



# | Connecting Strength

## K2 Base Report

# Lincoln College - Museum Road - PV - Final

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Project address	9 Museum Rd, Oxford OX1 3PX, UK
Customer	Lincoln College
Company	Renewable Energy Co-operative
Author	Daniel Tempest
Issue date & version	11/04/2024   K2 Base Version 3.1.123.1



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## About us

### K2 Systems. Innovative mounting system from a strong team.

Since 2004 we have been developing pioneering and highly functional mounting system solutions for photovoltaic installations around the world. Our systems are designed in our own product development department where we continually optimize and adapt mounting systems to the ever-changing market.

#### A knowledgeable and friendly team

Just like a mountain climbing team, K2 Systems is built on mutual trust. This applies to our customer service as well as within the company itself, because we believe a trusting partnership leads to successful photovoltaic projects.

Our employees place total focus on the needs and wishes of our customer. This is true in all company departments.

#### 10 locations and worldwide sales network

In our international team, everyone works together to provide customers with competent, comprehensive and entirely personalized service.

This is especially true in the constant training our employees undergo with regards to product optimization, quality assurance, or innovations in construction techniques.

#### Quality management and certificates

K2 Systems stands for Connecting Strength, the highest quality, and precision-crafted and customized components. Our customers and business partners deeply appreciate all of these factors. Three independent authorities have tested, confirmed, and certified our skills and components. External authorities are not the only ones to have put K2 Systems to the test. Our internal quality control ensures that all our products are subject to a constant review process.

These measures all ensure the outstanding quality standards that exemplify products from K2 Systems, and which we maintain through largely exclusive "Made in Germany" or "Made in Europe" practices.



#### Product guarantee

K2 Systems offers a 12-year product warranty on all products in its integrated range. The use of high quality materials and a three-level quality inspection ensure these standards.

#### In a nutshell

As roof-top specialists, we offer effective and economical solutions for roofs all around the world and provide professional, fast and reliable support for our customers in the solar industry.

The static report is not including module and building verification.



# Project overview

## Roofs

Roof	System	Module	Height	Quantity	Total power
<a href="#">Roof 1</a> Tile	<a href="#">SolidRail</a>	Alpha Pure-RX 1,728×1,205×30 mm 470 Wp	9.00 m	26	12.22 kWp
<a href="#">Roof 2</a> Tile	<a href="#">SolidRail</a>	Alpha Pure-RX 1,728×1,205×30 mm 470 Wp	8.32 m	26	12.22 kWp
<a href="#">Roof 3</a> Tile	<a href="#">SolidRail</a>	Alpha Pure-RX 1,728×1,205×30 mm 470 Wp	9.00 m	6	2.82 kWp
<a href="#">Roof 4</a> Tile	<a href="#">SolidRail</a>	Alpha Pure-RX 1,728×1,205×30 mm 470 Wp	8.50 m	14	6.58 kWp
<a href="#">Roof 5</a> Tile	<a href="#">SolidRail</a>	REC 420 AA Pure-R (Alpha) 1,730×1,118×30 mm 420 Wp	9.32 m	8	3.36 kWp
<b>Total</b>				<b>80</b>	<b>37.20 kWp</b>

## Project information

Address	9 Museum Rd, Oxford OX1 3PX, UK
Customer	Lincoln College
Contact person	Peter Nitsche-Whitfield
Author	Daniel Tempest

## Load settings

Design method	BS EN
Failure consequence class (CC)	CC2
Design working life	25 years
Terrain category	Town Terrain
Basic wind speed	21.5 m/s
Distance to shoreline	100.00 km
Distance inside town terrain	0.20 km
Snow load zone	2
Snow load on ground level	0.40 kN/m <sup>2</sup>

## Material values

**Aluminium EM-AW 6063 (EP, ET, ER/B) T66**



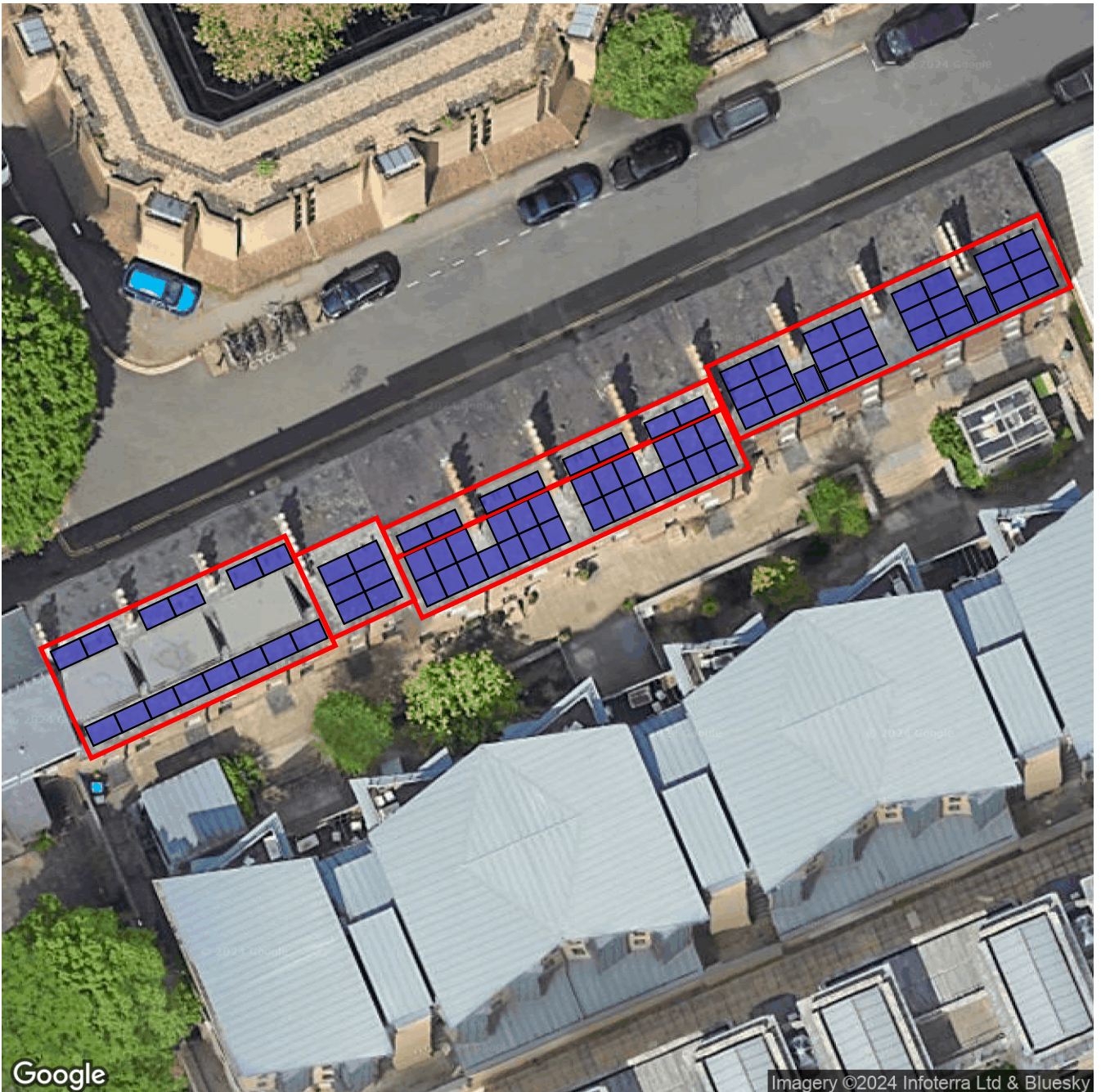
## Project overview

Elastic module	$E = 70.000 \text{ N/mm}^2$
Shear module	$G = 26.923 \text{ N/mm}^2$
Density	$g = 2.700 \text{ kg/m}^3$
Thermal coefficient	$\alpha_T = 2.3e^{-5}$
Yielding strength	$f_{o,k} = 200 \text{ N/mm}^2$
Ultimate strength	$f_{u,k} = 245 \text{ N/mm}^2$



THE PROJECT IS VERIFIED.  
please check the warning(s)!

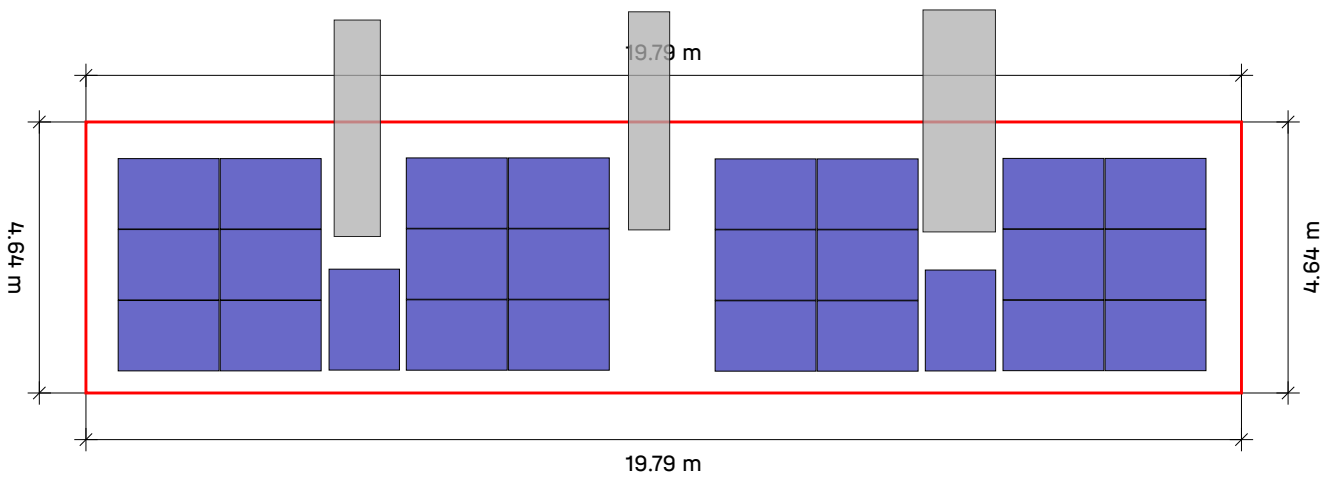
# Lincoln College - Museum Road - PV - Final



## Project information

Address	9 Museum Rd, Oxford OX1 3PX, UK
Customer	Lincoln College
Contact person	Peter Nitsche-Whitfield
Author	Daniel Tempest

# Roofs | Roof 1



Roof	System	Module	Height	Quantity	Total power
<a href="#">Roof 1</a>	<a href="#">SolidRail</a>	Alpha Pure-RX 1,728×1,205×30 mm 470 Wp	9.00 m	26	12.22 kWp
Tile					

# Roofs | Roof 1 | Assembly plan

## Base Rails

Type	Whole Rails		Rail cutting		
	Total Rail Length	Quantity 3.30 m	Part of Rail	Length	Rest
2*A	1.305		3.300	1.305 from 3.300	<u>1.985</u>
2*B	1.305		<u>1.985</u>	1.305 from 1.985	0.670
2*C	3.576	1*3.30 m	3.300	0.700 from 3.300	<u>2.590</u>
2*D	3.576	1*3.30 m	<u>2.590</u>	0.700 from 2.590	<u>1.880</u>
2*E	3.576	1*3.30 m	<u>1.880</u>	0.700 from 1.880	<u>1.170</u>
2*F	3.576	1*3.30 m	<u>1.170</u>	0.700 from 1.170	0.460
4*G	3.576	1*3.30 m	3.300	0.700 from 3.300	<u>2.590</u>
4*H	3.576	1*3.30 m	<u>2.590</u>	0.700 from 2.590	<u>1.880</u>
4*I	3.576	1*3.30 m	<u>1.880</u>	0.700 from 1.880	<u>1.170</u>
4*J	3.576	1*3.30 m	<u>1.170</u>	0.700 from 1.170	0.460

1 cm is viewed as lost for each cutting

Red numbers are leftover rails which will not be used any longer

## Fastener Spacing

Module	Array	Distance	maximum cantilever length	maximum fastener spacing
1	ridge	1.80 m	0.573	1.803
1	gableboard	1.80 m	0.542	1.803
1	corner region (eave)	1.35 m	0.563	1.598
2	ridge	1.80 m	0.573	1.803
2	eaves	1.35 m	0.626	1.747
3	ridge	1.80 m	0.573	1.803
3	eaves	1.35 m	0.626	1.747
4	ridge	1.80 m	0.573	1.803
4	gableboard	1.80 m	0.542	1.803





