Groundwater investigation should be undertaken on site to confirm accurate groundwater levels.</p>
Soil infiltration rate:	<p>Infiltration testing has not been undertaken at the site.</p> <p>An assumed infiltration rate of 1×10^{-6} has been used for preliminary design purposes, in accordance with the typical figures provided in CIRIA 156, Table 4.4.</p> <p>Infiltration testing shall be undertaken to assist in the design of soakaways, post planning.</p>
Nearby watercourses/rivers:	There are no ordinary watercourses within the immediate vicinity of the site.
Nearby surface water sewer:	<p>Southern Water's public sewer records are contained within Appendix E.</p> <p>The records do not demonstrate any dedicated public surface water sewers within the vicinity of the site.</p> <p>A 100mm dia. public foul water sewer is located beneath Rosebery Road, which is shown to flow in a southerly direction, communicating with a 150mm dia. public foul water sewer beneath B3046, flowing in a westerly direction.</p>
Additional drainage features	Rosebery Road is drained by traditional road gullies, located along the western channel line. It is assumed that these gullies discharge to a dedicated highway drainage system, although it has not been possible to confirm this at this stage.
Proposed method of disposal and reason:	<p>The proposed development involves the construction of a first-floor extension to the existing single-storey bungalow, in addition to the construction of a new single-storey garage.</p> <p>The existing property will continue to drain as existing, although the client has expressed an interest in utilising a simple rainwater recycling system. There is to be no increase in the footprint / roof area associated with the existing bungalow further to the extension being implemented, therefore no increase in the drained area or catchment.</p> <p>The proposed garage will drain to a dedicated soakaway, serving the roof catchment of 22m², subject to ground investigation and infiltration testing.</p> <p>If infiltration is found to be unviable for whatever reason, the garage will drain as per the existing arrangement of the existing bungalow.</p>
Design storms considered:	1:10 Year (in accordance with Building Regulations, Approved Document H (H3, 3.2.7))

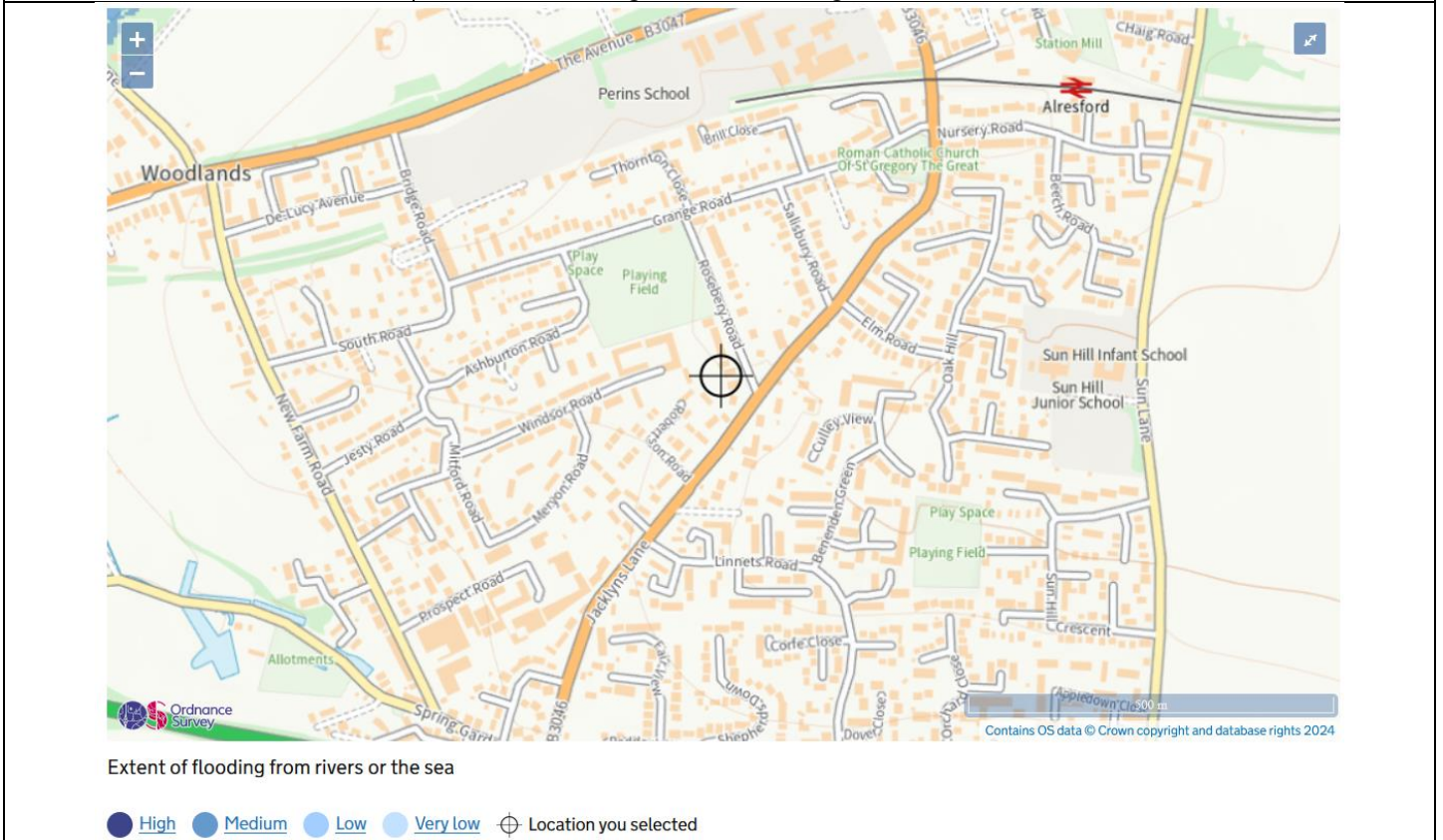
	The design considers the proposed new garage; the roof area associated with the existing bungalow is not increasing in view of the proposed development.
Climate change:	45% The design considers the proposed new garage; the roof area associated with the existing bungalow is not increasing in view of the proposed development.
Urban creep:	10% An allowance of 10% has been made for the potential future impacts of urban creep.
Proposed catchment areas:	Garage: 22m ² There will be no change to the drained area or catchment associated with the existing bungalow in view of the proposed development. The proposal concerns construction of a first-floor on the existing bungalow.
Proposed discharge rate:	An assumed infiltration rate of 1×10^{-6} has been used for preliminary design purposes, in accordance with the typical figures provided in CIRIA 156, Table 4.4. Infiltration testing shall be undertaken to assist in the design of soakaways, post planning.
Flow control method:	N/A
Volume of storage provided and method:	Storage will be provided in the form of a cellular soakaway (10.500m x 1.000m x 0.400m based on the assumed infiltration rate of 1×10^{-6}).
Offsite works:	None.

Flood Risk Summary

Flood Zone Classification: Flood Zone 1



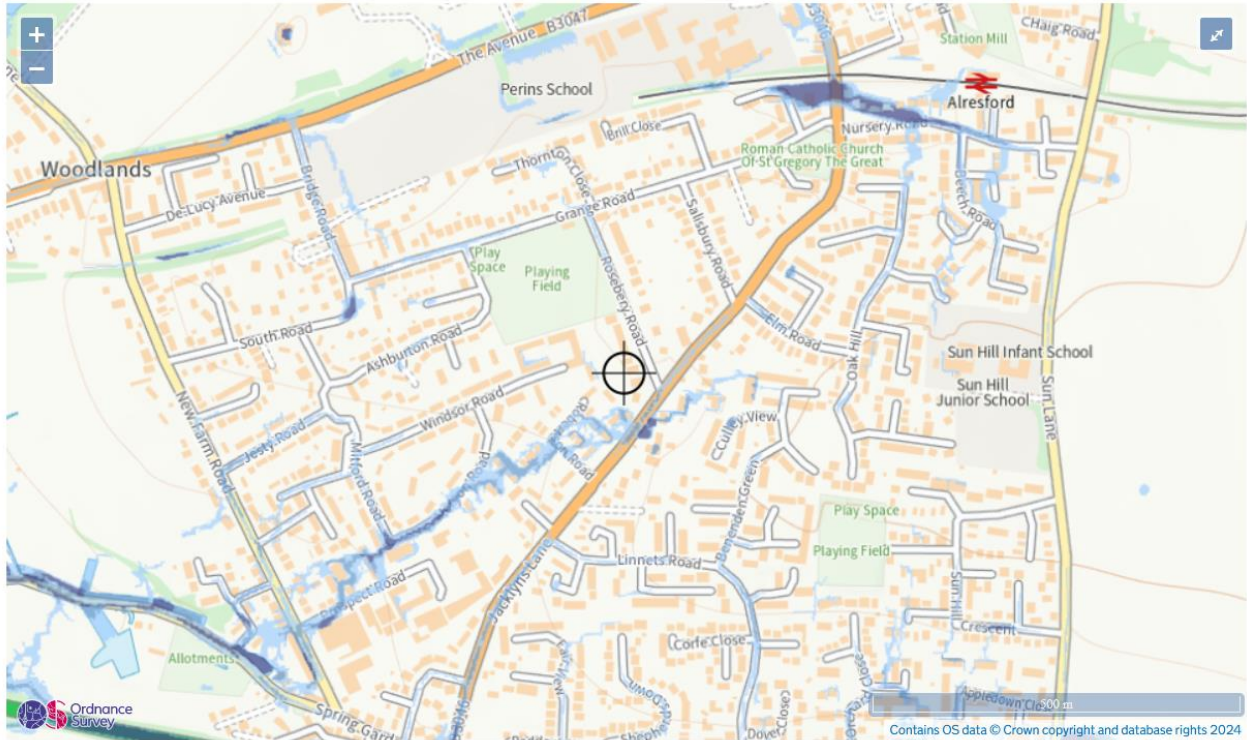
Rivers or the Sea: Upon review of the Long-Term Flood Risk Map for Rivers or the Sea, the site is not identified as being at risk of flooding from such sources.



Surface Water:

The site is identified as being at 'Very Low' risk of surface water flooding, according to the Long-Term Flood Risk Map for Surface Water.

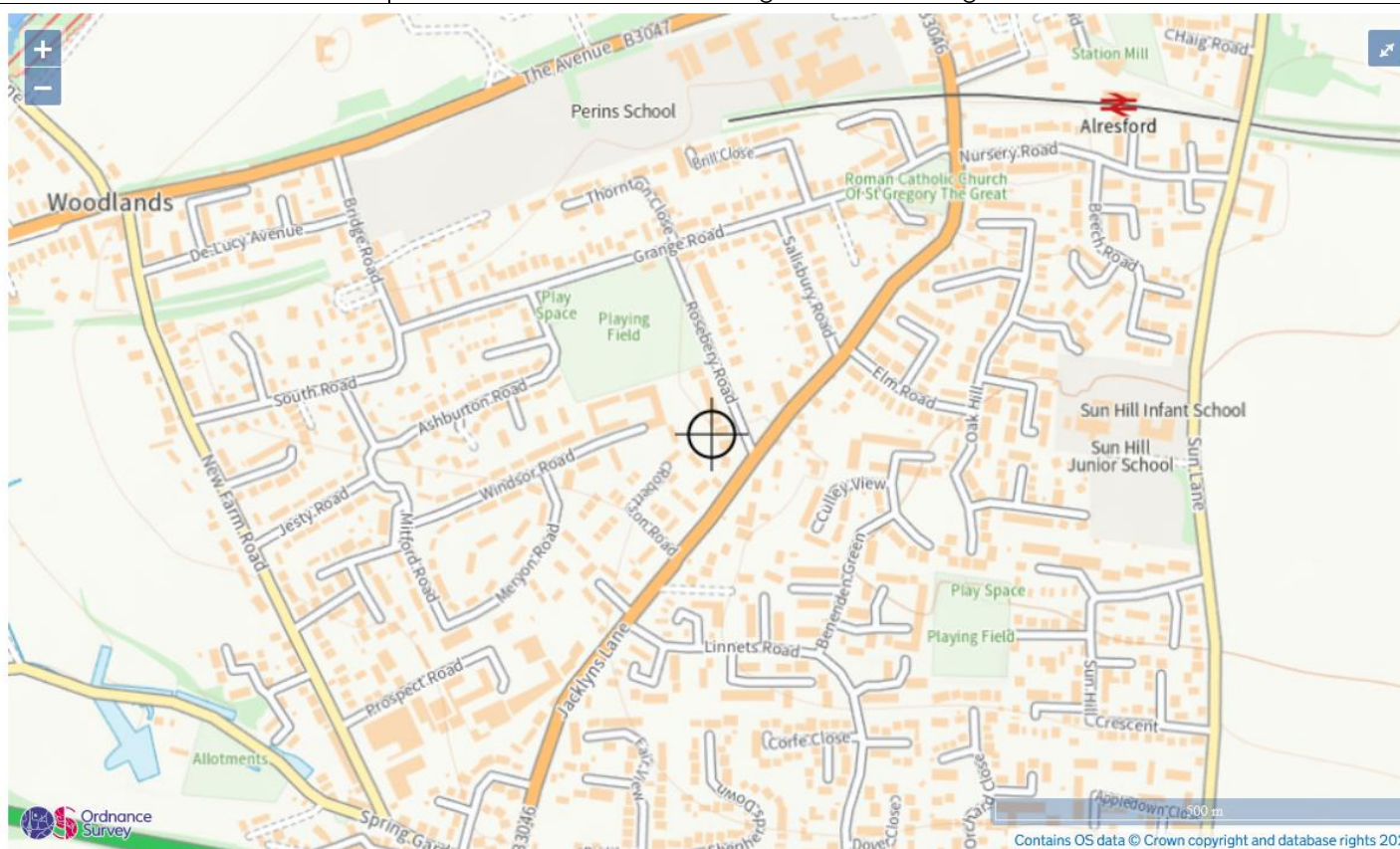
An overland flow route is identified to the south / south-west of the site with an associated risk of surface water flooding ranging from 'Low' to 'Medium'. The site itself is not identified as being at risk in relation to this flow route.



