	Proj: Admin
	Ref :
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Beam/lintel section	

# Calculations for timber beams/lintels are in accordance with BS5268:Pt 2:2002

Number of parallel p Section size of each Timber type	bieces making up b n timber - Sawn Softw	eam/lintel - 47 ood as Tat	7 wide x 175 dee ble NA.2 of BS E	ep N 336		= 3	
Span of beam/lintel Span type						= 3 m - Simple	)
End bearing	- left hand end - right hand end				X Y X V	= 0 mm = 50 mm = 0 mm	n
End notches Strength class from	- left hand end - right hand end Table 8 (service cl	- - asses 1 &	none specified none specified 2)			- C24	
Service class Maximum design m Design shear force Design shear force	- 2 (Covered oment at left hand suppor at right hand suppo	and heated t ort	d or unheated)			= 3.26 k = 4.35 k = 4.35 k	:Nm :N :N
Load Description	<b>Type</b> UDL	<b>A</b> 0	<b>В</b> 3	С	1	<b>Gk</b> .5	<b>Qk</b> 1.4

### Grade stresses

Bending parallel to grain	= 7.5 N/mm <sup>2</sup>
Shear parallel to grain	= 0.71 N/mm <sup>2</sup>
Compression perpendicular to grain	= 2.4 N/mm <sup>2</sup>
(wane prohibited at bearing areas)	

## Modification factors

For service class 2	- moment	K <sub>2M</sub>	= 1
	- shear	K <sub>2V</sub>	= 1
	- bearing	K <sub>2B</sub>	= 1
	- Youngs mod	K <sub>2E</sub>	= 1
	- Shear mod	K <sub>2E</sub>	= 1
For load duration	- long	K <sub>3</sub>	= 1
For end bearing	- left end	К <sub>4I</sub>	= 1
	- right end	K <sub>4r</sub>	= 1
For no end notch	- left end	К <sub>.51</sub>	= 1
For no end notch	- right end	K <sub>5r</sub>	= 1
For no end notch	- right end	K <sub>5I</sub> K <sub>5r</sub>	= 1

For depth between 72 and 300mm	K <sub>7</sub>	= (300/h) <sup>0.11</sup>
		$= (300/175)^{0.11}$ = 1.06
For load sharing system	K	= 1.1
For 3 pieces of softwood	ĸ	= 1.21

The section is checked assuming that all the pieces of the beam/lintel act together as a load sharing system. The pieces must be connected together in such a manner to ensure this is achieved.

### **Bending Design**

The allowable bending stress is

$$\sigma_{\text{bpall}} = \sigma_{\text{bp}}^{*} K_{2M}^{*} K_{3}^{*} K_{7}^{*} K_{8}^{*}$$
  
= 7.5\*1\*1\*1.06\*1.1  
= 8.75 N/mm<sup>2</sup>  
$$z_{\text{reqd}} = M^{*} 10^{6} / \sigma_{\text{bpall}}^{*}$$
  
= 3.26\*106/8.75

The required section modulus is

= 3.26\*10<sup>6</sup>/8.75 = 372571 mm<sup>3</sup>

The section modulus of the beam/lintel chosen is 720000 mm^3.

#### Shear Design

Left hand support

The allowable shear stress is

	σ <sub>cpaall</sub>	= $\sigma_{cpa}^{*}K_{2V}^{*}K_{3}^{*}K_{5l}^{*}K_{8}^{*}$ = 0.71*1*1*1*1.1 = 0.78 N/mm <sup>2</sup>
The required cross sectional area is	A <sub>lreqd</sub>	= 3*R <sub>I</sub> *10 <sup>3</sup> /(2*σ <sub>cpaall</sub> ) = 3*4.35*10 <sup>3</sup> /(2*0.78) = 8365 mm <sup>2</sup>
The cross sectional area of the beam	n/lintel chose A <sub>lprov</sub>	n is = N <sub>tim</sub> *b*h = 3*47*175 = 24675 mm <sup>2</sup>
Right hand support		
The allowable shear stress is	σ <sub>cpaall</sub>	= $\sigma_{cpa}^{*}K_{2V}^{*}K_{3}^{*}K_{5r}^{*}K_{8}^{*}$ = 0.71*1*1*1*1.1 = 0.78 N/mm <sup>2</sup>
The required cross sectional area is	A <sub>rreqd</sub>	= $3^{*}R_{r}^{*}10^{3}/(2^{*}\sigma_{cpaall})$ = $3^{*}4.35^{*}10^{3}/(2^{*}0.78)$ = $8365 \text{ mm}^{2}$
The cross sectional area of the beam	n/lintel chose A <sub>rprov</sub>	n is = N <sub>tim</sub> *b*h = 3*47*175 = 24675 mm <sup>2</sup>

### **Bearing Design**

Left hand support

The allowable bearing stress is	$\sigma_{ ext{cpeall}}$	$= \sigma_{cpe} *K_{2B} *K_{3} *K_{41} *K_{8}$ = 2.4*1*1*1*1.1 = 2.64 N/mm <sup>2</sup>
The required bearing area is	A <sub>blreqd</sub>	= $R_1^{*10^3/\sigma_{cpeall}}$ = 4.35*10 <sup>3</sup> /2.64 = 1648 mm <sup>2</sup>
The bearing area of the beam/lintel of	hosen is A <sub>blprov</sub>	= N <sub>tim</sub> *b*Y = 3*47*50 = 7050 mm <sup>2</sup>
Right hand support		
The allowable bearing stress is	σ cpeall	$= \sigma_{cpe}^{*} K_{2B}^{*} K_{3}^{*} K_{4r}^{*} K_{8}$ = 2.4*1*1*1*1.1 = 2.64 N/mm <sup>2</sup>
The required bearing area is	A brreqd	= $R_r^{*10^{3}/\sigma}_{cpeall}$ = 4.35*10 <sup>3</sup> /2.64 = 1648 mm <sup>2</sup>
The bearing area of the beam/lintel o	hosen is A <sub>brprov</sub>	= N <sub>tim</sub> *b*Y <sub>r</sub> = 3*47*50 = 7050 mm <sup>2</sup>
Deflection check		

The deflection calculated includes for shear deflection and is based on the following material properties which incorporate modification factors  $\rm K_2$  and  $\rm K_9$  as appropriate.

- Young's modulus	- E	= 8712 N/mm <sup>2</sup>
- Shear modulus	- G	= 545 N/mm <sup>2</sup>
- Shape factor	- F	= 1.2 (for rectangular sections)
section properties of - Area	- A	= 247 cm <sup>2</sup>

and

- Alca	- A	
- Mom. of inertia	- 1	= 6300 cm <sup>4</sup>

The maximum calculated deflection is 5.8 mm.

The allowable deflection in accordance with clause 2.10.7 is 9 mm (0.003\*Span).

The section PASSES all the checks.