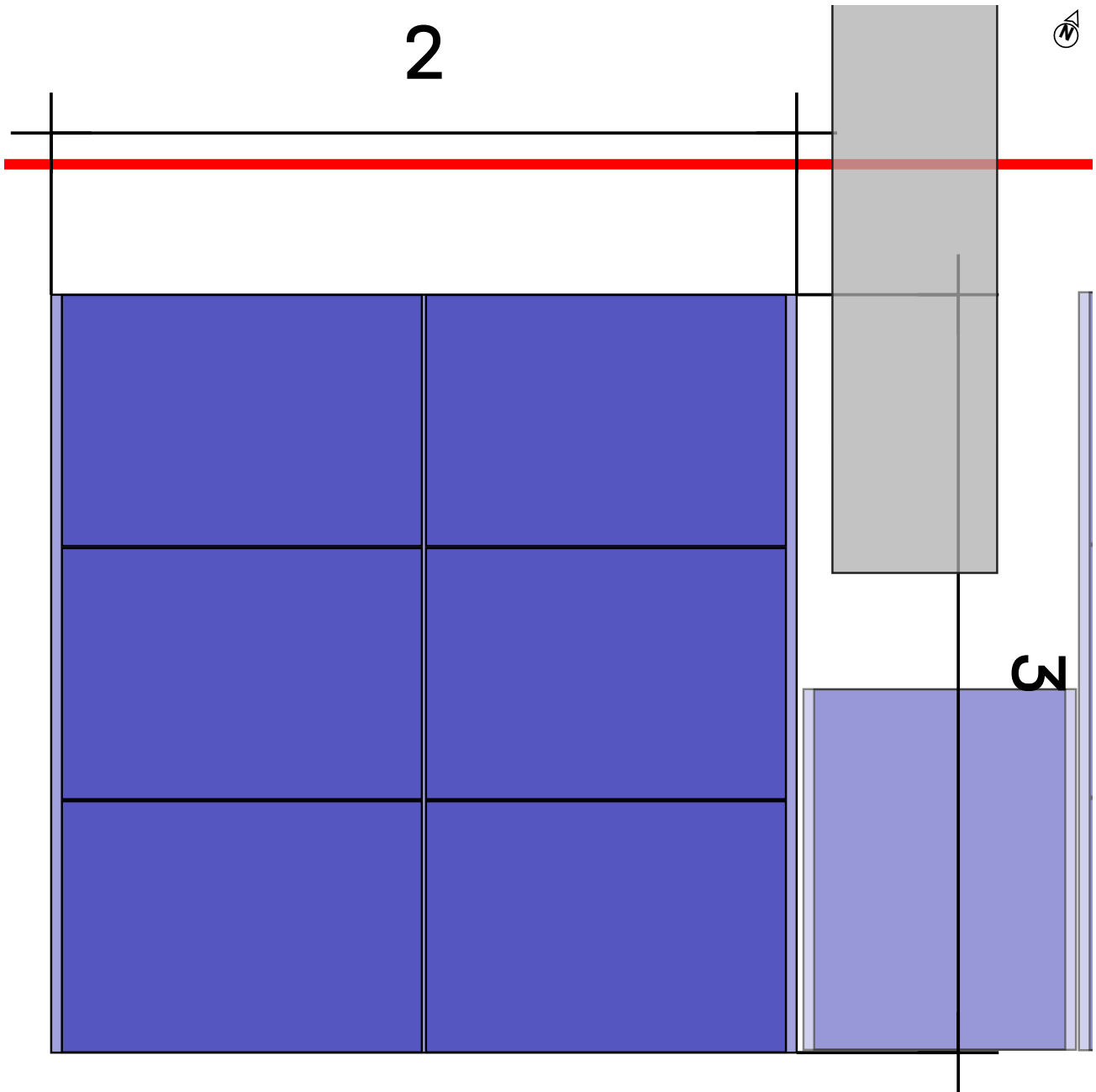
## Roofs | Roof 1 | Assembly plan

4	corner region (eave)	1.35 m	0.563	1.598
5	field area	0.90 m	0.616	1.257
5	corner region (eave)	0.90 m	0.550	1.115
5	eaves	0.90 m	0.610	1.218
6	field area	0.90 m	0.616	1.257
6	corner region (eave)	0.90 m	0.550	1.115
6	eaves	0.90 m	0.610	1.218

### Module arrays

Module array	Width[m]	Length[m]	Width in modules	Length in modules
1	3.48	3.64	2	3
2	3.48	3.64	2	3
3	3.48	3.63	2	3
4	3.48	3.64	2	3
5	1.21	1.73	1	1
6	1.21	1.73	1	1

# Roofs | Roof 1 | Module array 1



Roof ① Module array ①

Mounting System

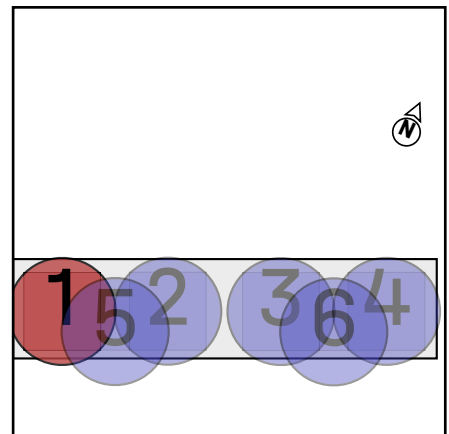
[SolidRail](#)

Module

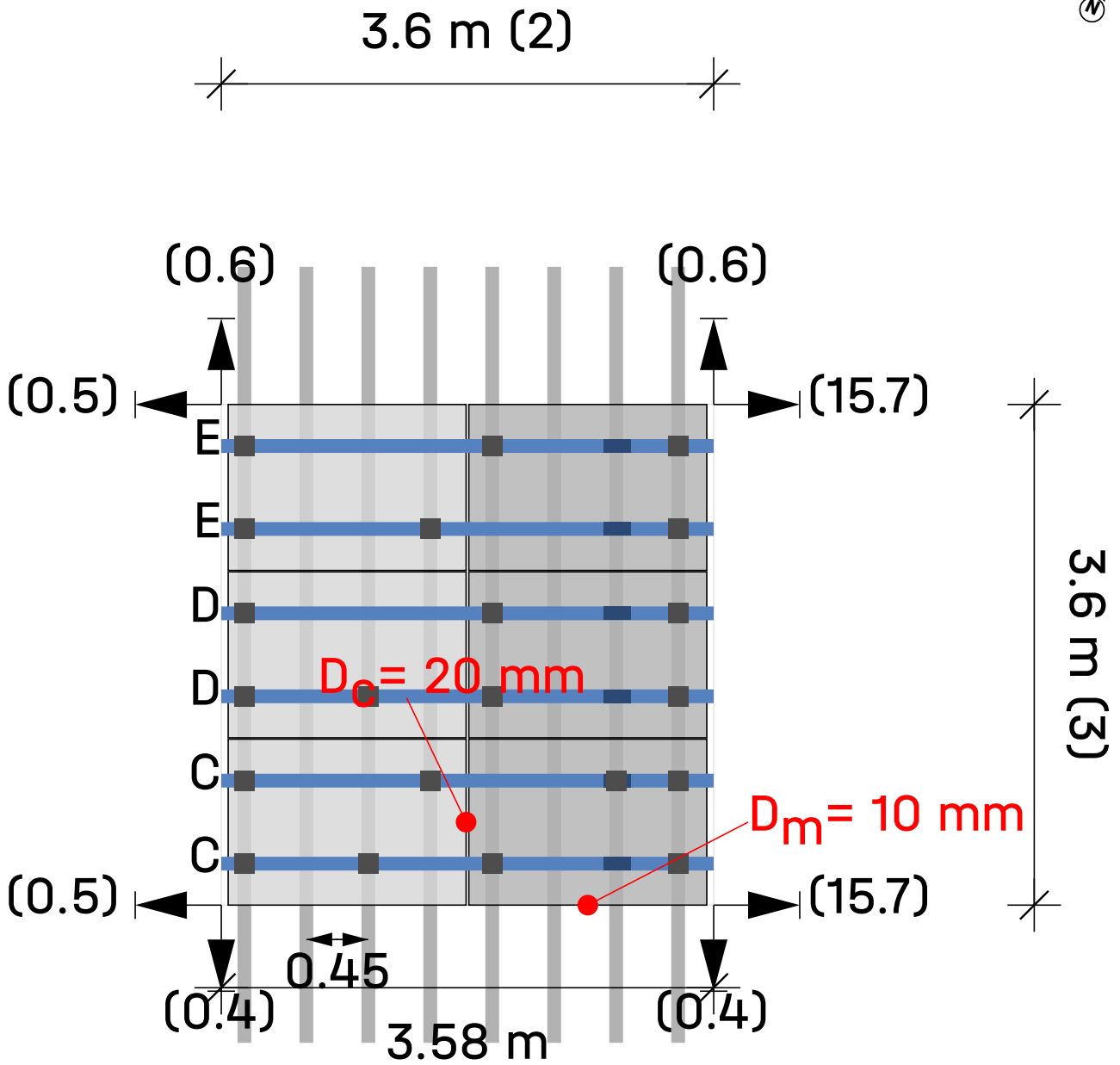
6(2.82 kWp) x Alpha Pure-RX

Row spacing

1.22 m



Roofs | Roof 1 | Module array 1 | Module blocks



Roof ① Module array ① Module block ①

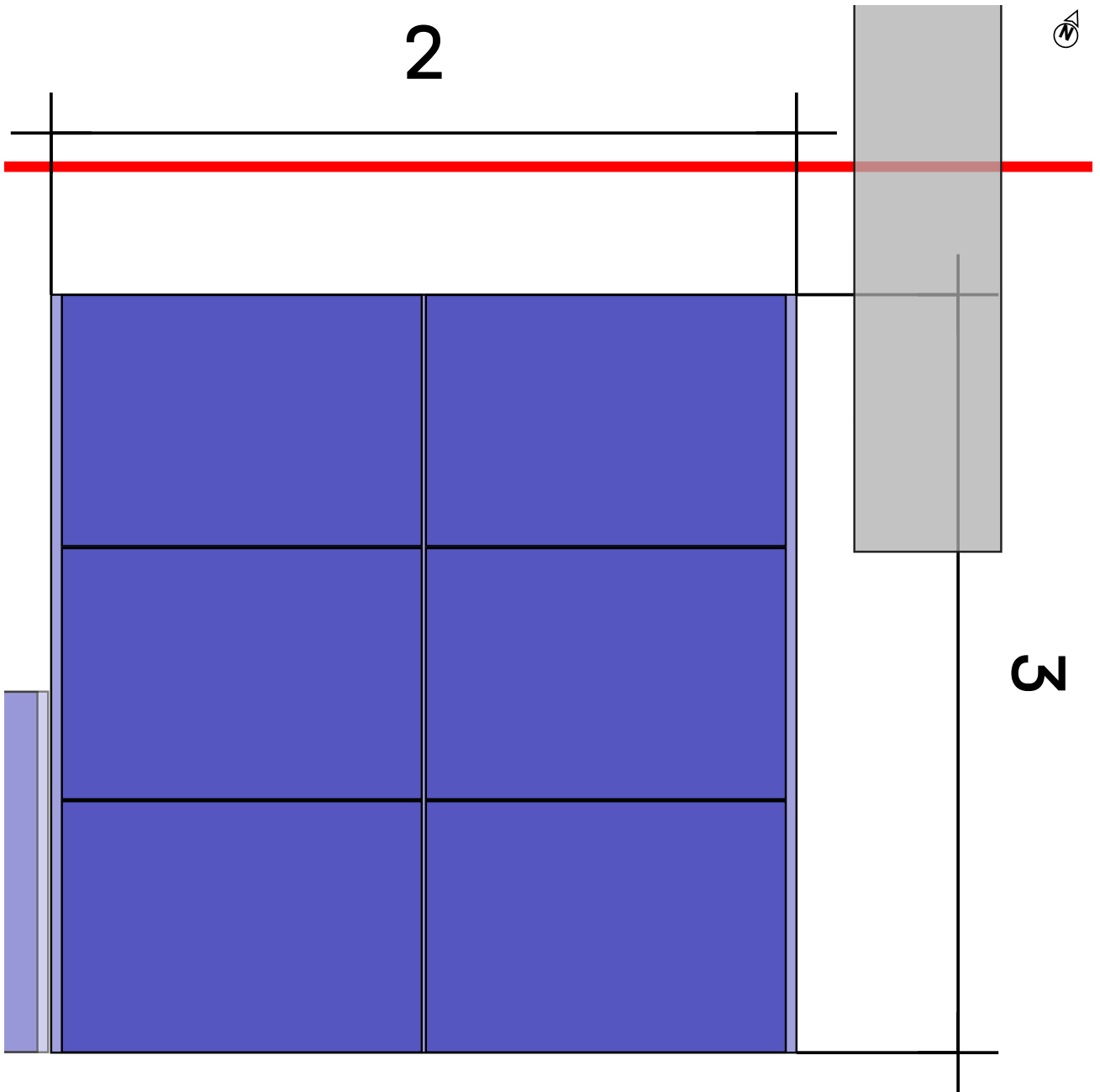
Modules 2 × 3 = 6

Legend

- Fastener
- Mounting rail: K2 SolidRail Light 37
- Distance to Roof Edge [m]
- $D_c$  Distance for clamping between modules
- $D_m$  Distance between modules



# Roofs | Roof 1 | Module array 2



Roof ① Module array ②

Mounting System

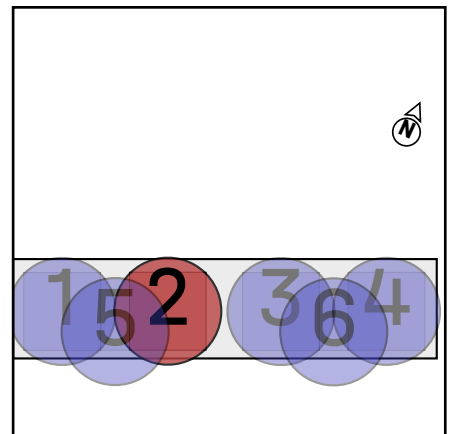
[SolidRail](#)