Extent of flooding from surface water

- High
- Medium
- Low
- Very low
- + Location you selected

Reservoirs:	<p>None.</p> <p>The site is not identified as being at risk of flooding from such sources.</p>
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Maximum extent of flooding from reservoirs:

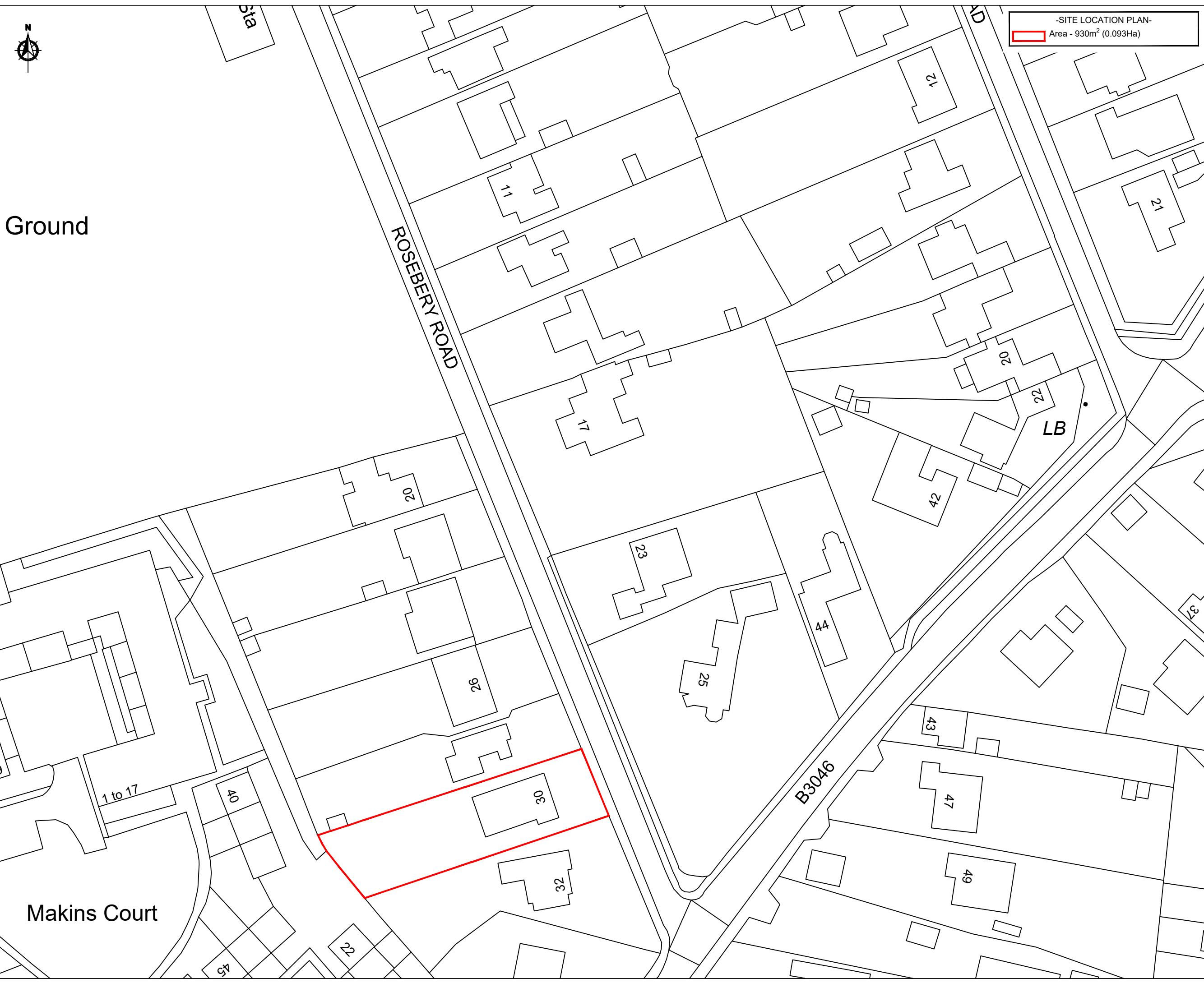
● when river levels are normal
 when there is also flooding from rivers
 ⊕ Location you selected

Groundwater:	<p>Groundwater investigation and/or monitoring has not been undertaken on site, therefore groundwater levels relative to the site itself have not been confirmed.</p> <p>At present, the risk of groundwater flooding is considered to be 'Very Low'.</p>
Historic Flooding	<p>No instances of historic flooding have been identified at the site, from the sources of information reviewed and assessed in the undertaking of this report.</p>

Foul Water Drainage Strategy

Number of properties:	1 No. existing dwelling
Method of discharge:	<p>Southern Water's public sewer records identify a 100mm dia. public foul water sewer located beneath Rosebery Road, which is shown to flow in a southerly direction, communicating with a 150mm dia. public foul water sewer beneath B3046, flowing in a westerly direction.</p> <p>The existing bungalow drains to this sewer.</p>
Point of discharge:	The site will continue to drain to Southern Water's public foul water sewer located beneath Rosebery Road.
Off-site works:	None. The existing connection to the public sewer network will be retained.

DRAWINGS



-SITE LOCATION PLAN-
 Area - 930m² (0.093Ha)

1. This drawing is to be read in conjunction with all other BP Civils drawings, and with all relevant Architect's and Engineer's drawings and specification. Any discrepancies found are to be reported immediately to the Engineer.
2. BP Civils accepts no responsibility for inaccuracies in data provided by third parties such as topographic surveys or Ordnance Survey mapping.
3. Do not scale, work to figured dimensions only. All dimensions are in millimeters unless noted otherwise and all levels are in metres from the topographic survey datum.
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5. All dimensions to be checked on site. All details and dimensions relating to sub-contractors work must be checked and agreed between the sub-contractor or supplier and the general contractor.
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	29/01/2024	Original Issue
Rev.	Date	Amendments



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 Tel: 0333 358 3260
 www.bpcivils.co.uk enquiries@bpcivils.co.uk

Drawing Status
PLANNING
NOT FOR CONSTRUCTION

Client
 Mr & Mrs J Phare

Project
 30 Rosebery Road,
 New Alresford, Winchester SO24 9HQ

Drawing Title
 Site Location Plan

Scale at A3	Date	Drawn By	Checked By	Project No.
1:250	Jan. '24	SPB	LDf	D2234

Drawing No.	Rev.
000-BPC-WD-ZZ-D-C-001	-

Ground

Makins Court

NOTES

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-TOPOGRAPHICAL SURVEY CONTOURS-
 90.000 CONTOUR (WITH LEVEL)



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Client
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Project
 30 Rosebery Road,
 New Alresford, Winchester SO24 9HQ

Drawing Title
 Topographical Survey Contours

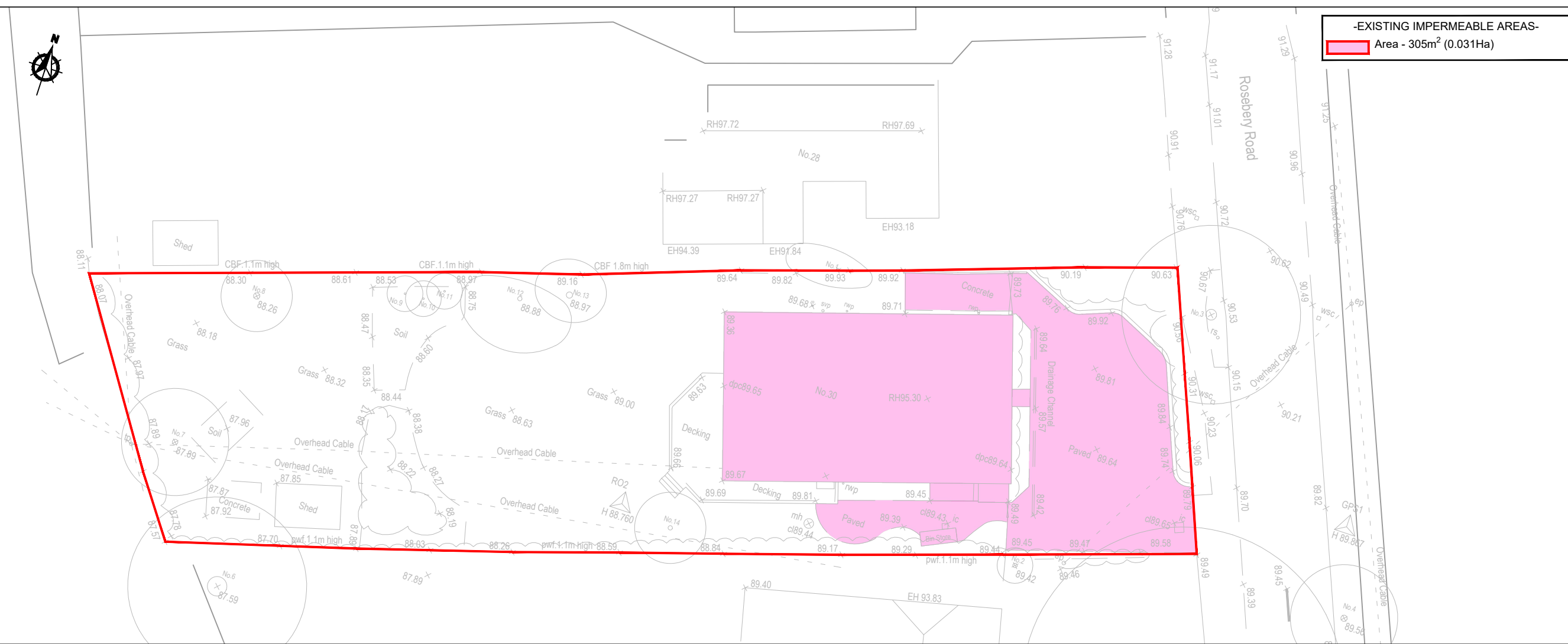
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1:250	Jan. '24	SPB	LDF	D2234

Drawing No.	Rev.
000-BPC-WD-ZZ-D-C-002	-

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-EXISTING IMPERMEABLE AREAS-
 Area - 305m² (0.031Ha)



-PROPOSED IMPERMEABLE AREAS-
 Area - 354m² (0.035Ha)



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Client
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Project
**30 Rosebery Road,
 New Alresford, Winchester SO24 9HQ**

Drawing Title
Impermeable Areas

Scale at A3	Date	Drawn By	Checked By	Project No.
1:250	Jan. '24	SPB	LDF	D2234

Drawing No.	Rev.
000-BPC-WD-ZZ-D-C-003	-

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-DRAINAGE STRATEGY-

- 90.000 CONTOUR (WITH LEVEL)
- EXISTING BUILDING CATCHMENT
- PROPOSED GARAGE CATCHMENT
- PROPOSED SURFACE WATER DRAIN
- PROPOSED SURFACE WATER INSPECTION CHAMBER
- PROPOSED SOAKAWAY

Soakaway designed subject to ground investigation and infiltration testing. Assumed rate of 1×10^{-6} utilised for preliminary design purposes.