Module

6(2.82 kWp) x Alpha Pure-RX

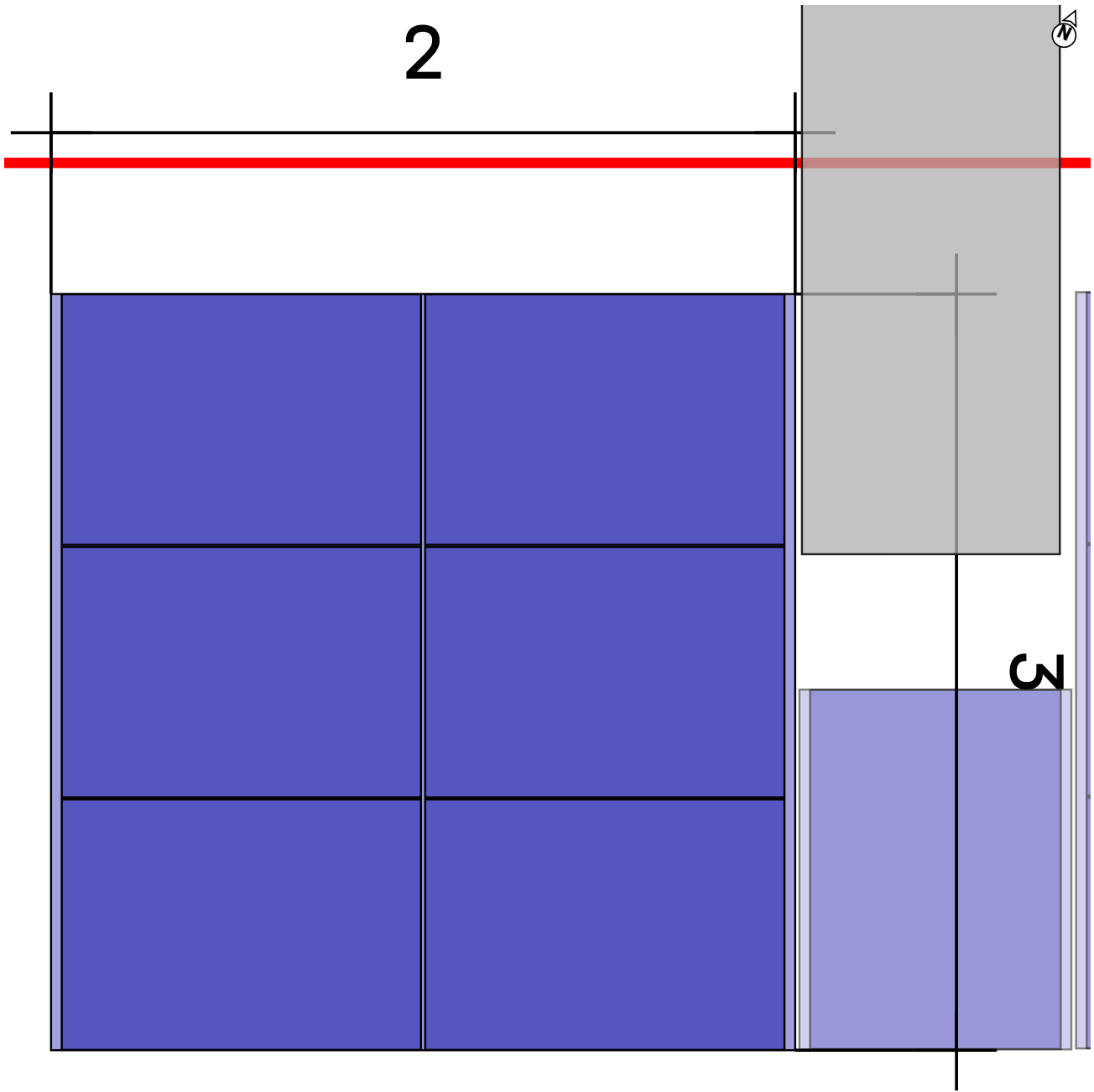
Row spacing

1.22 m





# Roofs | Roof 1 | Module array 3



Roof ① Module array ③

Mounting System

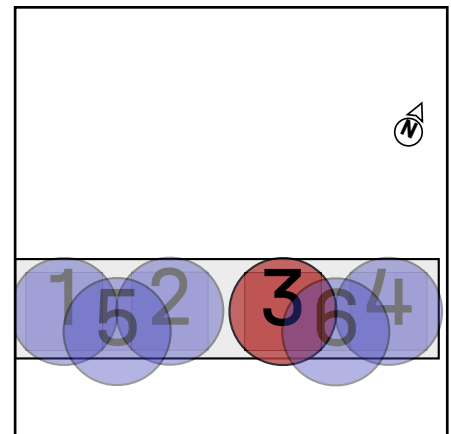
[SolidRail](#)

Module

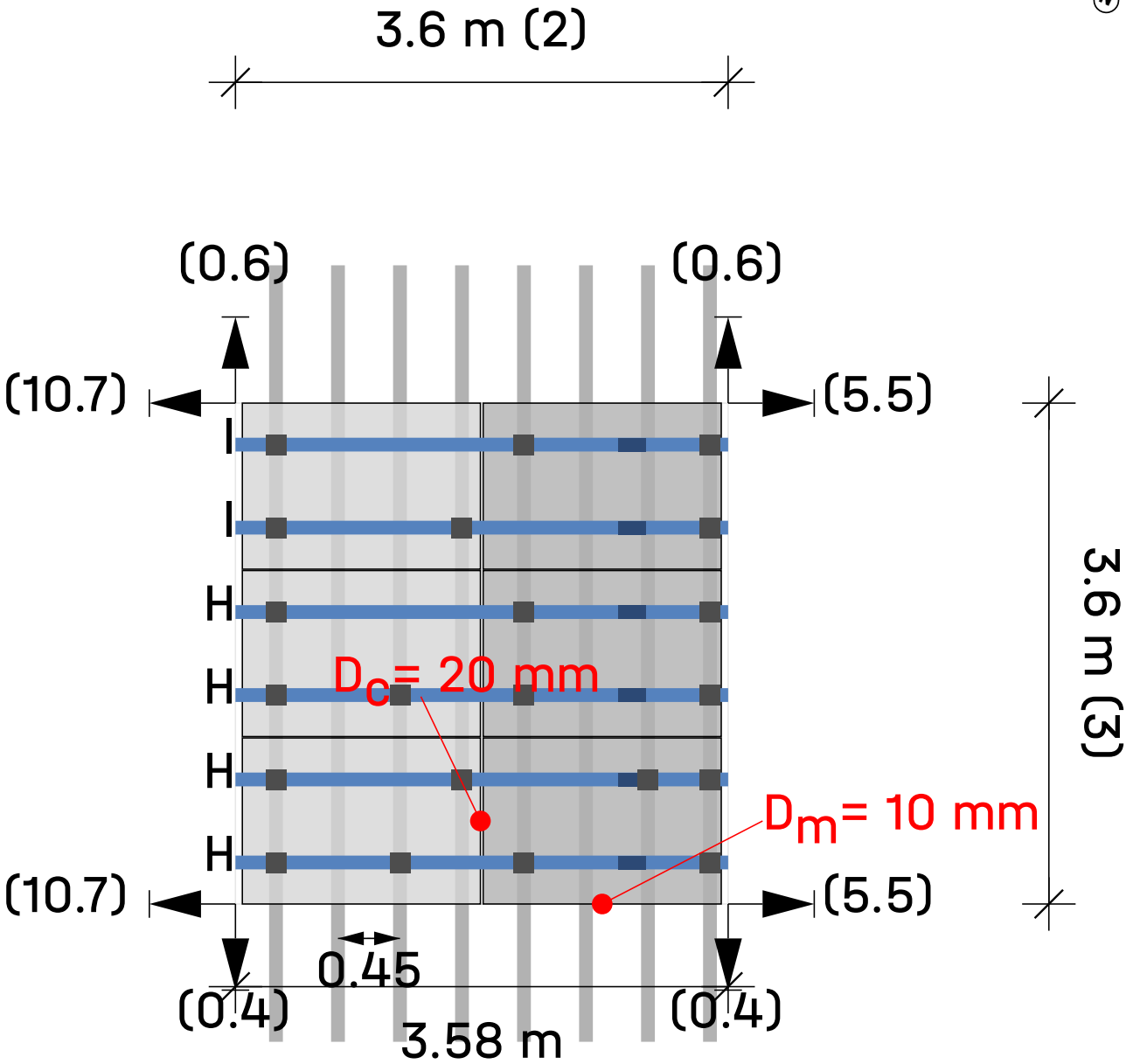
6(2.82 kWp) x Alpha Pure-RX

Row spacing

1.22 m



Roofs | Roof 1 | Module array 3 | Module blocks



Roof ① Module array ③ Module block ③

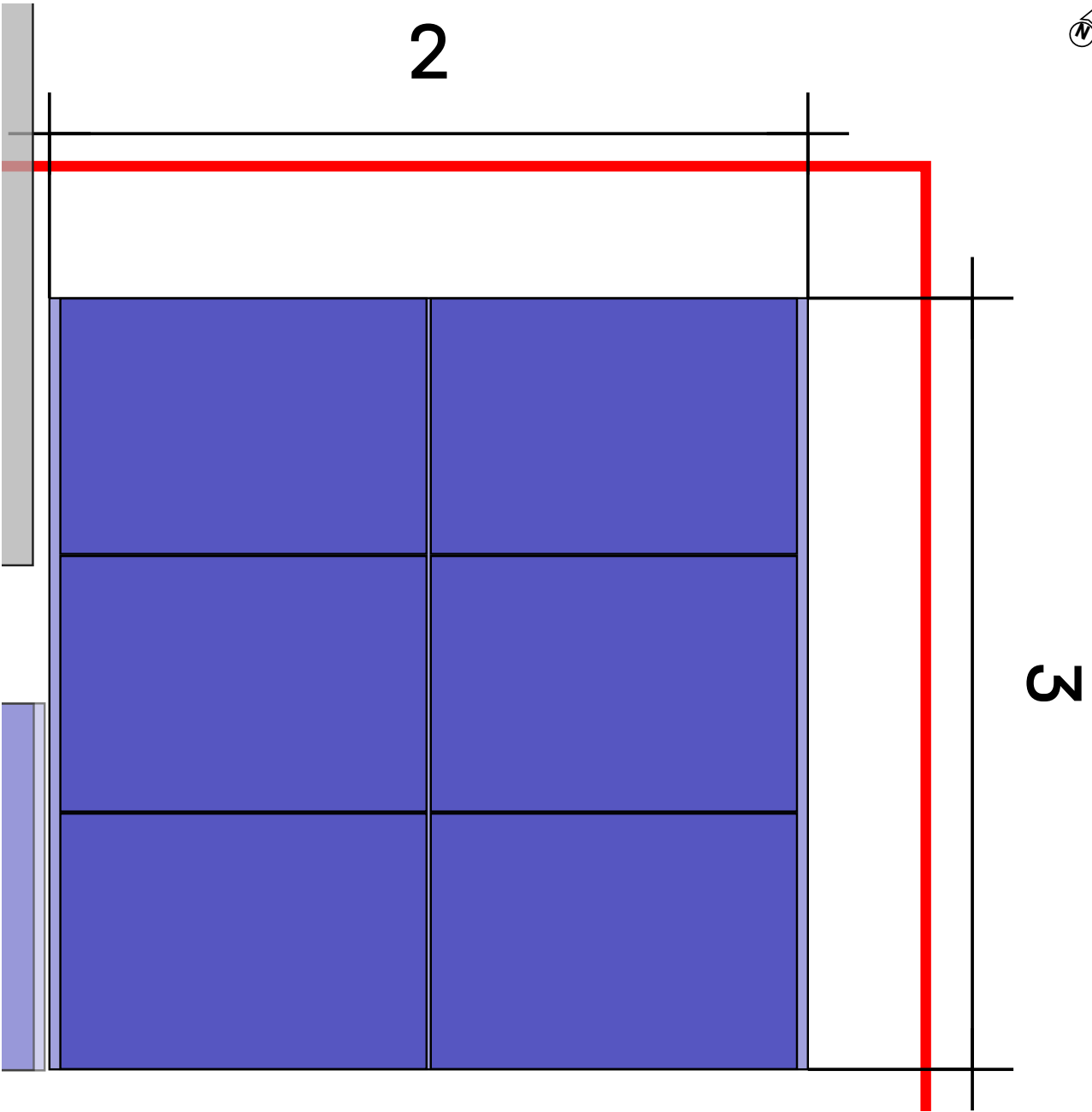
Modules  $2 \times 3 = 6$

Legend

- Fastener
- Mounting rail: K2 SolidRail Light 37
- Distance to Roof Edge [m]
- $D_c$  Distance for clamping between modules
- $D_m$  Distance between modules



# Roofs | Roof 1 | Module array 4



Roof ① Module array ④

Mounting System

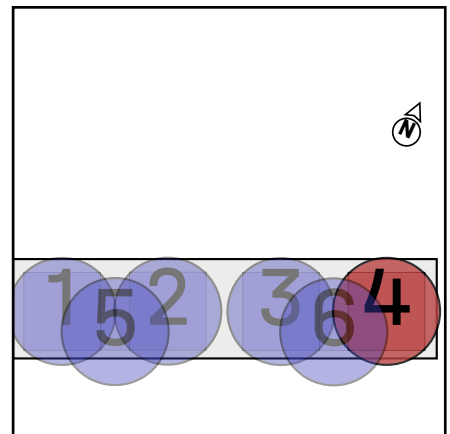
[SolidRail](#)

Module

6(2.82 kWp) x Alpha Pure-RX

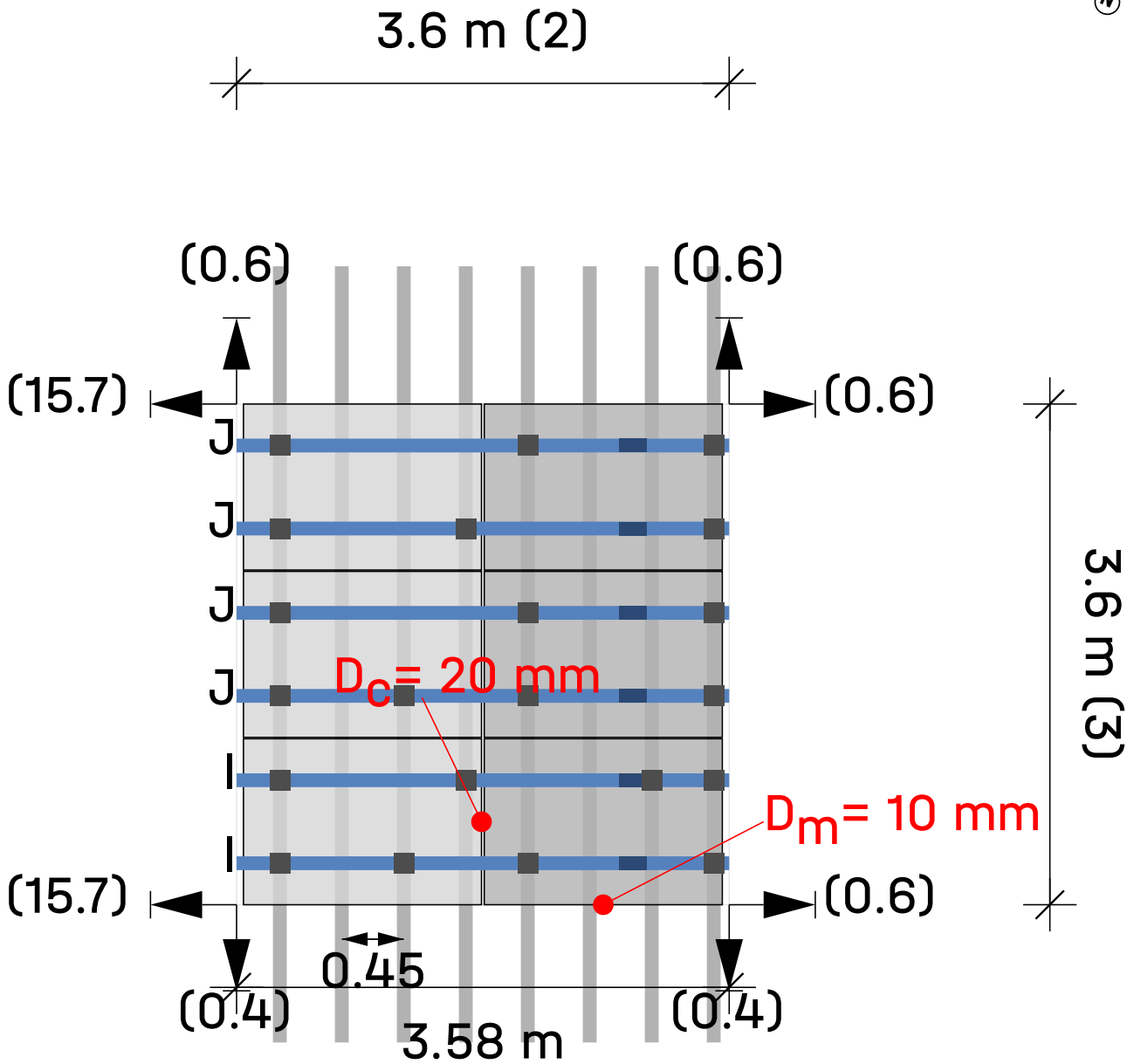
Row spacing

1.22 m





Roofs | Roof 1 | Module array 4 | Module blocks



Roof ① Module array ④ Module block ④

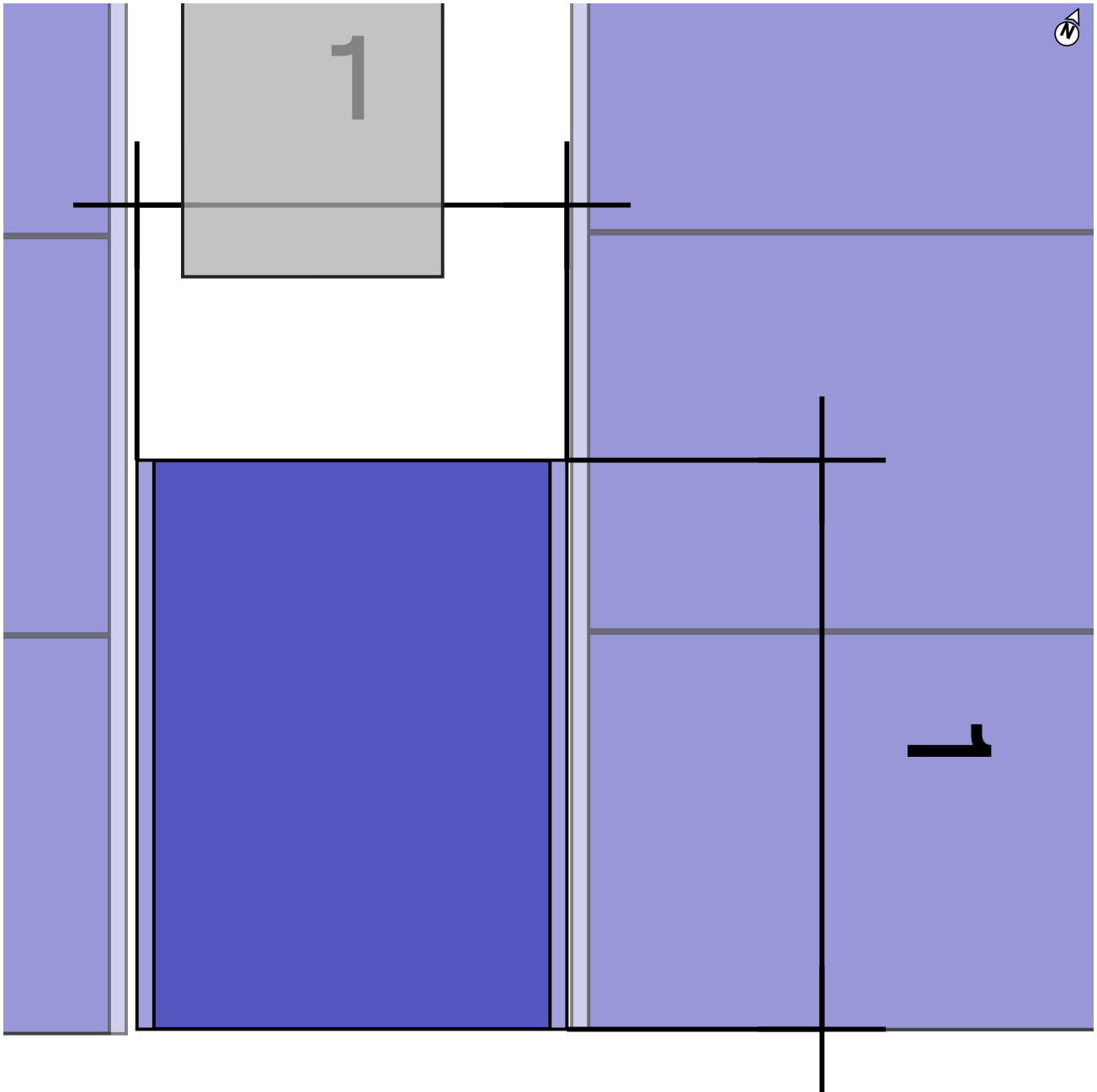
Modules  $2 \times 3 = 6$

Legend

- Fastener
- Mounting rail: K2 SolidRail Light 37
- Distance to Roof Edge [m]
- $D_c$  Distance for clamping between modules
- $D_m$  Distance between modules



# Roofs | Roof 1 | Module array 5



Roof ① Module array ⑤

Mounting System

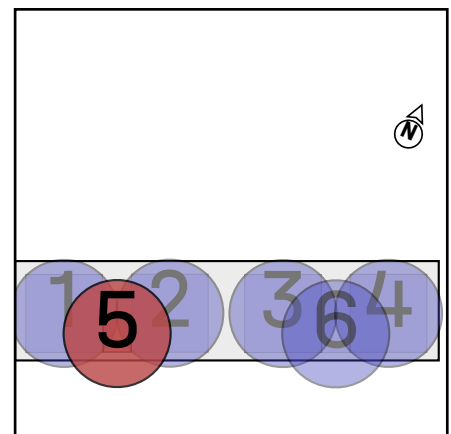
[SolidRail](#)

Module

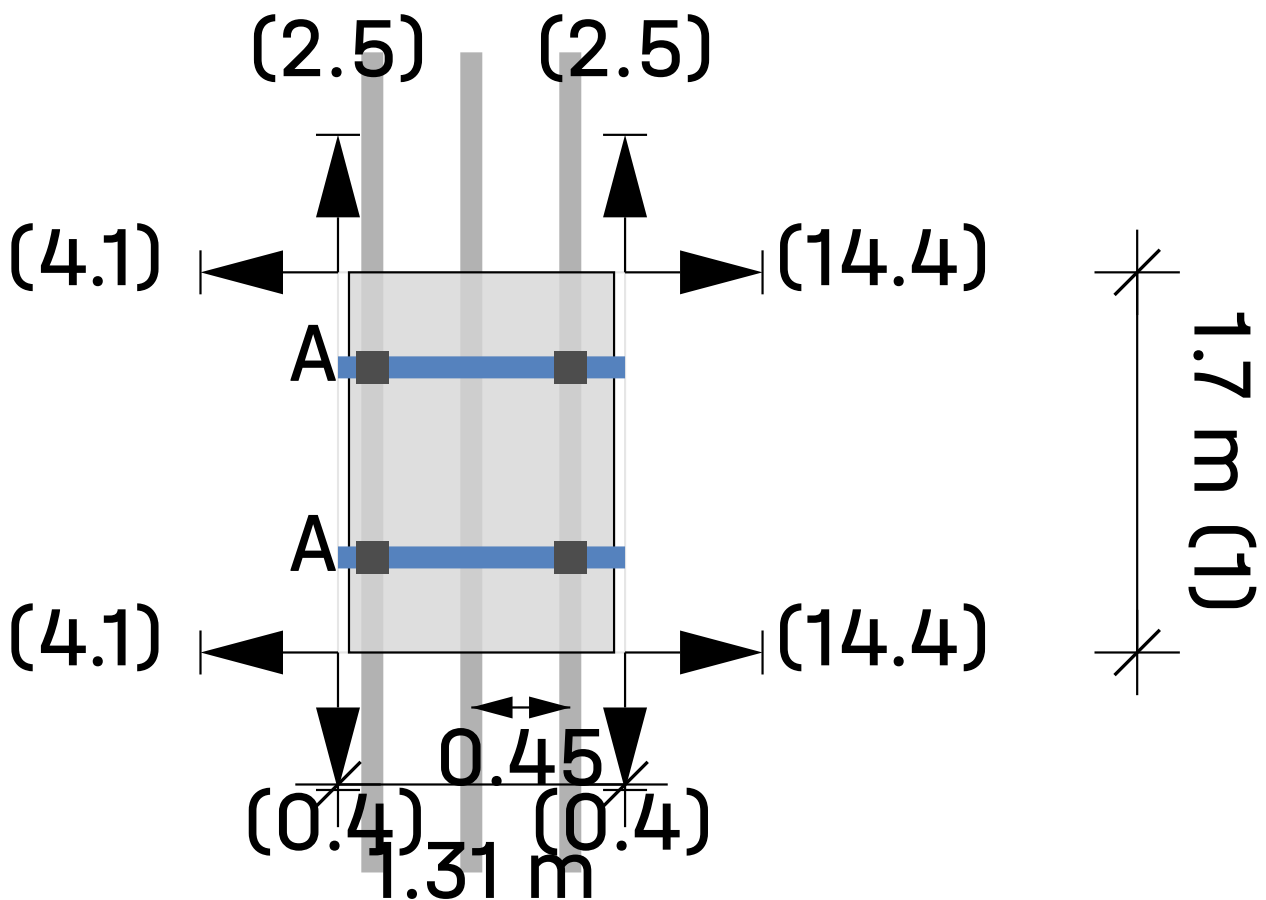
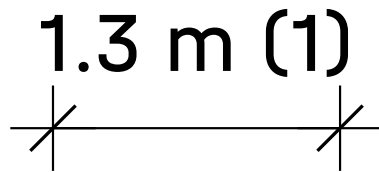
1(0.47 kWp) x Alpha Pure-RX

Row spacing

1.74 m



Roofs | Roof 1 | Module array 5 | Module blocks

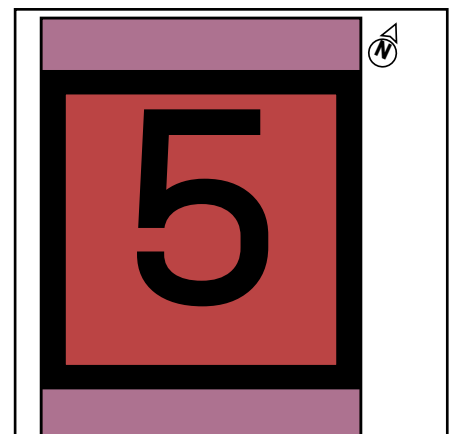


Roof ① Module array ⑤ Module block ⑤

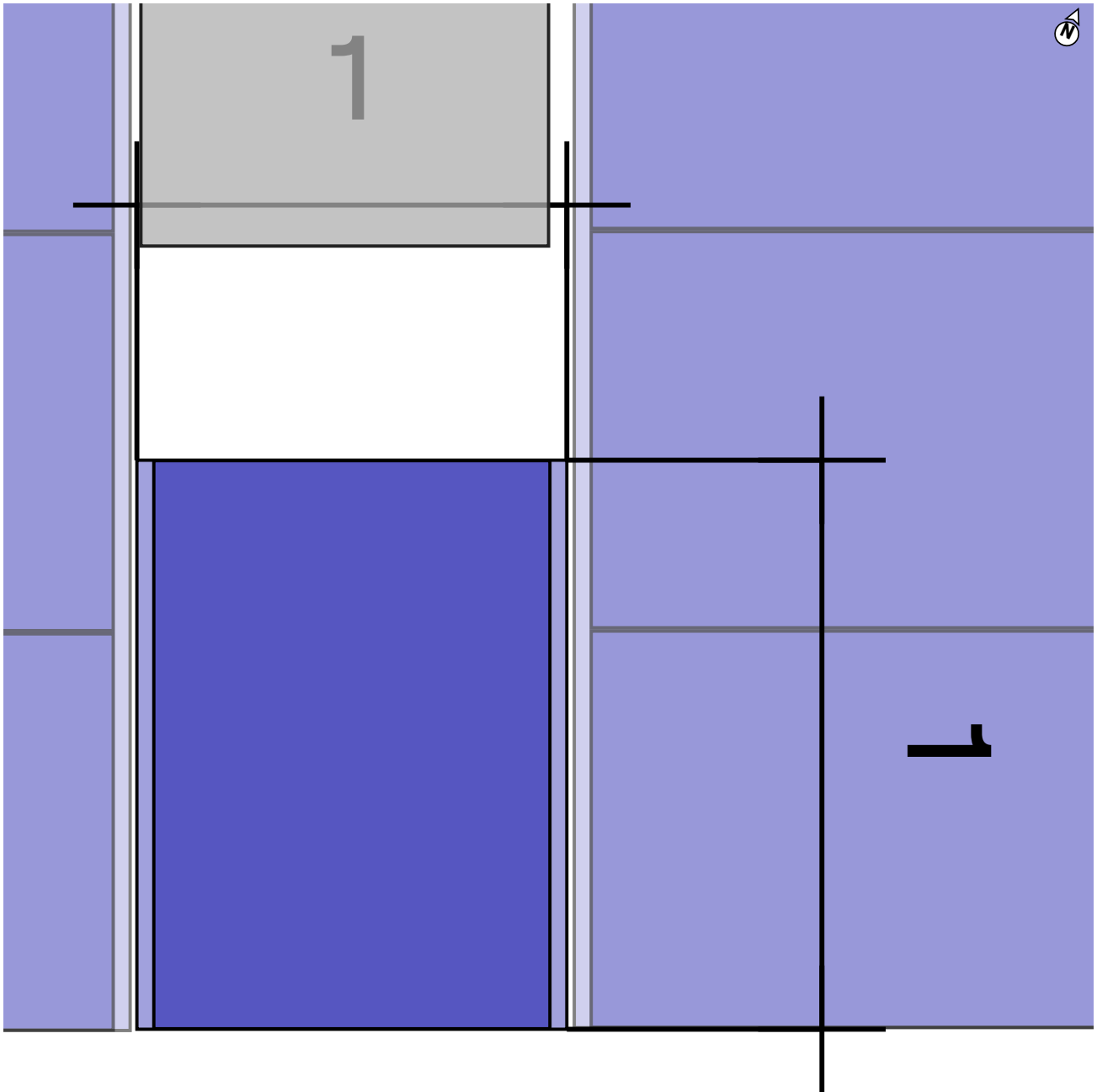
Modules 1 × 1 = 1

Legend

- Fastener
- Mounting rail: K2 SolidRail Light 37
- Distance to Roof Edge [m]
- D<sub>c</sub>** Distance for clamping between modules
- D<sub>m</sub>** Distance between modules