10.50m x 1.00m x 0.400m (Base Level: 87.000)
Designed in Accordance with Building Regulations, Approved Document H (H3, 3.2.7).

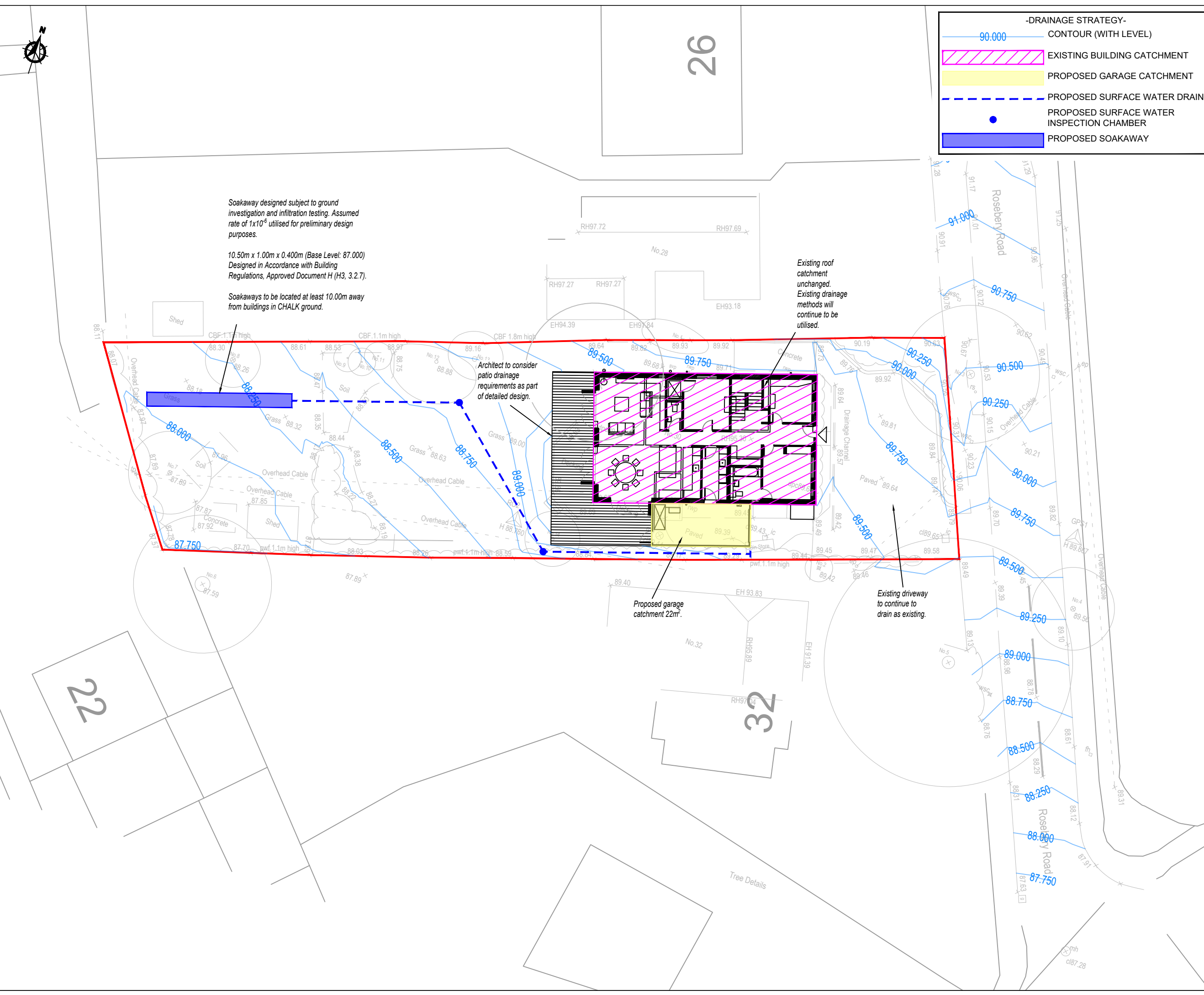
Soakaways to be located at least 10.00m away from buildings in CHALK ground.

Architect to consider patio drainage requirements as part of detailed design.

Existing roof catchment unchanged. Existing drainage methods will continue to be utilised.

Proposed garage catchment 22m².

Existing driveway to continue to drain as existing.



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Client
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Project
**30 Rosebery Road,
New Alresford, Winchester SO24 9HQ**

Drawing Title
**Proposed Surface Water
Drainage Strategy**

Scale at A3	Date	Drawn By	Checked By	Project No.
1:250	Jan. '24	SPB	LDF	D2234

Drawing No.	Rev.
000-BPC-WD-ZZ-D-C-004	-

APPENDICES

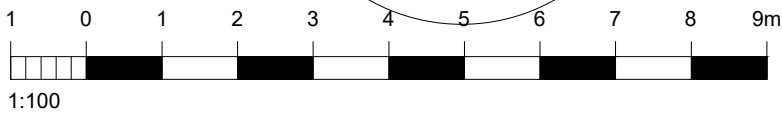
APPENDIX A – Development Proposals

30 Rosebery Road, New Alresford
site plan
1:200 at A3

drawing 002 rev P03 PLANNING
2024.01.07

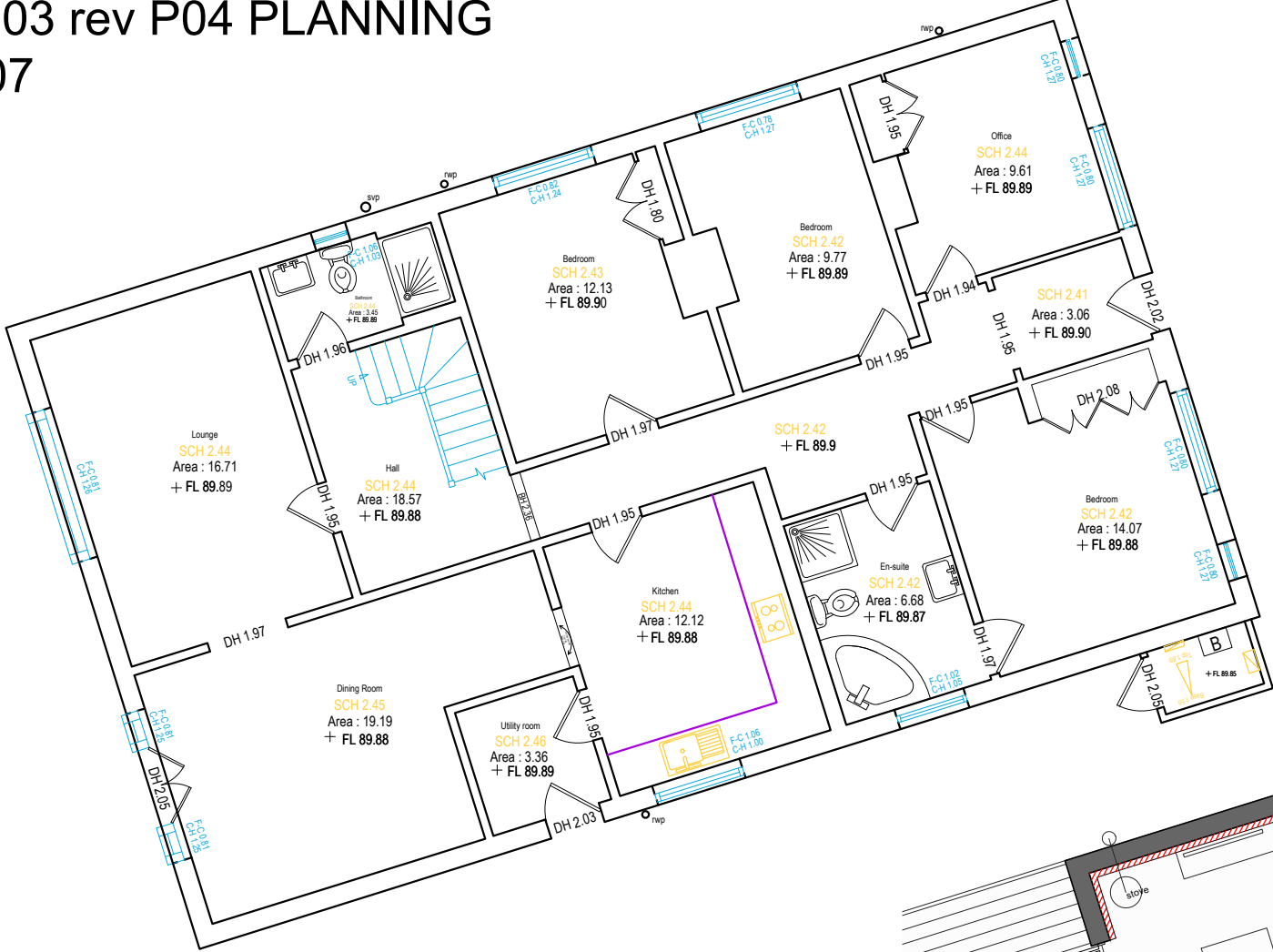


Plans 1:100 @ A3

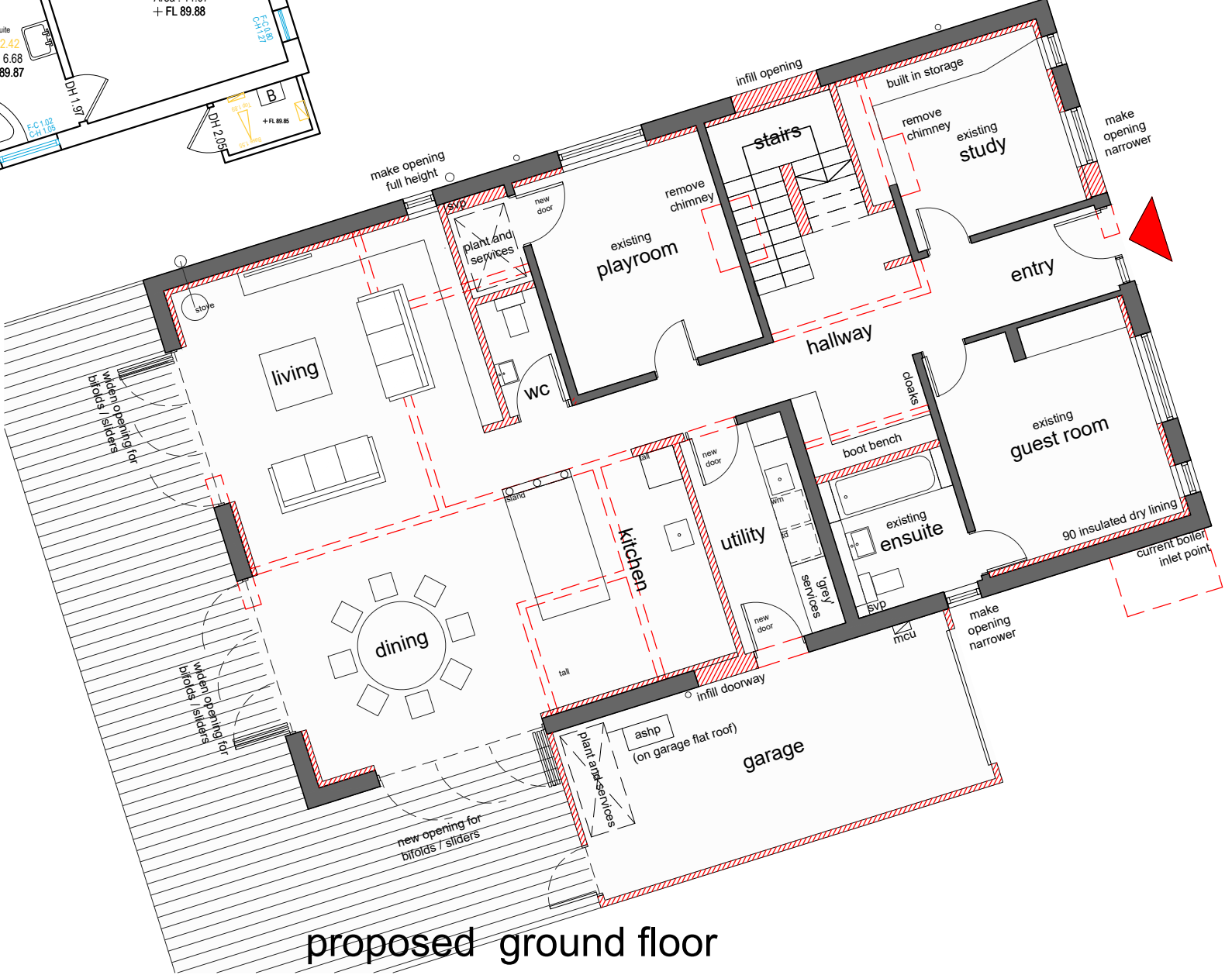


30 Rosebery Road, New Alresford
 floor plans - ground floor
 1:200 at A3

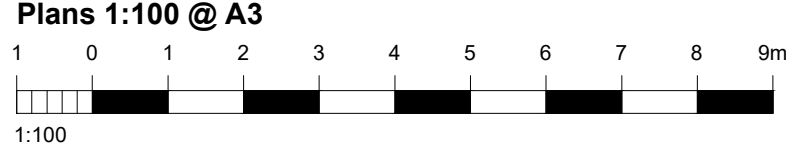
drawing 003 rev P04 PLANNING
 2024.01.07