# Roofs | Roof 1 | Module array 6



Roof ① Module array ⑥

Mounting System

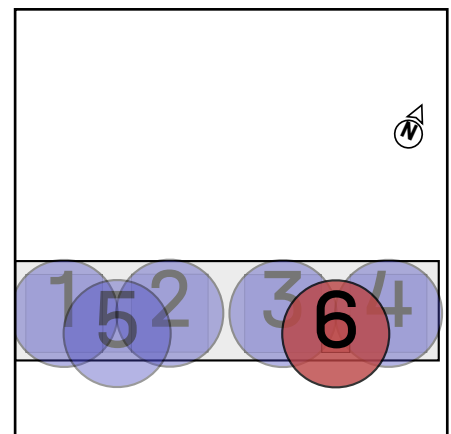
[SolidRail](#)

Module

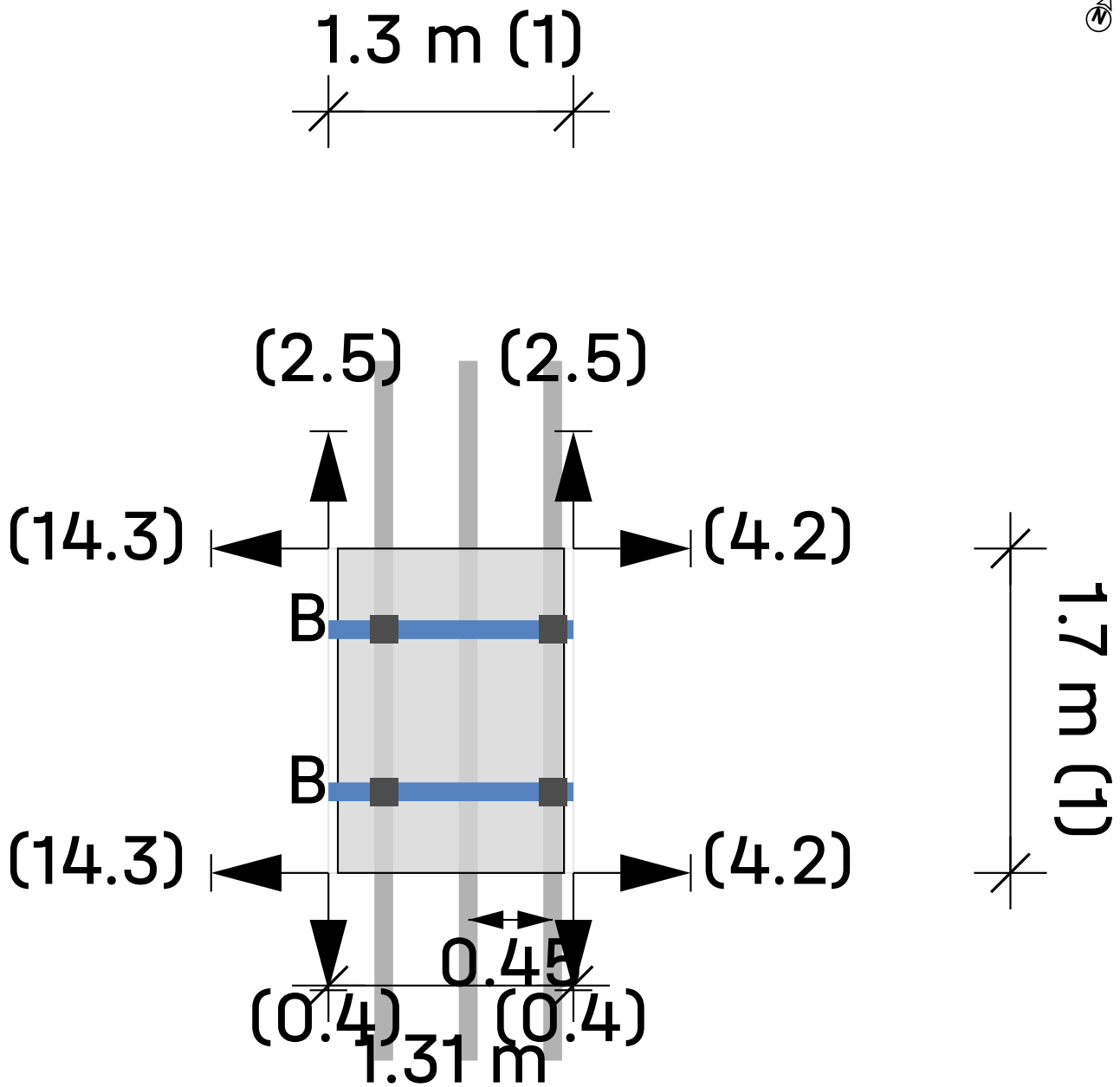
1(0.47 kWp) x Alpha Pure-RX

Row spacing

1.74 m



Roofs | Roof 1 | Module array 6 | Module blocks

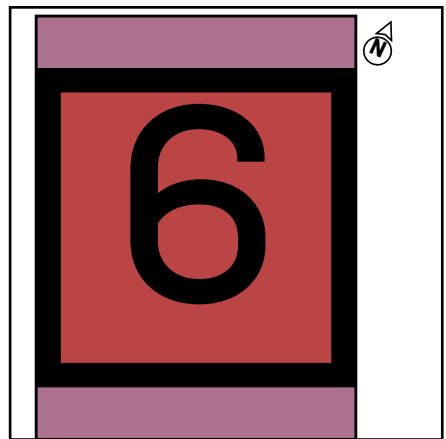


Roof ① Module array ⑥ Module block ⑥


Modules 1 × 1 = 1

Legend

- Fastener
- Mounting rail: K2 SolidRail Light 37
- Distance to Roof Edge [m]
- D<sub>c</sub>** Distance for clamping between modules
- D<sub>m</sub>** Distance between modules



# Results | Roof 1

Roof	System	Module	Height	Quantity	Total power
<a href="#">Roof 1</a>	<a href="#">SolidRail</a>	Alpha Pure-RX 1,728×1,205×30 mm 470 Wp	9.00 m	26	12.22 kWp
	Tile				

## Module

Name	Alpha Pure-RX
Manufacturer	REC
Output power	470 Wp
Dimensions	1,728×1,205×30 mm
Weight	23.4 kg

## Components

Fastener	SolidHook Pan Tile Vario 1
Base rails	K2 SolidRail Light 37

## Loads on modules (module dimensioning)

Array	A-TrA [m <sup>2</sup> ]	ultimate state [Pa]				Serviceability [Pa]			
		Pressure ⊥	Pressure	Uplift ⊥	Uplift	Pressure ⊥	Pressure	Uplift ⊥	Uplift
ridge	2.08	802.3	252.7	-1,005.7	45.7	546.2	173.0	-632.8	45.7
gableboard	2.08	802.3	252.7	-1,206.7	45.7	546.2	173.0	-766.8	45.7
corner region (eave)	2.08	914.9	252.7	-1,307.2	45.7	621.2	173.0	-833.8	45.7
ridge	2.08	802.3	252.7	-1,005.7	45.7	546.2	173.0	-632.8	45.7
eaves	2.08	830.5	252.7	-550.2	45.7	565.0	173.0	-329.1	45.7
ridge	2.08	802.3	252.7	-1,005.7	45.7	546.2	173.0	-632.8	45.7
eaves	2.08	830.5	252.7	-550.2	45.7	565.0	173.0	-329.1	45.7
ridge	2.08	802.3	252.7	-1,005.7	45.7	546.2	173.0	-632.8	45.7
gableboard	2.08	802.3	252.7	-1,206.7	45.7	546.2	173.0	-766.8	45.7
corner region (eave)	2.08	914.9	252.7	-1,307.2	45.7	621.2	173.0	-833.8	45.7
field area	2.08	802.3	252.7	-489.9	45.7	546.2	173.0	-288.9	45.7
corner region (eave)	2.08	914.9	252.7	-1,307.2	45.7	621.2	173.0	-833.8	45.7
eaves	2.08	830.5	252.7	-550.2	45.7	565.0	173.0	-329.1	45.7
field area	2.08	802.3	252.7	-489.9	45.7	546.2	173.0	-288.9	45.7
corner region (eave)	2.08	914.9	252.7	-1,307.2	45.7	621.2	173.0	-833.8	45.7



# Results | Roof 1

eaves                                      2.08      830.5      252.7      -550.2      45.7                                      565.0      173.0      -329.1      45.7

## Utilisation result

No.	roof areas	ultimate limit state			Usab.	Distances		maximum values	
		Pr $\sigma$ [%]	CL $\sigma$ [%]	Fst F[%]	Pr f[%]	Fst [m]	BR [m]	CL $L_{max}$ [m]	Fst D <sub>max</sub> [m]
1	ridge	49.4	5.1	99.8	59.5	1.800	---	0.573	1.803
1	gableboard	59.0	2.6	99.8	72.1	1.800	---	0.542	1.803
1	corner region (eave)	35.9	6.6	84.5	27.6	1.350	---	0.563	1.598
2	ridge	49.4	4.5	99.8	59.5	1.800	---	0.573	1.803
2	eaves	26.6	4.3	77.3	19.0	1.350	---	0.626	1.747
3	ridge	49.4	6.6	99.8	59.5	1.800	---	0.573	1.803
3	eaves	26.6	6.3	77.3	19.0	1.350	---	0.626	1.747
4	ridge	49.4	7.5	99.8	59.5	1.800	---	0.573	1.803
4	gableboard	59.0	1.1	99.8	72.1	1.800	---	0.542	1.803
4	corner region (eave)	35.9	9.7	84.5	27.6	1.350	---	0.563	1.598
5	field area	16.5	0.0	71.6	7.8	0.900	---	0.616	1.257
5	corner region (eave)	22.9	3.4	80.8	11.7	0.900	---	0.550	1.115
5	eaves	17.0	6.5	73.9	8.1	0.900	---	0.610	1.218
6	field area	16.5	0.0	71.6	7.8	0.900	---	0.616	1.257
6	corner region (eave)	22.9	1.7	80.8	11.7	0.900	---	0.550	1.115
6	eaves	17.0	9.1	73.9	8.1	0.900	---	0.610	1.218

Pr      Profile                                      Fst D<sub>max</sub>      maximum fastener spacing  
 Fst      Fastener                                      BR      Base Rail  
 $\sigma$       Stress                                      Usab.      serviceability limit state  
 f      Deflection                                      CL      Cantilever  
 F      Force  
 CL/ $L_{max}$       maximum cantilever length



# Results | Roof 1

## Notes

- The dimensioning of the timber construction screws is not part of this structural analysis. The dimensioning and positioning of the timber construction screws to be used must be carried out in accordance with the respective applicable codes of practice.
- The structure was statically verified in accordance with Eurocode 9: Design of aluminum structures (DIN EN 1999-1-1:2021) and offers sufficient load-bearing capacity and stability for the loads specified in the chapter 'Maximum actions on the components'.
- Adjustment factor for wind load regarding service life period,  $f_W$ , is according to DIN EN 1991-1-4/ NA, NDP for 4.2 (2P) note 5, table 3
- Adjustment factor for snow load regarding service life period,  $f_S$ , is according to DIN EN 1991-1-3/ annex D, table 4
- The structural design complies with BS EN 1990 - Basis of Structural Design.
- Snow loads are determined in accordance with National Annex BS NA EN 1991-1-3 (2018) - UK National Annex to EC1 - Action on structures - general actions - snow loads.
- Wind loads are determined in accordance with National Annex BS NA EN 1991-1-4 - UK National Annex to EC1 - Action on structures, general actions - wind actions.
- Service life is recognised according to 'Eurocode EN 1991 - Action on structures, Snow loads' and 'Eurocode EN 1991 - Actions on structures, Wind actions'. Subject to the Building Regulations and for security-relevant reasons the installation has to be dismantled at the end of its service life.
- Failure consequence class is considered according to 'Eurocode EN 1990 - Basis of structural design'.
- Data and results must be verified with regard to local conditions and checked by a suitably qualified person. Please see our TCU under <https://k2-systems.com/en/base-tcu>, in particular § 2 ("technical and specialist requirements for the customer"), § 7 ("warranty provisions") and § 8 ("limitation of liability").
- The clamping range of the module may not comply with for this installation variant, as the rail spacing depends on the laying of the tiles and thus the batten length. Please ensure that the clamping range specified by the module manufacturer is observed.