existing ground floor

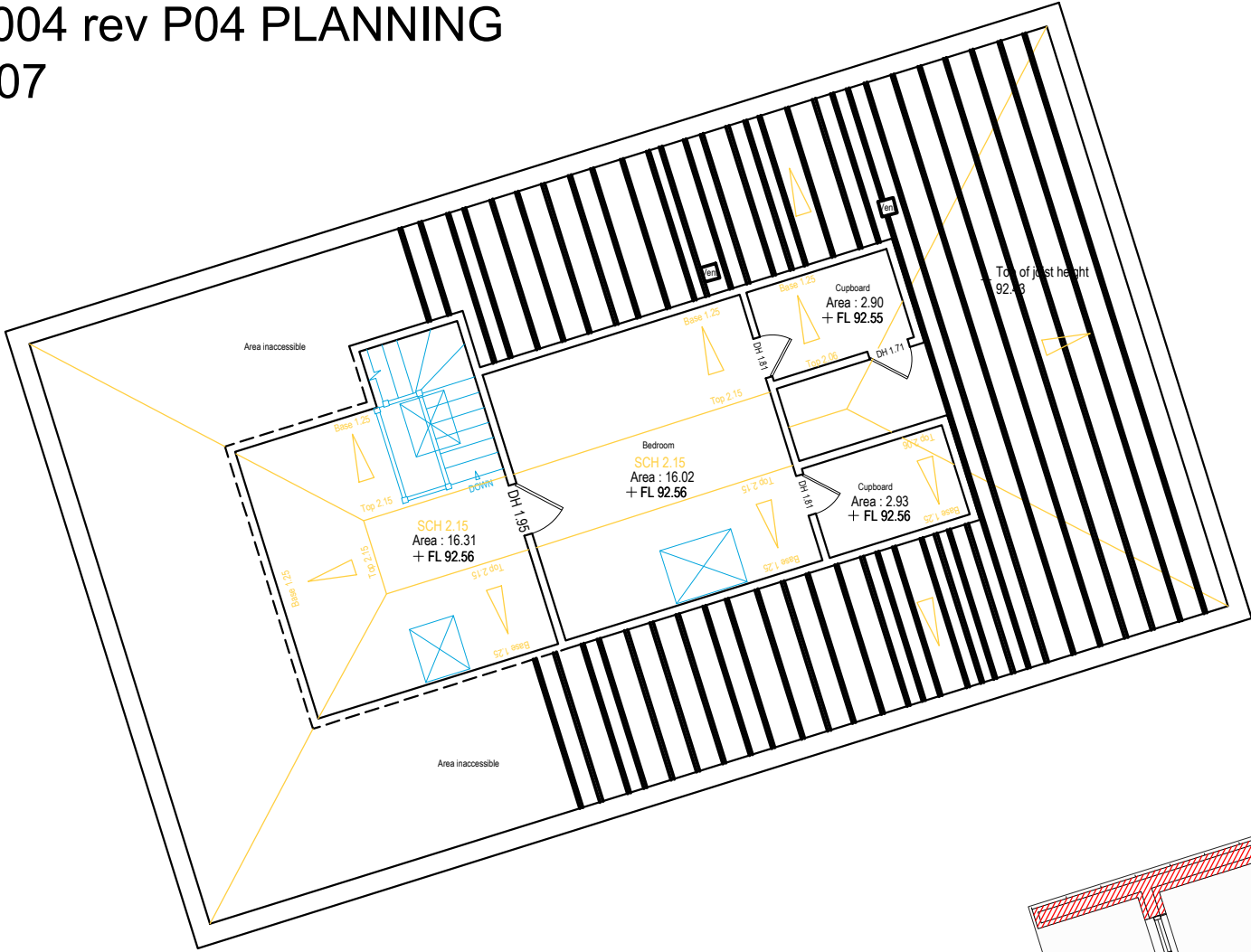


proposed ground floor



30 Rosebery Road, New Alresford
floor plans - first floor
1:200 at A3

drawing 004 rev P04 PLANNING
2024.01.07

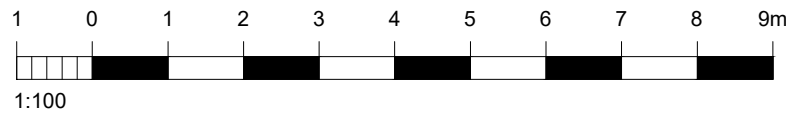


existing first floor



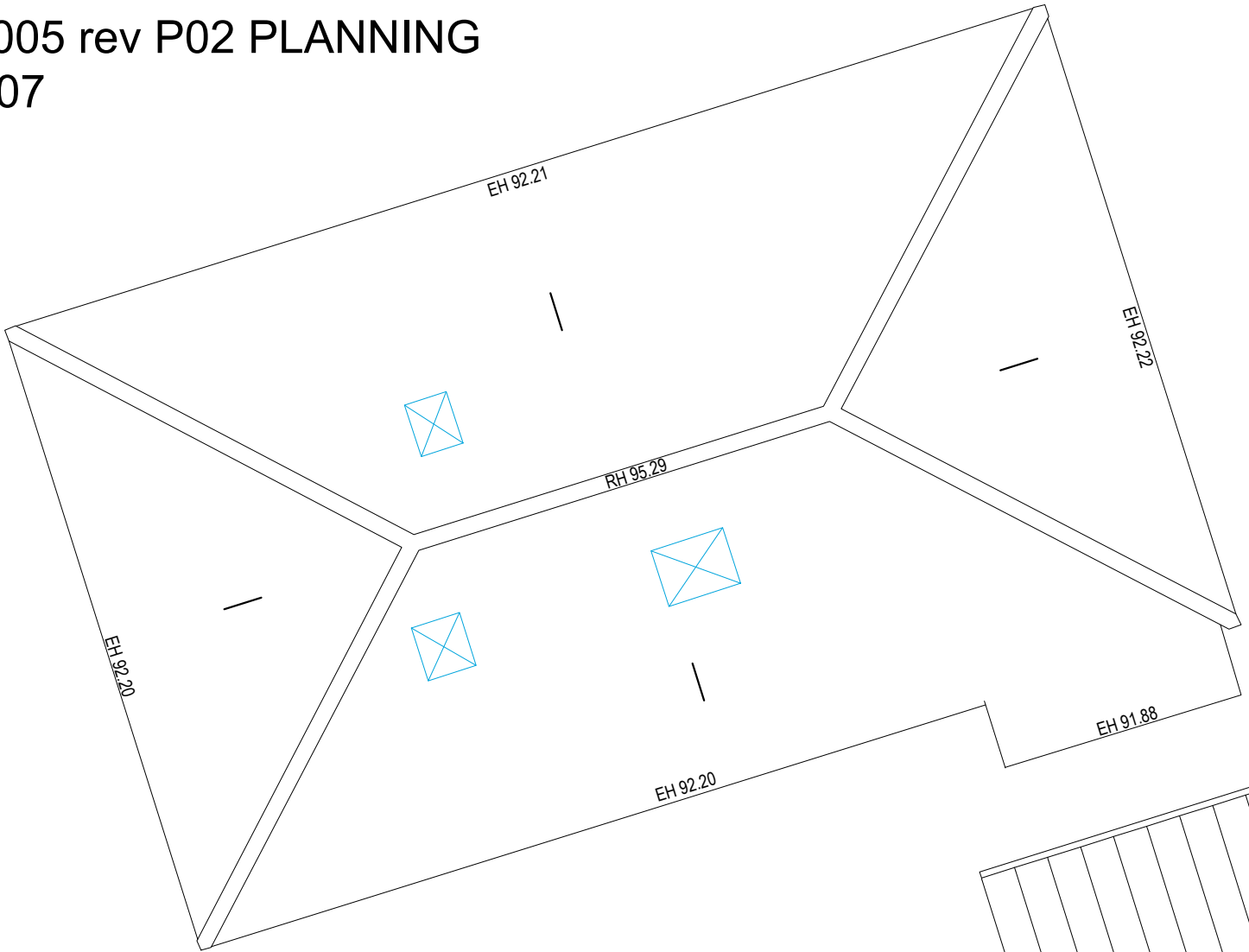
proposed first floor

Plans 1:100 @ A3

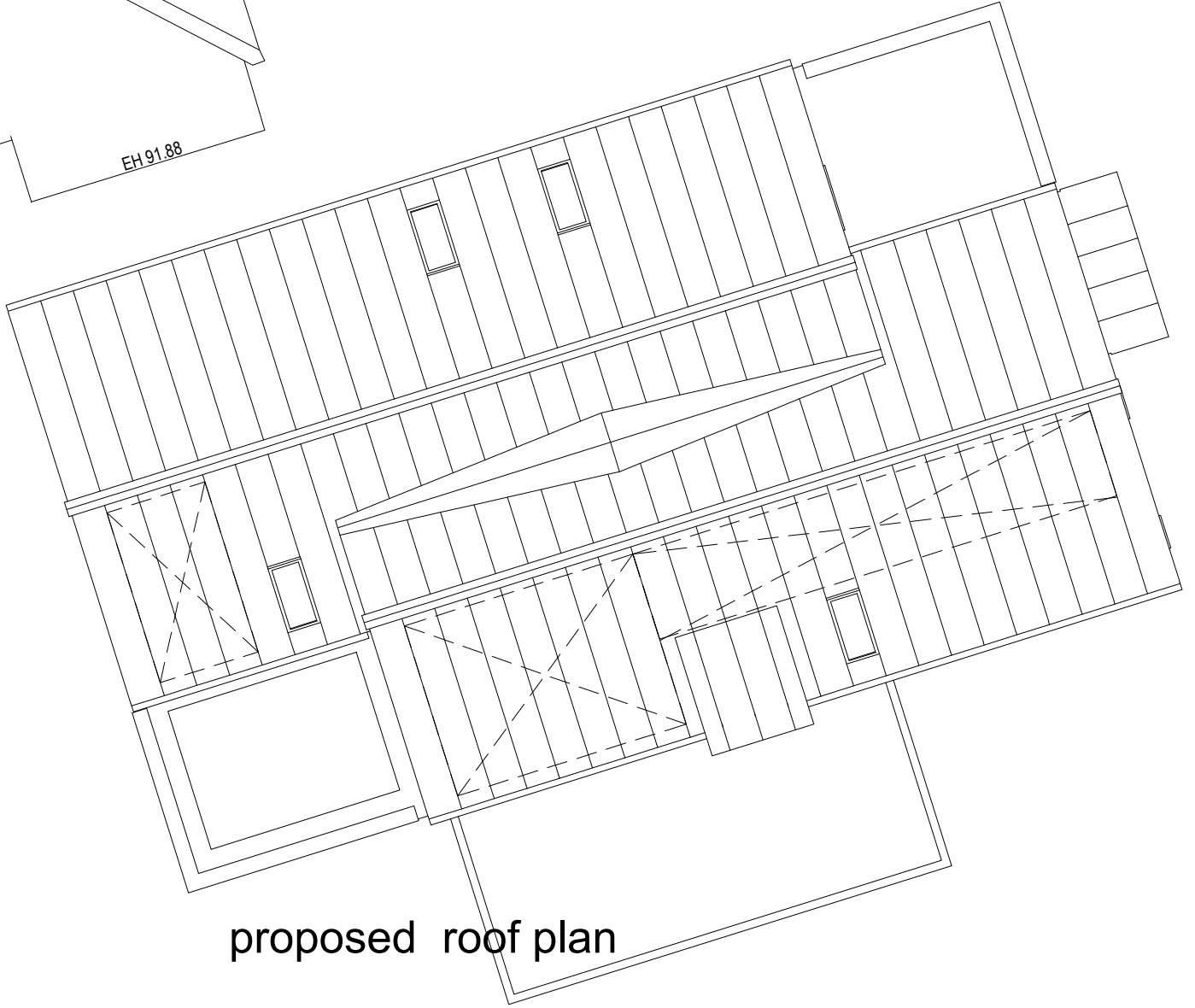


30 Rosebery Road, New Alresford
floor plans - roof plan
1:200 at A3

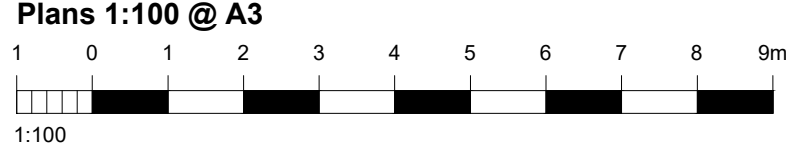
drawing 005 rev P02 PLANNING
2024.01.07



existing roof plan



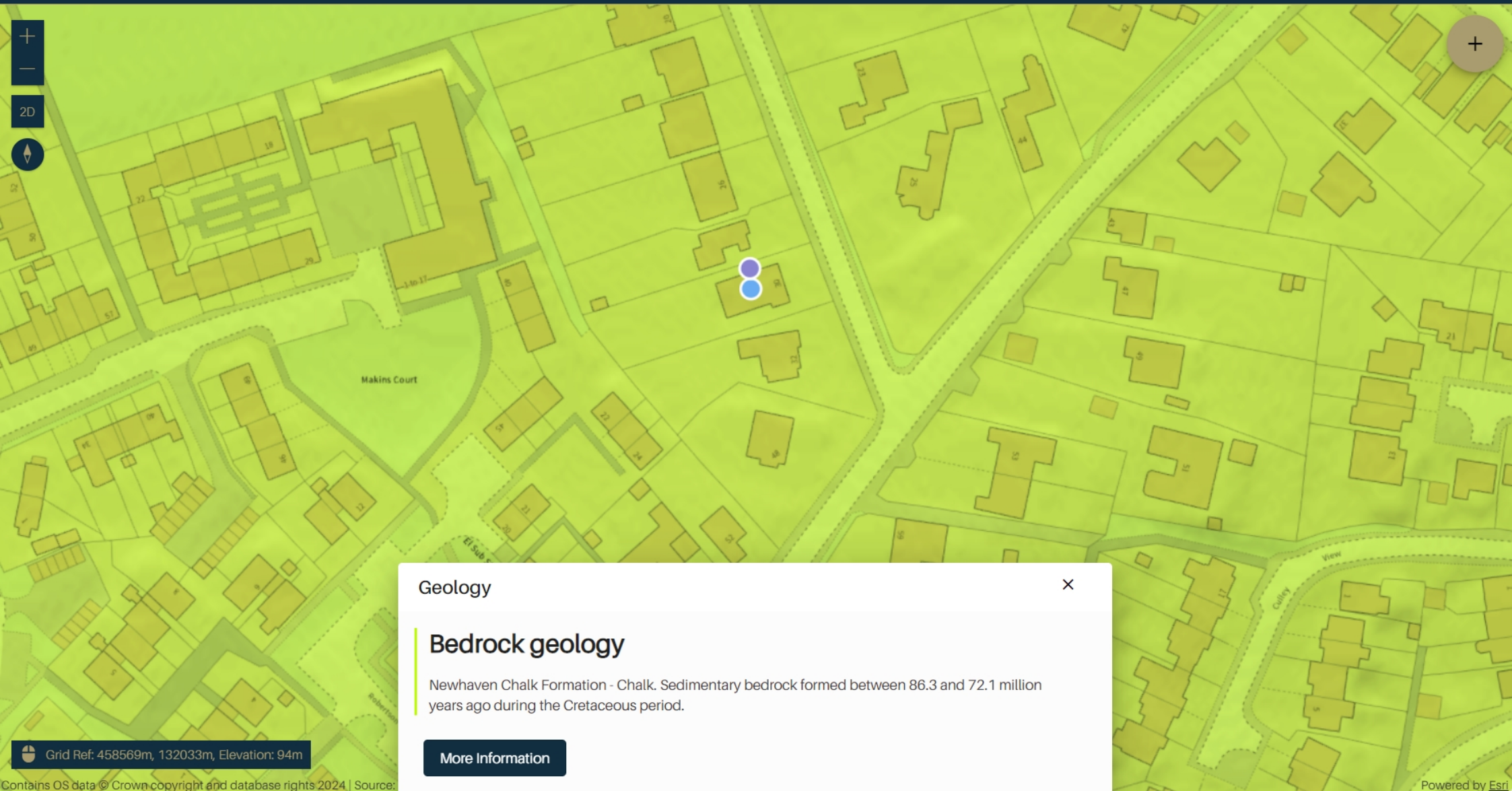
proposed roof plan



APPENDIX B – BGS Maps



2D



Geology

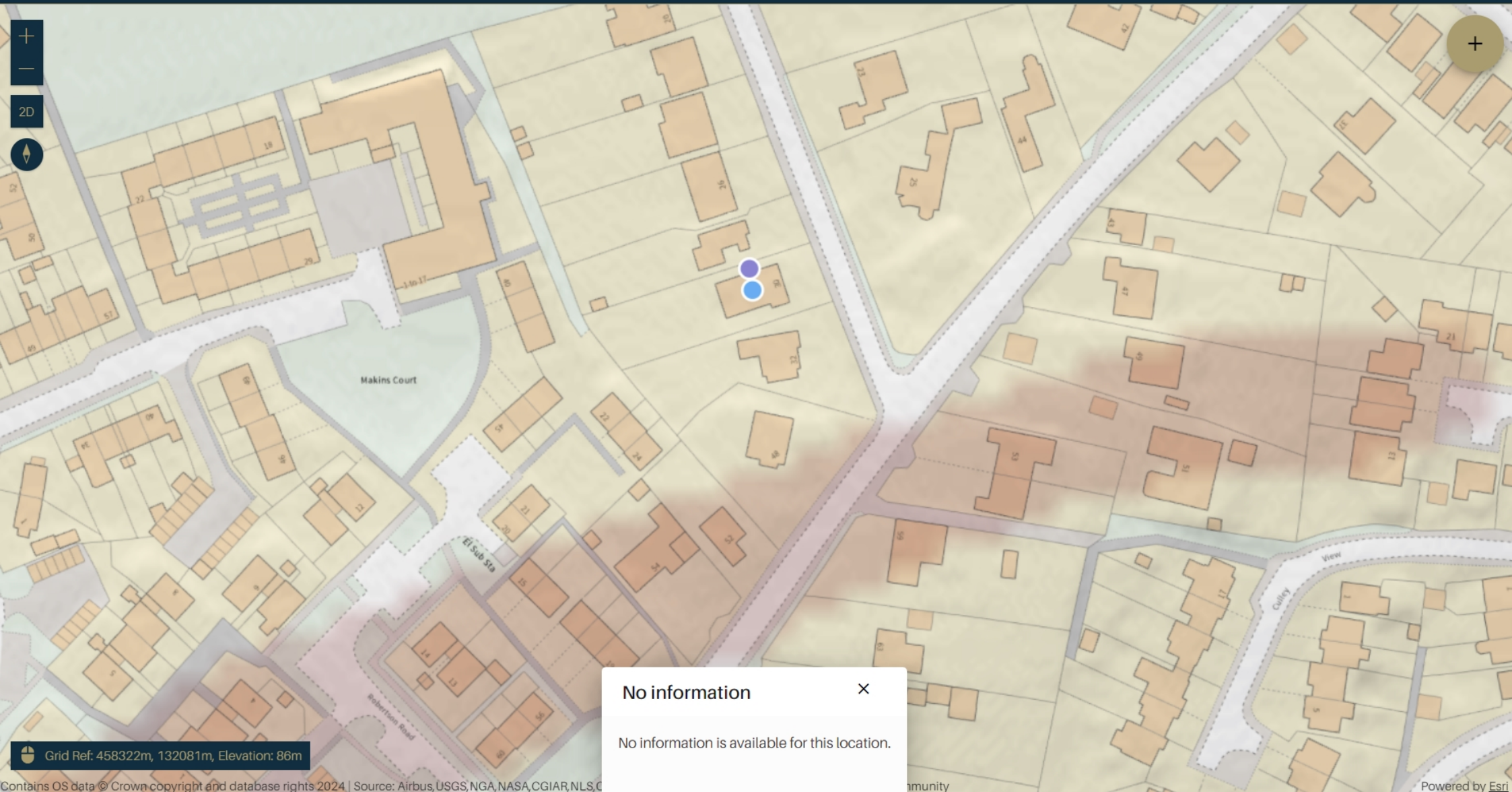


Bedrock geology

Newhaven Chalk Formation - Chalk. Sedimentary bedrock formed between 86.3 and 72.1 million years ago during the Cretaceous period.

[More Information](#)

Grid Ref: 458569m, 132033m, Elevation: 94m



2D



Makins Court

El Sub Sta

Robertson Road

Cully View

No information ×

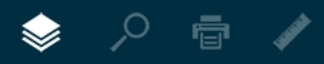
No information is available for this location.

Grid Ref: 458322m, 132081m, Elevation: 86m

APPENDIX C – BGS Borehole Records

GeoIndex Onshore

458398, 132051



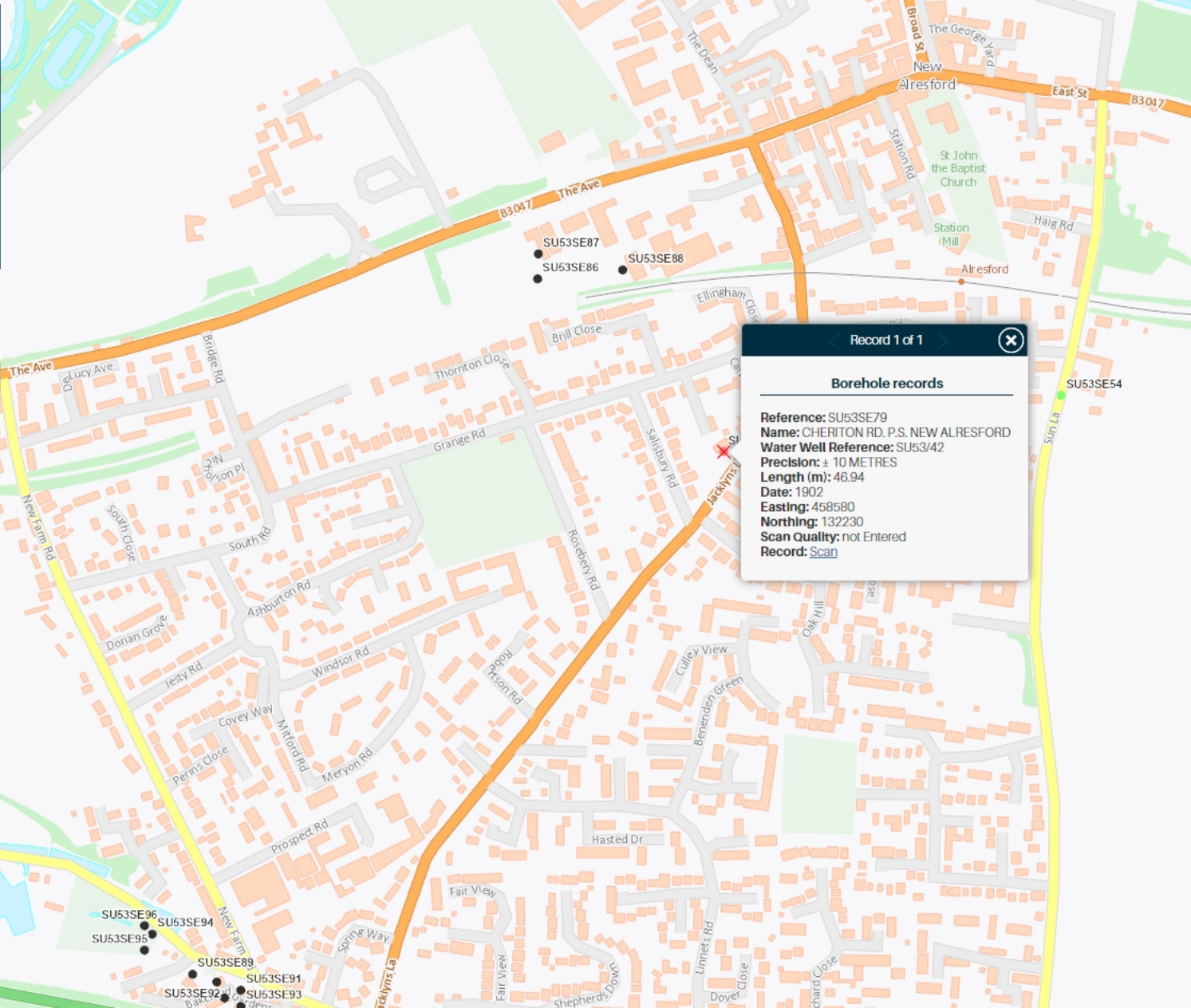
Data

Borehole records

[Add Data](#) [Show Legend](#)