# Structural analysis report | Roof 1

## General information

Name	Lincoln College - Museum Road - PV - Final
Mounting System	SolidRail
Author	Daniel Tempest

## Location information

Address	9 Museum Rd, Oxford OX1 3PX, UK
Ground elevation	65.14 m

## Roof information

Building height	9.00 m
Roof type	Gable roof
Roof pitch	22°
Roof covering	Tile
Min. roof edge distance	0.00 m
Rafter Spacing	0.450 m
Rafter width	50.0 mm
set edge rafter left	No
Rafter spacing left	220.0 mm
Rafter spacing right	No
Rafter Spacing	220.0 mm
Batten spacing	340.0 mm

## Loads

Design method	BS EN
Failure consequence class (CC)	CC2
Design working life	25 years
Terrain category	Town Terrain
Distance to shoreline	100.00 km
Distance inside town terrain	0.20 km

## Wind load

Velocity pressure, 50	$q_{p,50} = 0.727 \text{ kN/m}^2$
Adjustment factor for service life	$f_w = 1.000$
Velocity pressure, 25	$q_{p,25} = 0.670 \text{ kN/m}^2$



# Structural analysis report | Roof 1

## Roof areas

Array	load impact area [m <sup>2</sup> ]	maxCpe <sub>10</sub>	minCpe <sub>10</sub>	wind pressure [kN/m <sup>2</sup> ]	wind suction [kN/m <sup>2</sup> ]
ridge	10.00	0.293	-1.113	0.197	-0.746
gableboard	10.00	0.293	-1.313	0.197	-0.880
corner region (eave)	10.00	0.480	-1.413	0.322	-0.947
ridge	10.00	0.293	-1.113	0.197	-0.746
eaves	10.00	0.340	-0.660	0.228	-0.442
ridge	10.00	0.293	-1.113	0.197	-0.746
eaves	10.00	0.340	-0.660	0.228	-0.442
ridge	10.00	0.293	-1.113	0.197	-0.746
gableboard	10.00	0.293	-1.313	0.197	-0.880
corner region (eave)	10.00	0.480	-1.413	0.322	-0.947
field area	10.00	0.293	-0.600	0.197	-0.402
corner region (eave)	10.00	0.480	-1.413	0.322	-0.947
eaves	10.00	0.340	-0.660	0.228	-0.442
field area	10.00	0.293	-0.600	0.197	-0.402
corner region (eave)	10.00	0.480	-1.413	0.322	-0.947
eaves	10.00	0.340	-0.660	0.228	-0.442

## Snow load

Snow load zone	2
Snow guard	No
Snow load on ground level	$s_k = 0.400 \text{ kN/m}^2$
Shape Coefficient for Snow	$\mu_i = 0.800$
Factor for roof pitch	$d_r = 0.927$
Snow load on roof, 50	$s_{i,50} = 0.297 \text{ kN/m}^2$
Adjustment factor for service life	$f_s = 1.000$
Snow load on roof, 25	$s_{i,25} = 0.276 \text{ kN/m}^2$



# Structural analysis report | Roof 1

## Dead Load

Weight of module	$G_M = 23.4 \text{ kg}$
Weight of mounting system per module	$= 2.5 \text{ kg}$
Module area	$A_M = 2.08 \text{ m}^2$
Dead weight of module per $\text{m}^2$	$= 11.24 \text{ kg/m}^2$
Dead weight of mounting system per $\text{m}^2$	$= 1.20 \text{ kg/m}^2$
Total Dead Load (excl. ballast) per $\text{m}^2$	$= 0.12 \text{ kN/m}^2$

## Load Combinations

### Ultimate limit state

Partial safety factor unfavourable permanent load	$V_{G,sup} = 1.35$
Partial safety factor favourable permanent load	$V_{G,inf} = 1.00$
Partial safety factor destabilising permanent load	$V_{G,dst} = 1.10$
Partial safety factor stabilising permanent load	$V_{G,stb} = 0.90$
Partial safety factor variable loads	$V_Q = 1.50$
Combination coefficient with regards to wind	$\psi_{0,W} = 0.60$
Combination coefficient with regards to wind (additional varying influences)	$\psi_{1,W} = 0.20$
Combination coefficient with regards to Snow	$\psi_{0,S} = 0.50$
Importance factor permanent	$K_{Fl,G} = 1.00$
Importance factor variable	$K_{Fl,Q} = 1.00$
Characteristic dead weight	$G_k$
Characteristic snow load on the roof	$S_{i,n}$
Characteristic wind load	$W_k$
Load case combination 01	$LCC\ 01_{uls} = V_{G,sup} * K_{Fl,G} * G_k + V_Q * K_{Fl,Q} * S_{i,n}$
Load case combination 02	$LCC\ 02_{uls} = V_{G,sup} * K_{Fl,G} * G_k + V_Q * K_{Fl,Q} * W_{k,Pressure}$
Load case combination 03	$LCC\ 03_{uls} = V_{G,sup} * K_{Fl,G} * G_k + V_Q * K_{Fl,Q} * (W_{k,Pressure} + \psi_{0,S} * S_{i,n})$
Load case combination 04	$LCC\ 04_{uls} = V_{G,sup} * K_{Fl,G} * G_k + V_Q * K_{Fl,Q} * (S_{i,n} + \psi_{0,W} * W_{k,Pressure})$
Load case combination 06	$LCC\ 06_{uls} = V_{G,inf} * G_k + V_Q * K_{Fl,Q} * W_{k,Suction}$

### Serviceability limit state

Combination coefficient with regards to wind	$\psi_{0,W} = 0.60$
Combination coefficient with regards to Snow	$\psi_{0,S} = 0.50$
Load case combination 01	$LCC\ 01_{sls} = G_k + S_{i,n}$
Load case combination 02	$LCC\ 02_{sls} = G_k + W_{k,Pressure}$
Load case combination 03	$LCC\ 03_{sls} = G_k + W_{k,Pressure} + \psi_{0,S} * S_{i,n}$

# Structural analysis report | Roof 1

Load case combination 04

$$LCC\ 04\_sls = G_k + S_{i,n} + \psi_{0,W} * W_{k,Pressure}$$

Load case combination 06

$$LCC\ 06\_sls = G_k + W_{k,Suction}$$

## Maximum load on modules (Mounting system dimensioning)

Array	A-TrA [m <sup>2</sup> ]	ultimate state [kN/m <sup>2</sup> ]				Serviceability [kN/m <sup>2</sup> ]			
		Pressure ⊥	Pressure	Uplift ⊥	Uplift	Pressure ⊥	Pressure	Uplift ⊥	Uplift
ridge	10.00	0.802	0.253	-1.006	0.046	0.546	0.173	-0.633	0.046
gableboard	10.00	0.802	0.253	-1.207	0.046	0.546	0.173	-0.767	0.046
corner region (eave)	10.00	0.915	0.253	-1.307	0.046	0.621	0.173	-0.834	0.046
ridge	10.00	0.802	0.253	-1.006	0.046	0.546	0.173	-0.633	0.046
eaves	10.00	0.830	0.253	-0.550	0.046	0.565	0.173	-0.329	0.046
ridge	10.00	0.802	0.253	-1.006	0.046	0.546	0.173	-0.633	0.046
eaves	10.00	0.830	0.253	-0.550	0.046	0.565	0.173	-0.329	0.046
ridge	10.00	0.802	0.253	-1.006	0.046	0.546	0.173	-0.633	0.046
gableboard	10.00	0.802	0.253	-1.207	0.046	0.546	0.173	-0.767	0.046
corner region (eave)	10.00	0.915	0.253	-1.307	0.046	0.621	0.173	-0.834	0.046
field area	10.00	0.802	0.253	-0.490	0.046	0.546	0.173	-0.289	0.046
corner region (eave)	10.00	0.915	0.253	-1.307	0.046	0.621	0.173	-0.834	0.046
eaves	10.00	0.830	0.253	-0.550	0.046	0.565	0.173	-0.329	0.046
field area	10.00	0.802	0.253	-0.490	0.046	0.546	0.173	-0.289	0.046
corner region (eave)	10.00	0.915	0.253	-1.307	0.046	0.621	0.173	-0.834	0.046
eaves	10.00	0.830	0.253	-0.550	0.046	0.565	0.173	-0.329	0.046

## Max. load on fastener

Array	A-TrA [m <sup>2</sup> ]	ultimate state [kN]				Serviceability [kN]			
		Pressure ⊥	Pressure	Uplift ⊥	Uplift	Pressure ⊥	Pressure	Uplift ⊥	Uplift
ridge	10.00	0.957	0.301	-1.200	0.055	0.652	0.206	-0.755	0.055
gableboard	10.00	0.957	0.301	-1.440	0.055	0.652	0.206	-0.915	0.055
corner region (eave)	10.00	0.819	0.226	-1.170	0.041	0.556	0.155	-0.746	0.041
ridge	10.00	0.957	0.301	-1.200	0.055	0.652	0.206	-0.755	0.055
eaves	10.00	0.743	0.226	-0.492	0.041	0.505	0.155	-0.294	0.041
ridge	10.00	0.957	0.301	-1.200	0.055	0.652	0.206	-0.755	0.055
eaves	10.00	0.743	0.226	-0.492	0.041	0.505	0.155	-0.294	0.041



# Structural analysis report | Roof 1

ridge	10.00	0.957	0.301	-1.200	0.055	0.652	0.206	-0.755	0.055
gableboard	10.00	0.957	0.301	-1.440	0.055	0.652	0.206	-0.915	0.055
corner region (eave)	10.00	0.819	0.226	-1.170	0.041	0.556	0.155	-0.746	0.041
field area	10.00	0.686	0.216	-0.419	0.039	0.467	0.148	-0.247	0.039
corner region (eave)	10.00	0.783	0.216	-1.118	0.039	0.531	0.148	-0.713	0.039
eaves	10.00	0.710	0.216	-0.471	0.039	0.483	0.148	-0.281	0.039
field area	10.00	0.686	0.216	-0.419	0.039	0.467	0.148	-0.247	0.039
corner region (eave)	10.00	0.783	0.216	-1.118	0.039	0.531	0.148	-0.713	0.039
eaves	10.00	0.710	0.216	-0.471	0.039	0.483	0.148	-0.281	0.039

## Resistance Values of Components

### Base Rails

Base Rails	A [cm <sup>2</sup> ]	I <sub>y</sub> [cm <sup>4</sup> ]	I <sub>z</sub> [cm <sup>4</sup> ]	W <sub>y</sub> [cm <sup>3</sup> ]	W <sub>z</sub> [cm <sup>3</sup> ]
K2 SolidRail Light 37	3.150	4.36	6.98	2.25	3.54

### Fastener

Fastener	R <sub>D,Uplift,Perpendicular</sub> [kN]	R <sub>D,Pressure,Perpendicular</sub> [kN]	R <sub>D,Pressure,Parallel</sub> [kN]
SolidHook Pan Tile Vario 1	2.21	1.05	1.93

### Utilisation result

No.	roof areas	ultimate limit state			Usab.	Distances		maximum values	
		Pr σ[%]	CL σ[%]	Fst F[%]	Pr f[%]	Fst [m]	BR [m]	CL L <sub>max</sub> [m]	Fst D <sub>max</sub> [m]
1	ridge	49.4	5.1	99.8	59.5	1.800	---	0.573	1.803
1	gableboard	59.0	2.6	99.8	72.1	1.800	---	0.542	1.803
1	corner region (eave)	35.9	6.6	84.5	27.6	1.350	---	0.563	1.598
2	ridge	49.4	4.5	99.8	59.5	1.800	---	0.573	1.803
2	eaves	26.6	4.3	77.3	19.0	1.350	---	0.626	1.747
3	ridge	49.4	6.6	99.8	59.5	1.800	---	0.573	1.803
3	eaves	26.6	6.3	77.3	19.0	1.350	---	0.626	1.747
4	ridge	49.4	7.5	99.8	59.5	1.800	---	0.573	1.803
4	gableboard	59.0	1.1	99.8	72.1	1.800	---	0.542	1.803
4	corner region (eave)	35.9	9.7	84.5	27.6	1.350	---	0.563	1.598



# Structural analysis report | Roof 1

5	field area	16.5	0.0	71.6	7.8	0.900	---	0.616	1.257
5	corner region (eave)	22.9	3.4	80.8	11.7	0.900	---	0.550	1.115
5	eaves	17.0	6.5	73.9	8.1	0.900	---	0.610	1.218
6	field area	16.5	0.0	71.6	7.8	0.900	---	0.616	1.257
6	corner region (eave)	22.9	1.7	80.8	11.7	0.900	---	0.550	1.115
6	eaves	17.0	9.1	73.9	8.1	0.900	---	0.610	1.218

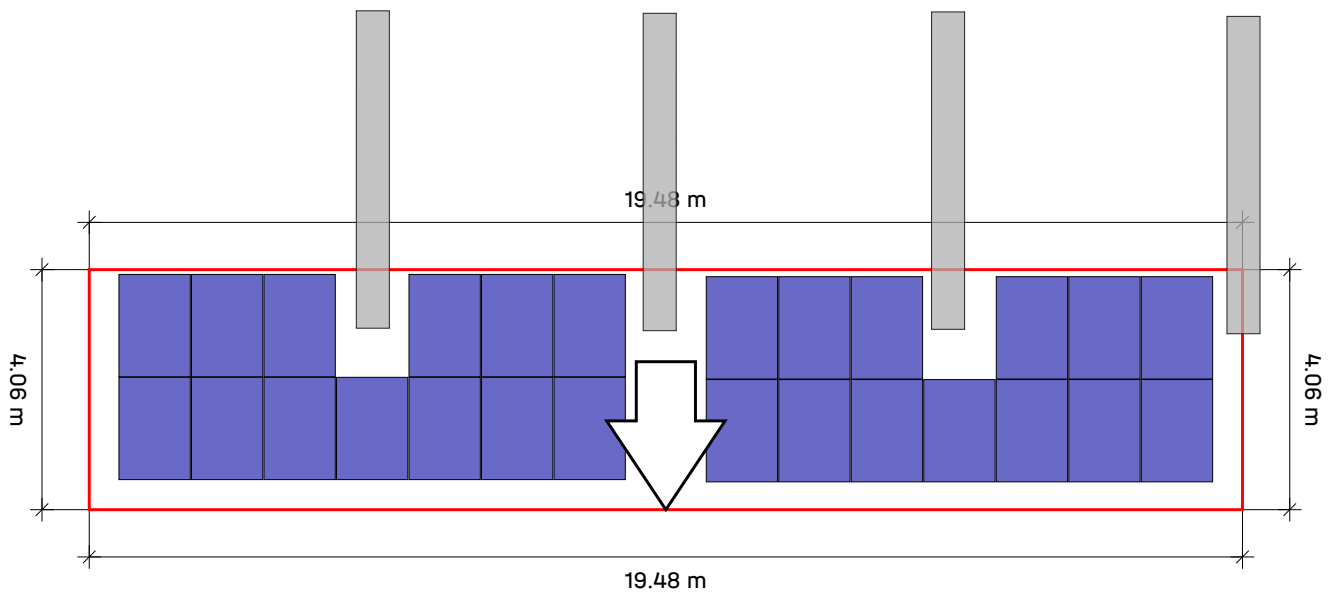
Pr	<b>Profile</b>	Fst D <sub>max</sub>	<b>maximum fastener spacing</b>
Fst	<b>Fastener</b>	BR	<b>Base Rail</b>
σ	<b>Stress</b>	Usab.	<b>serviceability limit state</b>
f	<b>Deflection</b>	CL	<b>Cantilever</b>
F	<b>Force</b>		
CL/L <sub>max</sub>	<b>maximum cantilever length</b>		