Location: 459599, 132195



Record 1 of 1

Borehole records

Reference: SU53SE79
Name: CHERITON RD. P.S. NEW ALRESFORD
Water Well Reference: SU53/42
Precision: ± 10 METRES
Length (m): 46.94
Date: 1902
Easting: 458580
Northing: 132230
Scan Quality: not Entered
Record: [Scan](#)



British Geological Survey

GeoIndex Onshore

458398, 132051







Data

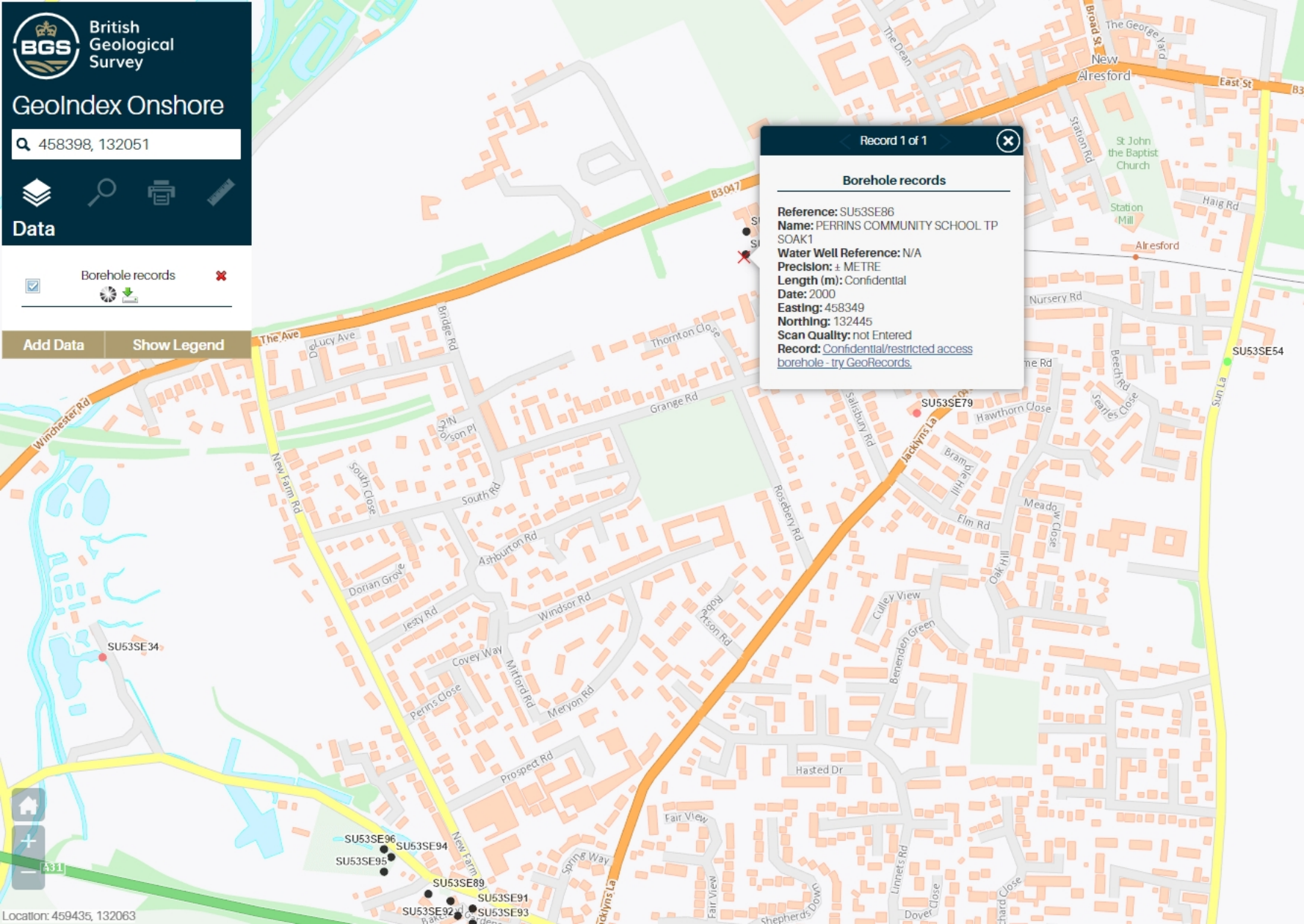
Borehole records 



Record 1 of 1 

Borehole records

Reference: SU53SE86
Name: PERRINS COMMUNITY SCHOOL TP SOAK1
Water Well Reference: N/A
Precision: ± METRE
Length (m): Confidential
Date: 2000
Easting: 458349
Northing: 132445
Scan Quality: not Entered
Record: [Confidential/restricted access borehole - try GeoRecords.](#)





NGRC
BOREHOLE RECORDS
ADJUSTMENT FORM

QUARTER SHEET SU53SE

BH REGISTRATION NUMBER 71-81

RECORDS ENTERED AND HELD BY WALLINGFORD

BH REGISTRATION NUMBER(S)



300/110 Winchester R.D.C., Pumping Station, Cheriton Road, New Alresford.

(Filled in)

SU53/42

Surface +314. Shaft 110; rest bore 18 in. Water struck at +307.

R.W.L. +307. c. 1902.

Suction +204. Recovered to R.W.L. in 5 min. Yield 3,500 g.p.h.; 8 h.p.d.

Hardness: P. 45, T. 195. 1935. Yield 3,500 g.p.h., 10 h.p.d. Apr. 1941.

R.W.L. +206. P.W.L. +197. Yield 10,000 g.p.h. Oct. 1950. R.W.L. +203.

P.W.L. +197. Yield 3,333 g.p.h. Mar. 1953.

UGk

...

...

154

154

*Estimated
Clarvia*

No details known.

pp. C.P. Thomas

9-1-69.



Inland Water Survey for Great Britain.

300/110
SU 53 42
W.P. 87

Name or Description of Authority or Undertaking..... ALRESFORD WATER COMPANY, LTD.

Postal Address..... East Street, Alresford, Hants

Alresford RDC from 1953
now disused.

WB correspondence
REA

Para: New Alresford
Hants U2NW/W

(A) OVER-GROUND WATER. SU 5858 3223

(I) (a) Do you take systematic records of levels of water in :-

- (1) rivers
- (2) streams
- (3) reservoirs
- (4) lakes
- (5) canals or navigable rivers

(b) If so, please give a short description of the method used.

(c) How often are the readings taken?

(d) Exact points at which the records are taken. (A map or sketch would be helpful.)

(e) Have the levels been related to Ordnance Datum Level or to some other standard (in the latter case please specify standard)?

(f) Are all the levels (e.g., highest and lowest) covered satisfactorily by the records taken?

(g) Are arrangements made for extra readings during rise and fall of floods, etc.?

(II) What types of systematic records of discharge other than records of levels are kept as regards :-

- (1) rivers
- (2) streams
- (3) reservoirs
- (4) lakes
- (5) canals or navigable waterways

Uck - 154 feet
42
L.P.T.
9/1/62
Hants/N.W. (W).
Site by Mr. R. V. Harrold of
The Alresford Water Co.
Details as enclosed, except
that pumping is now for
about 10 hours daily
ASB. 7. 4. 41

WB letter 31 Mar. 1954.
Winchester R.D.C. Taken over
Oct. 1952
Now Disused.

SW 14/7/54
No trace of ... presumably
pumping station demolished
& well presumably filled in.
Visited R.C.D.
14. 7. 62

WQ/300/VAM
Winchester R.D.C. say well filled in.
10.12.65.



(III) (a) Have measurements been made from which the data for levels can be converted to records of discharge of:—

- (1) rivers and streams
- (2) reservoirs
- (3) lakes
- (4) canals or navigable waterways

(b) If so, how have these measurements been made (e.g., by current meters, velocities of floats, surveys of sections, calibration of weirs, records of water used for locking, etc.)?

(IV) (a) Are records kept in the case of springs breaking overground of the amount of water yielded?

(b) If so, what form of recording is used?

(c) How often are readings taken?

(d) Exact location of the spring (A map or sketch would be helpful.)

(V) Since when have the records under I, II, III and IV been kept?

(VI) Are past records available?

(VII) REMARKS.

(Please indicate here any further information or particulars which may be thought likely to assist in the survey.)



300 Su53/42
110

(B) UNDERGROUND WATER—(WELLS AND BORINGS).

(In each case please state whether a well and/or boring is in question.)

I. GENERAL.

1. Exact site of well or boring Cheriton Road and
(A map or sketch showing position would be useful.) Grange Road,
... .. New Alresford,
... .. Hants
2. Surface level of ground above Ordnance Datum 314.15 ft.
3. Date of construction About 1902

WELLS.

4. Depth of well from surface level of ground (i.e., 2 above). If top of well is below the surface level of the ground (i.e., 2 above) state how much 110 ft.
5. Depth of floor of galleries at site of well; also dimension and direction of galleries No galleries
Only boring ft.

BORINGS.

6. Depth of boring from surface level of ground (i.e., 2 above). If boring is in bottom of well, state depth of well Well 110 ft.
... .. Boring 44 "
... .. 154 feet
... .. 18 in.
7. (a) Diameter of top of boring 18 in.
(b) Diameter of bottom of boring... .. 18 in.
8. Tubed from top of boring to No tubing ft.
... .. Solid chalk
9. Lining tubes perforated at depths of ----- ft.
10. Water struck during boring at depths of Before boring 7 ft.
11. What was rest level on completion of boring? 7 ft.

WELLS AND BORINGS.

12. Is the water raised by pump or air lift? Pump
13. Depth from top of well or boring to bottom of suction pipe 110 ft.

3

II. If systematic measurements of water levels are made, state whether these include:—

- (a) Pumping levels Yes (b) Rest levels ... Yes
- (c) Time of recovery to rest level on cessation of pumping Five minutes
- (d) Changes in pumping level, if rate of pumping is altered. No changes
- Also state: (e) at what intervals records are taken (i.e., daily, weekly, etc.) Weekly

Please furnish a specimen graph of records taken over as long a period as available (up to 1 year).

Very little variation.
Averaging from 6 to 10 feet according to season.

III. If measurements are made only occasionally, please indicate what is, or has been, done in this respect and furnish examples of any graphs or figures available.

IV. YIELDS.

- (1) Number of gallons pumped per hour 3500
- (2) Is pumping continuous? No.
- (3) If not, how many hours pumping per day? 8 hours *10 hours per day 7.4.41*
- (4) Maximum daily yields available As many hours pumping as required
- Estimated
- Based on actual tests

V. If a section or record of strata can be given please attach to this form.

VI. (1) If a chemical analysis can be given please attach.

(2) If not state hardness 19.5 temp. 4.5 perm.

(3) For what purpose is the water used? Domestic generally

Trade slightly.