## Roofs | Roof 1 | Bill of material

Position	Item no.	Item description	Quantity	Weight
1	2004112	Wood screw 8×100	184	5.0 kg
2	2002589	OneEnd Black Set 30-42	56	4.9 kg
3	1000125	SolidHook Pan Tile Vario 1	92	65.5 kg
4	1000637	T-Bolt 28/15 M10×20	92	1.7 kg
5	1000042	Hexagon flange nut M10	92	1.0 kg
6	2003072	OneMid Black Set 30-42	24	1.9 kg
7	1004765	SolidRail Light End Cap	56	0.3 kg
8	2002870	K2 Solar Cable Manager	26	0.1 kg
9	2003232	SolidRail Light; 3.30 m	32	89.6 kg
10	1004107	SolidRail UltraLight+Light RailConnector Set	24	5.4 kg
<b>Total</b>				<b>175.3 kg</b>

# Roofs | Roof 2



Roof	System	Module	Height	Quantity	Total power
<a href="#">Roof 2</a>	<a href="#">SolidRail</a>	Alpha Pure-RX 1,728×1,205×30 mm 470 Wp	8.32 m	26	12.22 kWp
Tile					



# Roofs | Roof 2 | Assembly plan

## Base Rails

Type	Whole Rails		Rail cutting		
	Total Rail Length	Quantity 3.30 m	Part of Rail	Length	Rest
4*A	8.655	2*3.30 m	3.300	2.055 from 3.300	<u>1.235</u>
4*B	3.755	1*3.30 m	<u>1.235</u>	0.700 from 1.235	0.525
1*C	3.755	1*3.30 m	3.300	0.700 from 3.300	<u>2.590</u>
1*D	3.755	1*3.30 m	<u>2.590</u>	0.700 from 2.590	<u>1.880</u>
1*E	3.755	1*3.30 m	<u>1.880</u>	0.700 from 1.880	<u>1.170</u>
1*F	3.755	1*3.30 m	<u>1.170</u>	0.700 from 1.170	0.460

1 cm is viewed as lost for each cutting

Red numbers are leftover rails which will not be used any longer

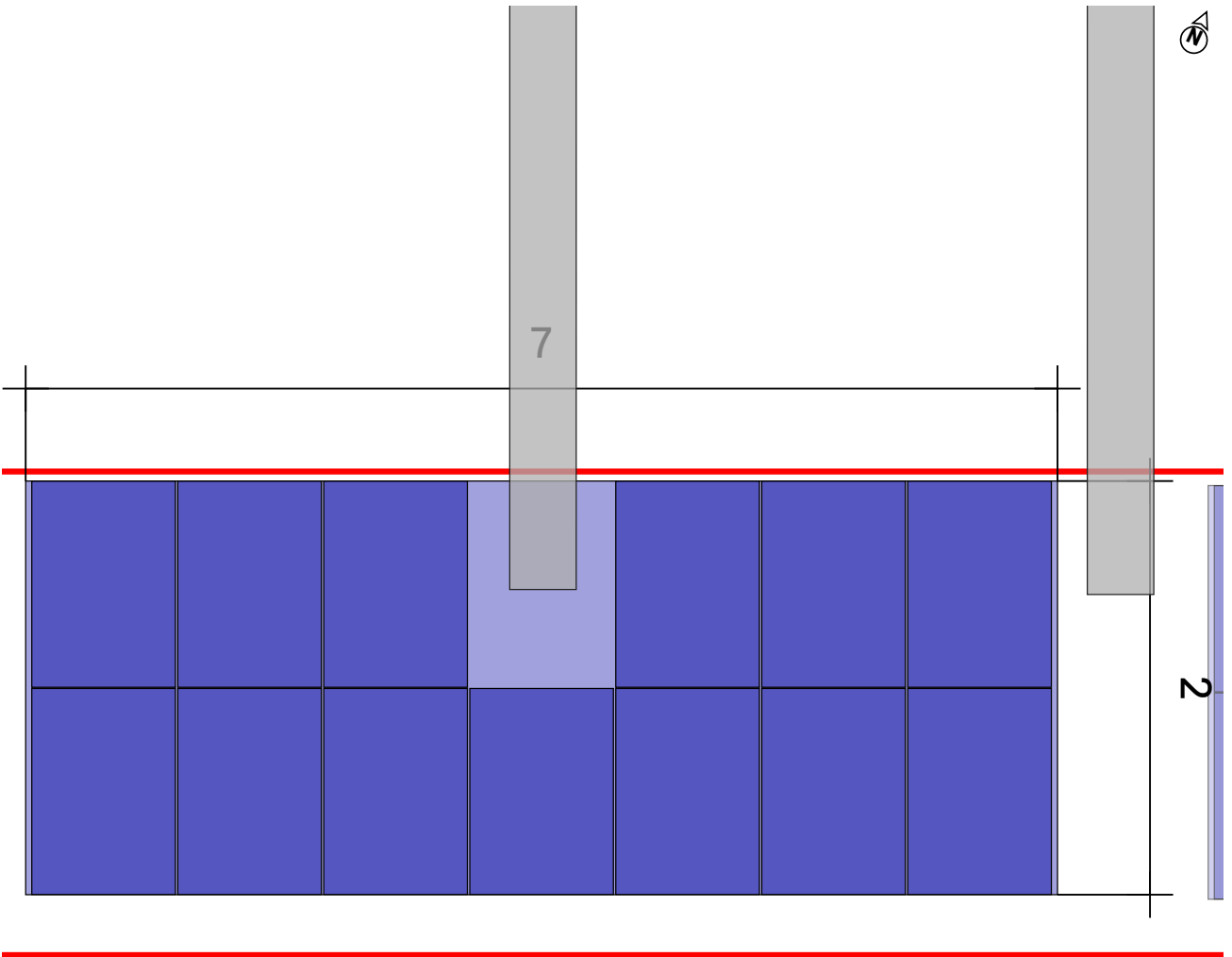
## Fastener Spacing

Module	Array	Distance	maximum cantilever length	maximum fastener spacing
1	ridge	0.90 m	0.598	1.113
1	gableboard	0.90 m	0.598	1.113
1	corner region (eave)	0.45 m	0.450	0.885
1	eaves	0.90 m	0.588	1.051
2	ridge	0.90 m	0.598	1.113
2	gableboard	0.90 m	0.598	1.113
2	corner region (eave)	0.45 m	0.450	0.885
2	eaves	0.90 m	0.588	1.051

## Module arrays

Module array	Width[m]	Length[m]	Width in modules	Length in modules
1	8.56	3.47	7	2
2	8.56	3.47	7	2

# Roofs | Roof 2 | Module array 1



Roof ② Module array ①

Mounting System

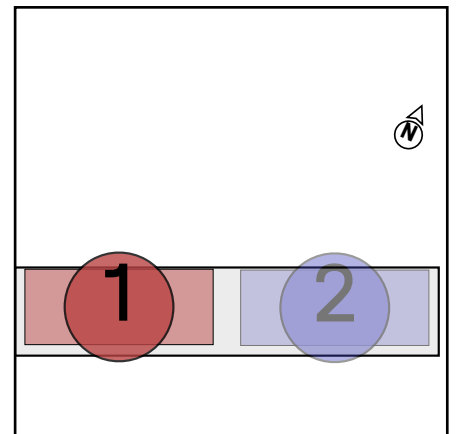
[SolidRail](#)