Data Bank

APPENDIX D – MAGIC Maps

Table of Contents

- Habitats and Species
- Land Based Schemes
- Landscape
- Geology and Soils
 - Aquifer Designation Map (Bedrock) (England) ⓘ
 - Principal
 - Secondary A
 - Secondary B
 - Secondary (undifferentiated)
 - Unproductive
 - Aquifer Designation Map (Superficial Drift) (England) ⓘ
 - Groundwater Vulnerability Map (England) ⓘ
 - Geological Places to Visit (England) ⓘ
 - Geological Descriptions (England) ⓘ
 - Soilscape (England) ⓘ
- Landscape Classifications
- Marine
- Aerial Photography
- Background Mapping
- Base Map
- OS Black and White Mapping
- OS Colour Mapping

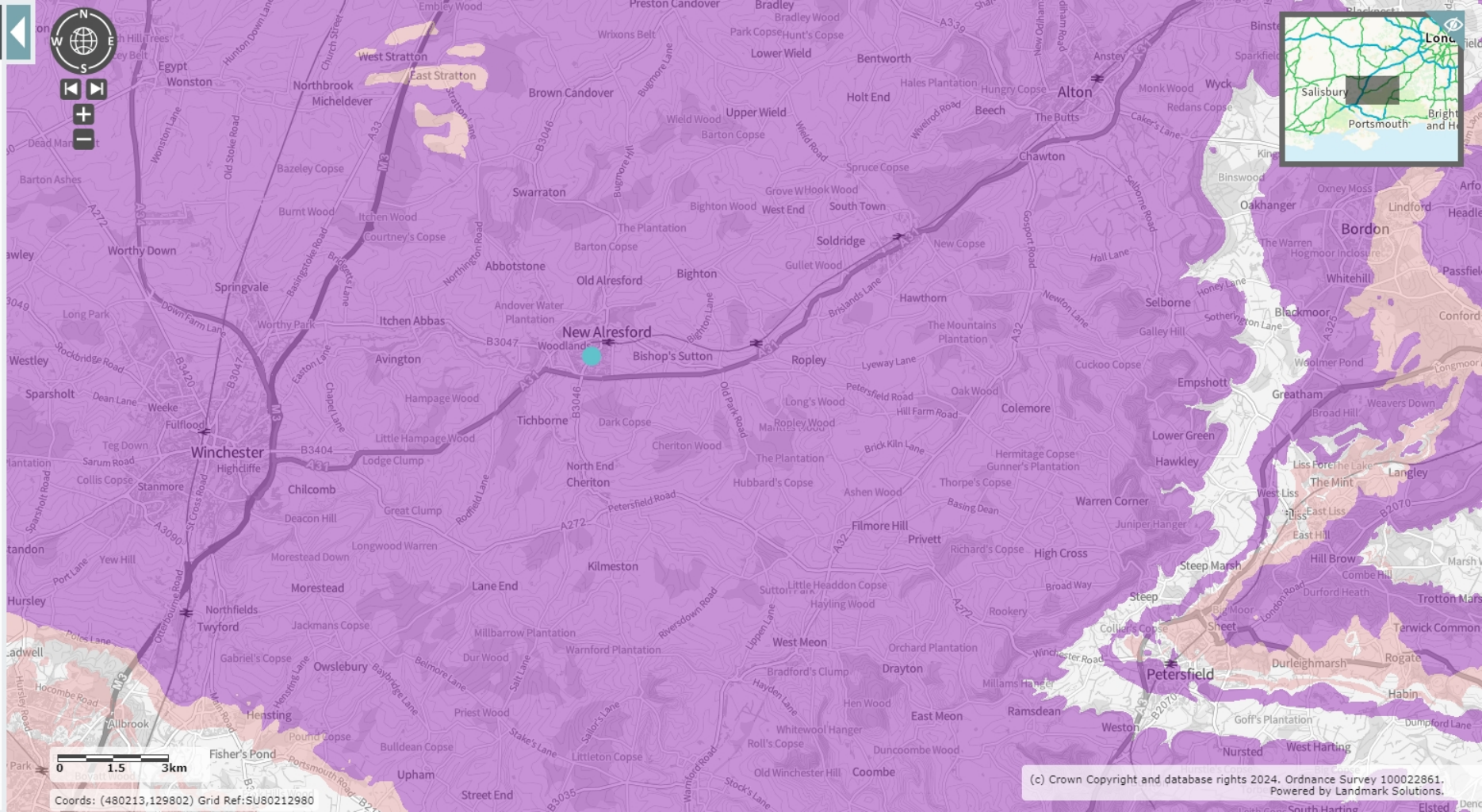


Table of Contents

Countrywide Stewardship Targeting & Scoring Layers

- Designations
- Habitats and Species
- Land Based Schemes
- Landscape

Geology and Soils

- Aquifer Designation Map (Bedrock) (England)
- Aquifer Designation Map (Superficial Drift) (England)
- Principal
- Secondary A
- Secondary B
- Secondary (undifferentiated)
- Unknown (lakes+landslip)
- Unproductive
- Groundwater Vulnerability Map (England)
- Geological Places to Visit (England)
- Geological Descriptions (England)
- Soilscape (England)
- Landscape Classifications

Marine

- Aerial Photography
- Background Mapping

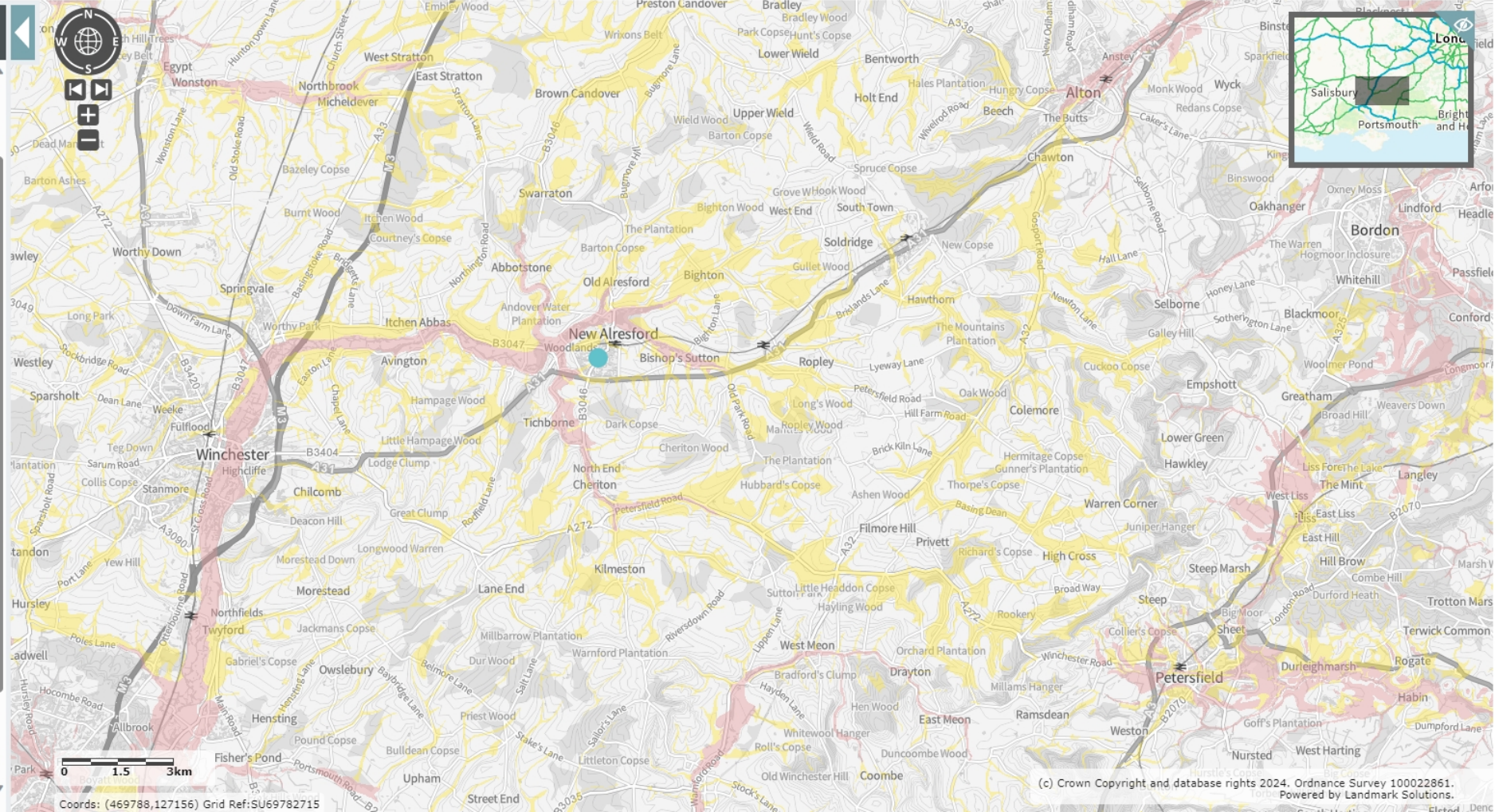
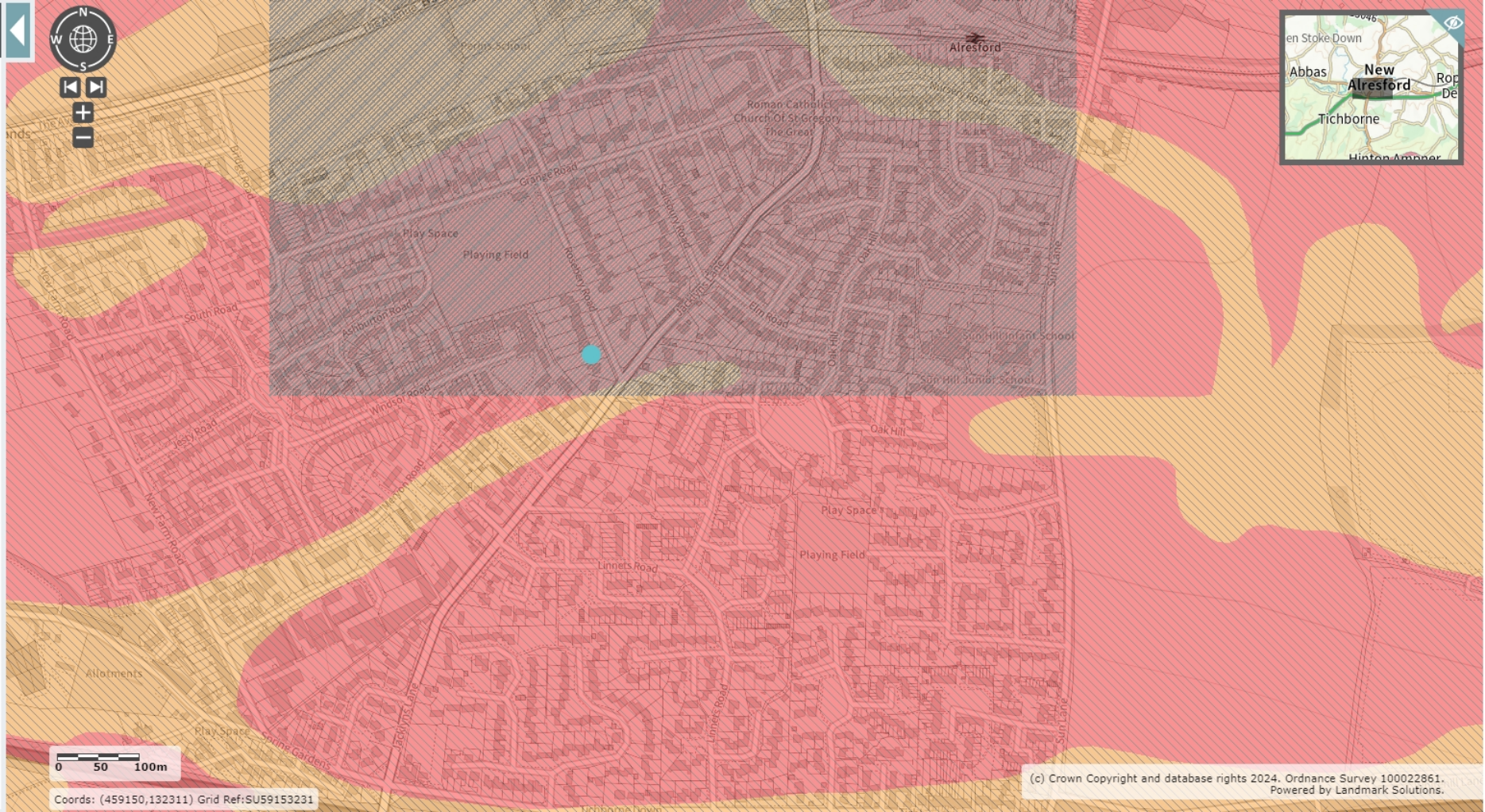


Table of Contents

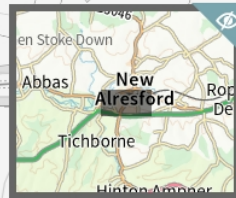
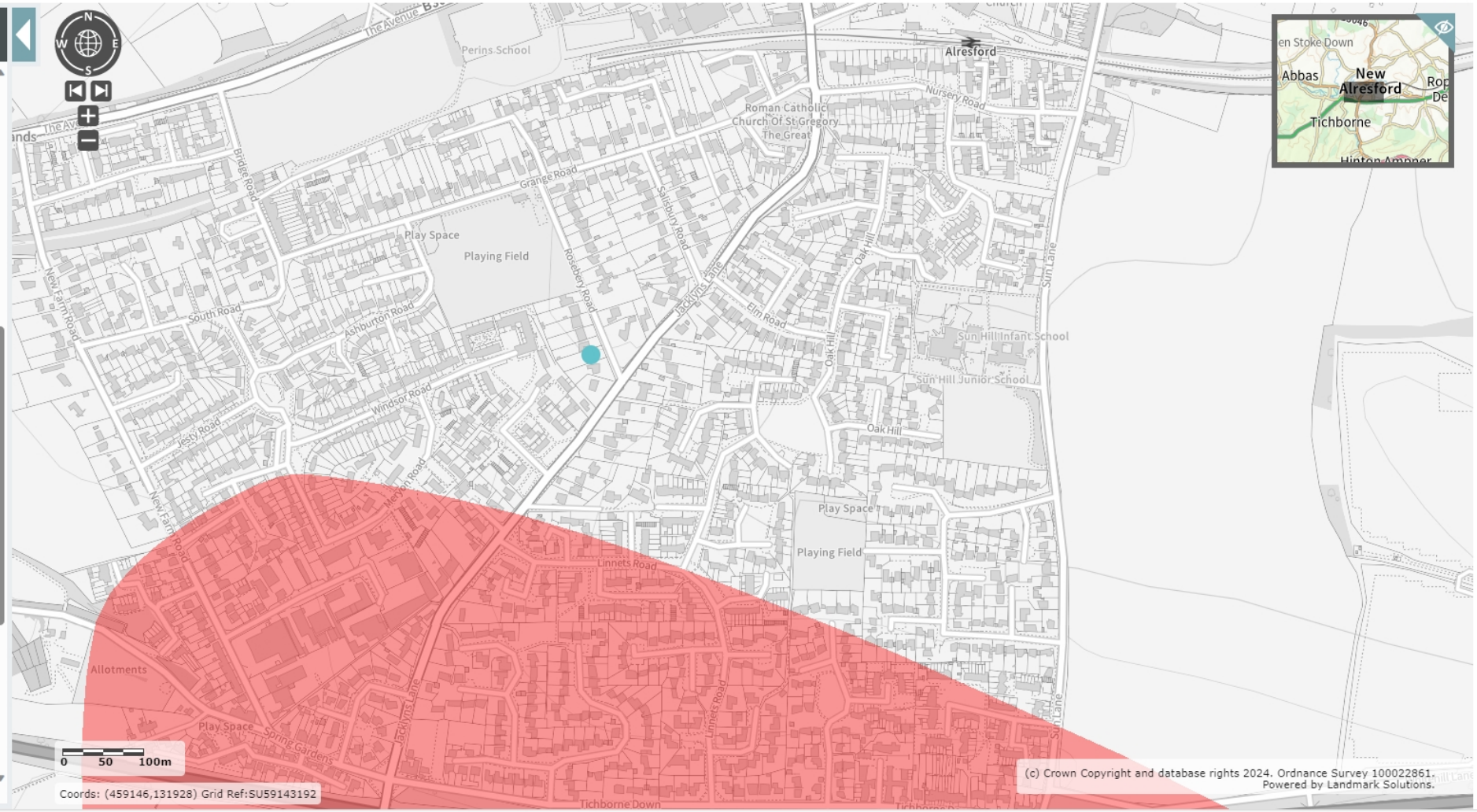
- Landscape
 - Geology and Soils
 - Aquifer Designation Map (Bedrock) (England) ⓘ
 - Aquifer Designation Map (Superficial Drift) (England) ⓘ
 - Groundwater Vulnerability Map (England) ⓘ
 - Local Information
 - Soluble Rock Risk
 - High
 - Medium - High
 - Medium
 - Medium - Low
 - Low
 - Unproductive
 - Geological Places to Visit (England) ⓘ
 - Geological Descriptions (England) ⓘ
 - Soilscape (England) ⓘ
 - Landscape Classifications
- Marine
- Aerial Photography
- Background Mapping
- Base Map
- OS Black and White Mapping
- OS Colour Mapping



Coords: (459150,132311) Grid Ref:SU59153231

Table of Contents

- Drinking Water Protected Areas (Surface Water) (England) ⓘ
- Drinking Water Safeguard Zones (Surface Water) (England) ⓘ
- Drinking Water Safeguard Zones (Groundwater) (England) ⓘ
- Source Protection Zones merged (England) ⓘ
- Zone I - Inner Protection Zone
- Zone I - Subsurface Activity
- Zone II - Outer Protection Zone
- Zone II - Subsurface Activity
- Zone III - Total Catchment
- Zone III - Subsurface Activity
- Zone of Special Interest
- Marine Designations
- Habitats and Species
- Land Based Schemes
- Agri-Environment Schemes
- Forestry and Woodland Schemes
- Other Schemes
- Landscape
- Geology and Soils
 - Aquifer Designation Map (Bedrock) (England) ⓘ
 - Aquifer Designation Map (Superficial Drift) (England) ⓘ
 - Groundwater Vulnerability Map (England) ⓘ
 - Geological Places to Visit (England) ⓘ



APPENDIX E – Public Sewer Records



(c) Crown copyright and database rights 2024 Ordnance Survey 100031673

Date: 29/01/24

Scale: 1:1250

Map Centre: 458398,132051

Data updated: 21/11/23

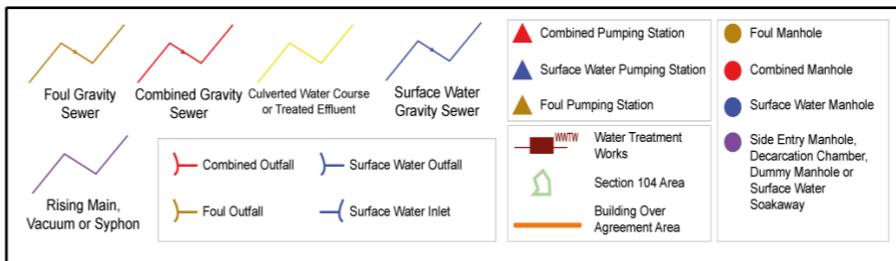
Our Ref: 1380432 - 1

Wastewater Plan A3

The positions of pipes shown on this plan are believed to be correct, but Southern Water Services Ltd accept no responsibility in the event of inaccuracy. The actual positions should be determined on site. This plan is produced by Southern Water Services Ltd (c) Crown copyright and database rights 2024 Ordnance Survey 100031673. This map is to be used for the purposes of viewing the location of Southern Water plant only. Any other uses of the map data or further copies is not permitted.

WARNING: BAC pipes are constructed of Bonded Asbestos Cement.

WARNING: Unknown (UNK) materials may include Bonded Asbestos Cement.



searches@bpcivils.co.uk

30 Rosebery Road



APPENDIX F – Wallingford Greenfield Run-off Calculations

Calculated by:

Site name:

Site location:

Site Details

Latitude:

Longitude:

Reference:

Date:

This is an estimation of the greenfield runoff rates that are used to meet normal best practice criteria in line with Environment Agency guidance "Rainfall runoff management for developments", SC030219 (2013), the SuDS Manual C753 (Ciria, 2015) and the non-statutory standards for SuDS (Defra, 2015). This information on greenfield runoff rates may be the basis for setting consents for the drainage of surface water runoff from sites.

Runoff estimation approach

Site characteristics

Total site area (ha):

Methodology

Q_{BAR} estimation method:

SPR estimation method:

Notes

(1) Is $Q_{BAR} < 2.0$ l/s/ha?

When Q_{BAR} is < 2.0 l/s/ha then limiting discharge rates are set at 2.0 l/s/ha.

Soil characteristics

	Default	Edited
SOIL type:	1	1
HOST class:	N/A	N/A
SPR/SPRHOST:	0.1	0.1

(2) Are flow rates < 5.0 l/s?

Where flow rates are less than 5.0 l/s consent for discharge is usually set at 5.0 l/s if blockage from vegetation and other materials is possible. Lower consent flow rates may be set where the blockage risk is addressed by using appropriate drainage elements.

Hydrological characteristics

	Default	Edited
SAAR (mm):	825	825
Hydrological region:	7	7
Growth curve factor 1 year:	0.85	0.85
Growth curve factor 30 years:	2.3	2.3
Growth curve factor 100 years:	3.19	3.19
Growth curve factor 200 years:	3.74	3.74

(3) Is $SPR/SPRHOST \leq 0.3$?

Where groundwater levels are low enough the use of soakaways to avoid discharge offsite would normally be preferred for disposal of surface water runoff.

Greenfield runoff rates

Default Edited

Q _{BAR} (l/s):	0.02	0.02
1 in 1 year (l/s):	0.02	0.02
1 in 30 years (l/s):	0.05	0.05
1 in 100 year (l/s):	0.06	0.06
1 in 200 years (l/s):	0.08	0.08

This report was produced using the greenfield runoff tool developed by HR Wallingford and available at www.uksuds.com. The use of this tool is subject to the UK SuDS terms and conditions and licence agreement, which can both be found at www.uksuds.com/terms-and-conditions.htm. The outputs from this tool are estimates of greenfield runoff rates. The use of these results is the responsibility of the users of this tool. No liability will be accepted by HR Wallingford, the Environment Agency, CEH, Hydrosolutions or any other organisation for the use of this data in the design or operational characteristics of any drainage scheme.

APPENDIX F – Causeway Flow Hydraulic Calculations



Design Settings

Rainfall Methodology	FEH-22	Minimum Velocity (m/s)	1.00
Return Period (years)	2	Connection Type	Level Soffits
Additional Flow (%)	0	Minimum Backdrop Height (m)	0.500
CV	1.000	Preferred Cover Depth (m)	1.200
Time of Entry (mins)	5.00	Include Intermediate Ground	✓
Maximum Time of Concentration (mins)	30.00	Enforce best practice design rules	✓
Maximum Rainfall (mm/hr)	50.0		

Nodes

Name	Area (ha)	Cover Level (m)	Depth (m)
Garage Soakaway	0.002	88.000	1.000

Simulation Settings

Rainfall Methodology	FEH-22	Analysis Speed	Normal	Additional Storage (m ³ /ha)	20.0
Summer CV	1.000	Skip Steady State	x	Check Discharge Rate(s)	x
Winter CV	1.000	Drain Down Time (mins)	1440	Check Discharge Volume	x

Storm Durations

15	60	180	360	600	960	2160	4320	7200	10080
30	120	240	480	720	1440	2880	5760	8640	

Return Period (years)	Climate Change (CC %)	Additional Area (A %)	Additional Flow (Q %)
2	0	0	0
2	45	10	0
10	0	0	0
10	45	10	0

Node Garage Soakaway Soakaway Storage Structure

Base Inf Coefficient (m/hr)	0.00360	Invert Level (m)	87.000	Depth (m)	0.400
Side Inf Coefficient (m/hr)	0.00360	Time to half empty (mins)	4404	Inf Depth (m)	0.400
Safety Factor	2.0	Pit Width (m)	10.500	Number Required	1
Porosity	0.95	Pit Length (m)	1.000		



Results for 2 year Critical Storm Duration. Lowest mass balance: 100.00%

Node Event	US Node	Peak (mins)	Level (m)	Depth (m)	Inflow (l/s)	Node Vol (m ³)	Flood (m ³)	Status
240 minute winter	Garage Soakaway	156	87.035	0.035	0.1	0.3518	0.0000	OK

Link Event (Upstream Depth)	US Node	Link	Outflow (l/s)
240 minute winter	Garage Soakaway	Infiltration	0.0



Results for 2 year +45% CC +10% A Critical Storm Duration. Lowest mass balance: 100.00%

Node Event	US Node	Peak (mins)	Level (m)	Depth (m)	Inflow (l/s)	Node Vol (m ³)	Flood (m ³)	Status
480 minute winter	Garage Soakaway	312	87.073	0.073	0.1	0.7354	0.0000	OK

Link Event (Upstream Depth)	US Node	Link	Outflow (l/s)
480 minute winter	Garage Soakaway	Infiltration	0.0



Results for 10 year Critical Storm Duration. Lowest mass balance: 100.00%

Node Event	US Node	Peak (mins)	Level (m)	Depth (m)	Inflow (l/s)	Node Vol (m ³)	Flood (m ³)	Status
600 minute winter	Garage Soakaway	375	87.069	0.069	0.1	0.6891	0.0000	OK

Link Event (Upstream Depth)	US Node	Link	Outflow (l/s)
600 minute winter	Garage Soakaway	Infiltration	0.0



Results for 10 year +45% CC +10% A Critical Storm Duration. Lowest mass balance: 100.00%

Node Event	US Node	Peak (mins)	Level (m)	Depth (m)	Inflow (l/s)	Node Vol (m ³)	Flood (m ³)	Status
600 minute winter	Garage Soakaway	420	87.119	0.119	0.1	1.1937	0.0000	OK

Link Event (Upstream Depth)	US Node	Link	Outflow (l/s)
600 minute winter	Garage Soakaway	Infiltration	0.0