Module

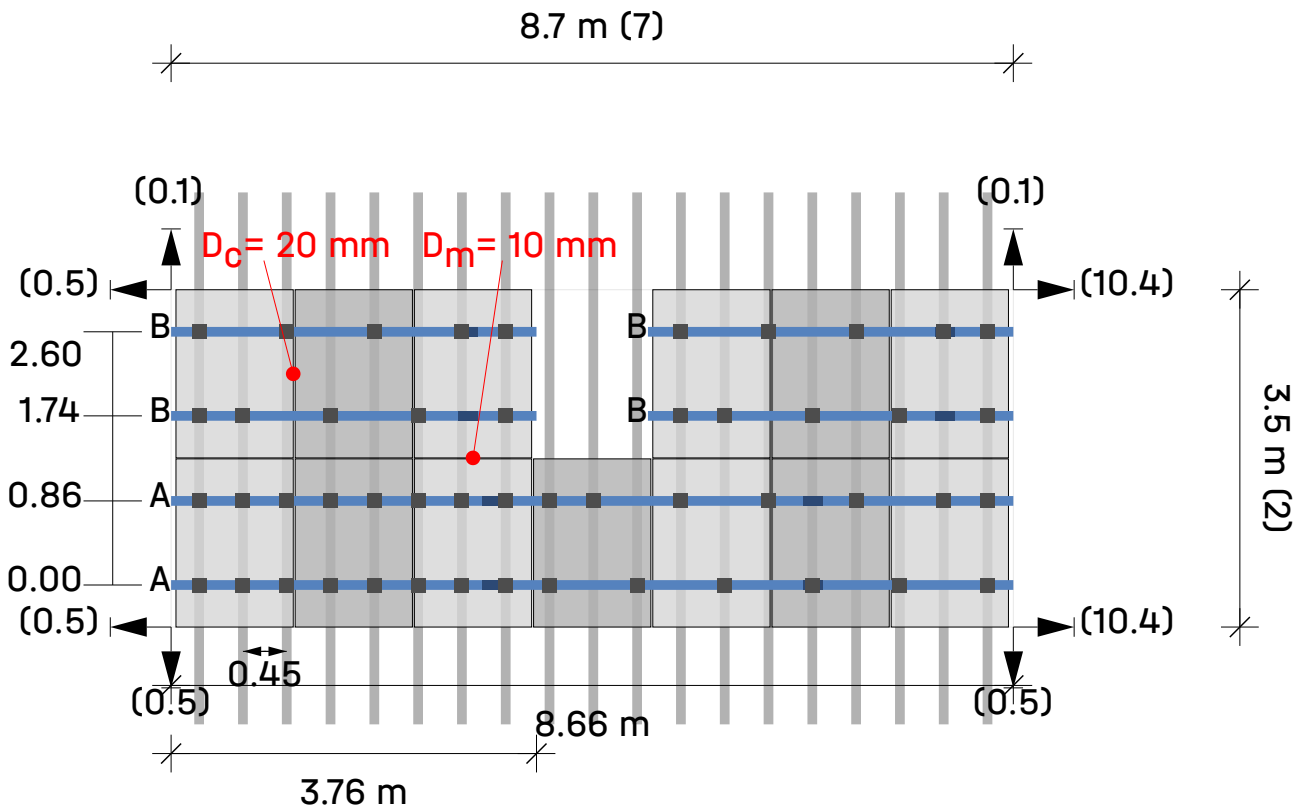
13(6.11 kWp) x Alpha Pure-RX

Row spacing

1.74 m



# Roofs | Roof 2 | Module array 1 | Module blocks

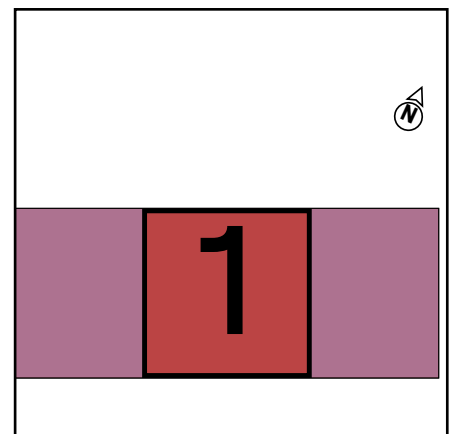


Roof ② Module array ① Module block 1

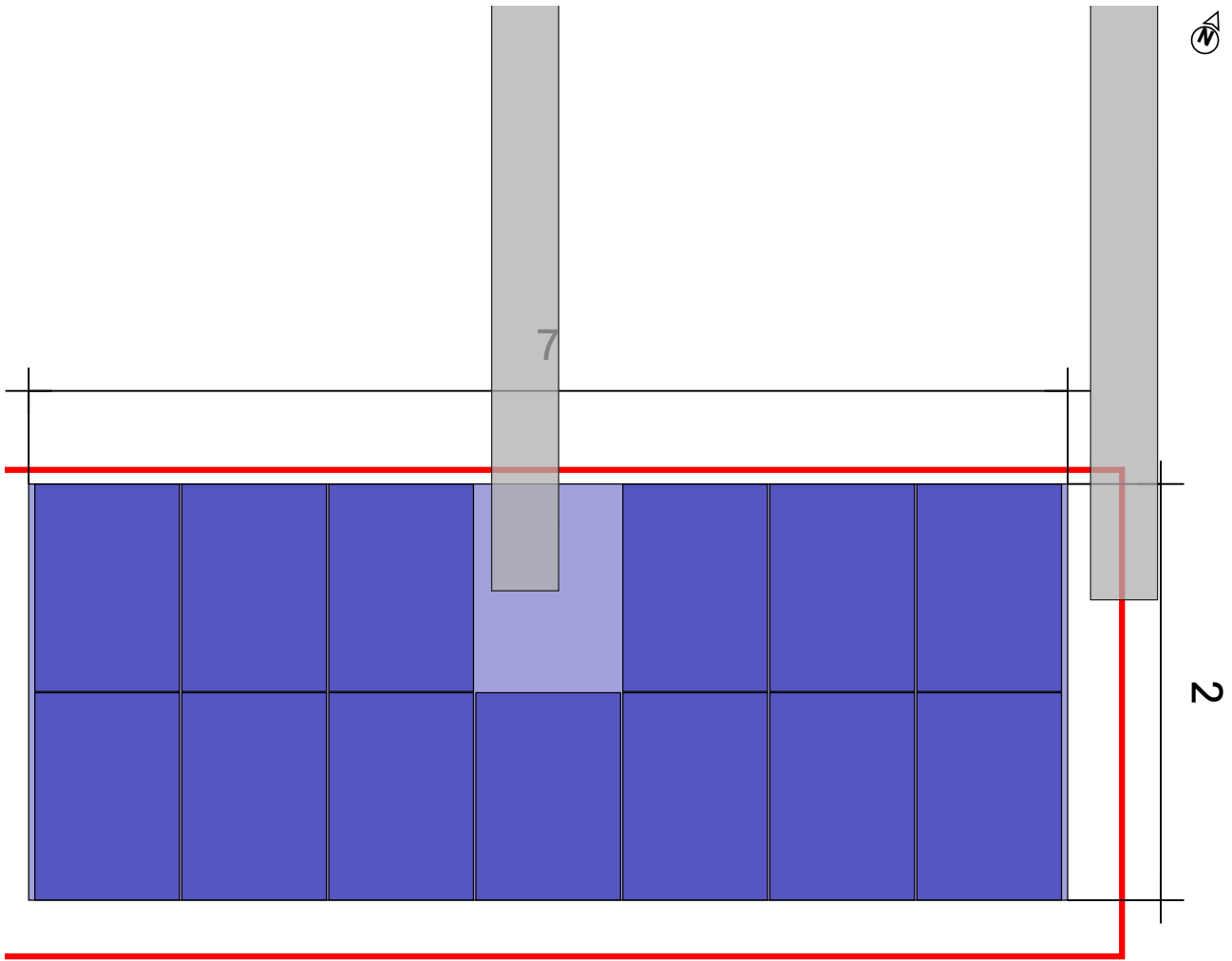
Modules (7 × 2) - 1 = 13

Legend

- Fastener
- Mounting rail: K2 SolidRail Light 37
- Distance to Roof Edge [m]
- $D_c$  Distance for clamping between modules
- $D_m$  Distance between modules



# Roofs | Roof 2 | Module array 2



Roof ② Module array ②

Mounting System

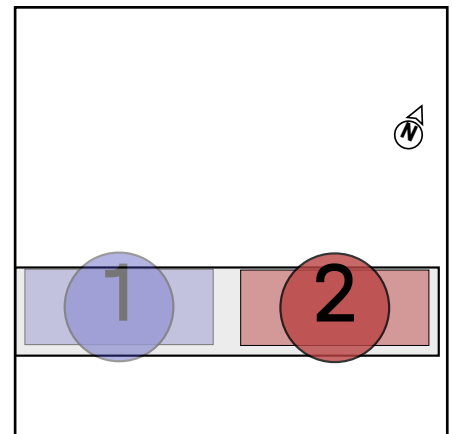
[SolidRail](#)

Module

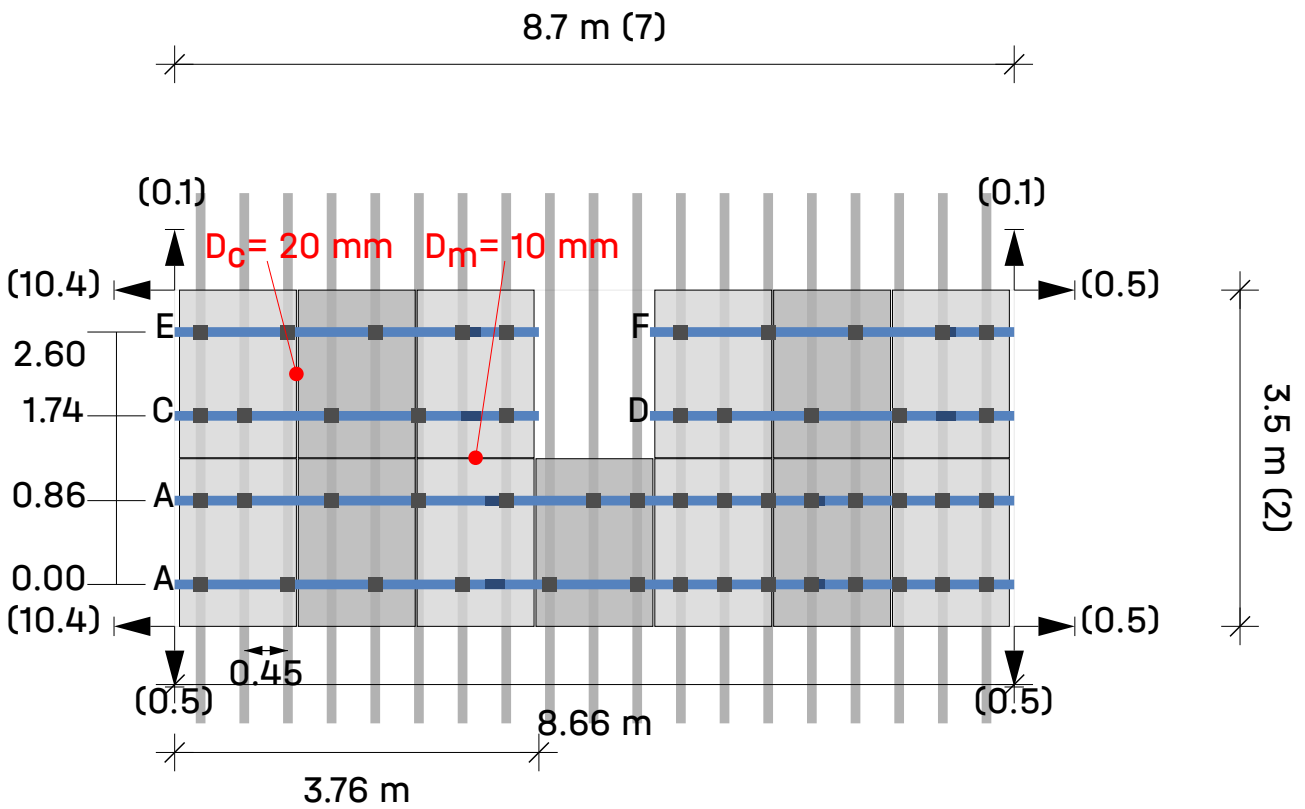
13(6.11 kWp) x Alpha Pure-RX

Row spacing

1.74 m



# Roofs | Roof 2 | Module array 2 | Module blocks

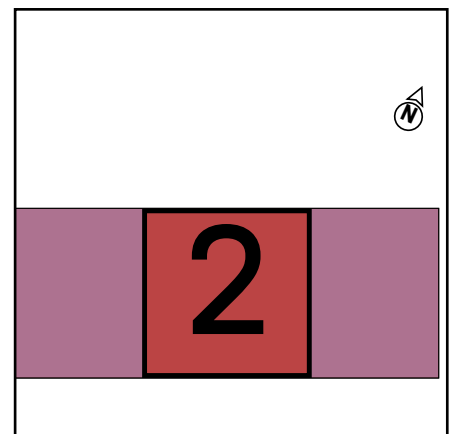


Roof 2    Module array 2    Module block 2


Modules  $(7 \times 2) - 1 = 13$

Legend

- Fastener
- Mounting rail: K2 SolidRail Light 37
- Distance to Roof Edge [m]
- D<sub>c</sub> Distance for clamping between modules
- D<sub>m</sub> Distance between modules



# Results | Roof 2

Roof	System	Module	Height	Quantity	Total power
<a href="#">Roof 2</a>	<a href="#">SolidRail</a>	Alpha Pure-RX 1,728×1,205×30 mm 470 Wp	8.32 m	26	12.22 kWp
	Tile				

## Module

Name	Alpha Pure-RX
Manufacturer	REC
Output power	470 Wp
Dimensions	1,728×1,205×30 mm
Weight	23.4 kg

## Components

Fastener	SolidHook Pan Tile Vario 1
Base rails	K2 SolidRail Light 37

## Loads on modules (module dimensioning)

Array	A-TrA [m <sup>2</sup> ]	ultimate state [Pa]				Serviceability [Pa]			
		Pressure ⊥	Pressure ∥	Uplift ⊥	Uplift ∥	Pressure ⊥	Pressure ∥	Uplift ⊥	Uplift ∥
ridge	2.08	880.2	372.0	-779.2	61.0	597.4	254.1	-484.2	61.0
gableboard	2.08	880.2	372.0	-975.8	61.0	597.4	254.1	-615.3	61.0
corner region (eave)	2.08	1,180.0	372.0	-1,074.1	61.0	797.2	254.1	-680.9	61.0
eaves	2.08	939.2	372.0	-484.2	61.0	636.7	254.1	-287.6	61.0
ridge	2.08	880.2	372.0	-779.2	61.0	597.4	254.1	-484.2	61.0
gableboard	2.08	880.2	372.0	-975.8	61.0	597.4	254.1	-615.3	61.0
corner region (eave)	2.08	1,180.0	372.0	-1,074.1	61.0	797.2	254.1	-680.9	61.0
eaves	2.08	939.2	372.0	-484.2	61.0	636.7	254.1	-287.6	61.0



# Results | Roof 2

## Utilisation result

No.	Module Array	roof areas	ultimate limit state			Usab.	Distances		maximum values	
			Pr σ[%]	CL σ[%]	Fst F[%]	Pr f[%]	Fst [m]	BR [m]	CL L <sub>max</sub> [m]	Fst D <sub>max</sub> [m]
1		ridge	19.1	13.6	80.9	8.7	0.900	---	0.598	1.113
1		gableboard	19.1	9.8	80.9	8.7	0.900	---	0.598	1.113
1		corner region (eave)	6.1	12.4	50.9	1.4	0.450	---	0.450	0.885
1		eaves	20.1	8.7	85.7	9.2	0.900	---	0.588	1.051
2		ridge	19.1	13.4	80.9	8.7	0.900	---	0.598	1.113
2		gableboard	19.1	9.6	80.9	8.7	0.900	---	0.598	1.113
2		corner region (eave)	6.1	12.2	50.9	1.4	0.450	---	0.450	0.885
2		eaves	20.1	8.9	85.7	9.2	0.900	---	0.588	1.051

Pr **Profile**  
 Fst **Fastener**  
 σ **Stress**  
 f **Deflection**  
 F **Force**  
 CL/L<sub>max</sub> **maximum cantilever length**

Fst D<sub>max</sub> **maximum fastener spacing**  
 BR **Base Rail**  
 Usab. **serviceability limit state**  
 CL **Cantilever**



## Results | Roof 2

### Notes

- The dimensioning of the timber construction screws is not part of this structural analysis. The dimensioning and positioning of the timber construction screws to be used must be carried out in accordance with the respective applicable codes of practice.
- The structure was statically verified in accordance with Eurocode 9: Design of aluminum structures (DIN EN 1999-1-1:2021) and offers sufficient load-bearing capacity and stability for the loads specified in the chapter 'Maximum actions on the components'.
- Adjustment factor for wind load regarding service life period,  $f_W$ , is according to DIN EN 1991-1-4/ NA, NDP for 4.2 (2P) note 5, table 3
- Adjustment factor for snow load regarding service life period,  $f_S$ , is according to DIN EN 1991-1-3/ annex D, table 4
- The structural design complies with BS EN 1990 - Basis of Structural Design.
- Snow loads are determined in accordance with National Annex BS NA EN 1991-1-3 (2018) - UK National Annex to EC1 - Action on structures - general actions - snow loads.
- Wind loads are determined in accordance with National Annex BS NA EN 1991-1-4 - UK National Annex to EC1 - Action on structures, general actions - wind actions.
- Service life is recognised according to 'Eurocode EN 1991 - Action on structures, Snow loads' and 'Eurocode EN 1991 - Actions on structures, Wind actions'. Subject to the Building Regulations and for security-relevant reasons the installation has to be dismantled at the end of its service life.
- Failure consequence class is considered according to 'Eurocode EN 1990 - Basis of structural design'.
- Data and results must be verified with regard to local conditions and checked by a suitably qualified person. Please see our TCU under <https://k2-systems.com/en/base-tcu>, in particular § 2 ("technical and specialist requirements for the customer"), § 7 ("warranty provisions") and § 8 ("limitation of liability").



# Structural analysis report | Roof 2

## General information

Name	Lincoln College - Museum Road - PV - Final
Mounting System	SolidRail
Author	Daniel Tempest

## Location information

Address	9 Museum Rd, Oxford OX1 3PX, UK
Ground elevation	65.14 m

## Roof information

Building height	8.32 m
Roof type	Gable roof
Roof pitch	30°
Roof covering	Tile
Min. roof edge distance	0.00 m
Rafter Spacing	0.450 m
Rafter width	50.0 mm
set edge rafter left	No
Rafter spacing left	290.0 mm
Rafter spacing right	No
Rafter Spacing	290.0 mm
Batten spacing	340.0 mm

## Loads

Design method	BS EN
Failure consequence class (CC)	CC2
Design working life	25 years
Terrain category	Town Terrain
Distance to shoreline	100.00 km
Distance inside town terrain	0.20 km

## Wind load

Velocity pressure, 50	$q_{p,50} = 0.712 \text{ kN/m}^2$
Adjustment factor for service life	$f_w = 1.000$
Velocity pressure, 25	$q_{p,25} = 0.655 \text{ kN/m}^2$

# Structural analysis report | Roof 2

## Roof areas

Array	load impact area [m <sup>2</sup> ]	maxCpe <sub>10</sub>	minCpe <sub>10</sub>	wind pressure [kN/m <sup>2</sup> ]	wind suction [kN/m <sup>2</sup> ]
ridge	10.00	0.400	-0.900	0.262	-0.590
gableboard	10.00	0.400	-1.100	0.262	-0.721
corner region (eave)	10.00	0.800	-1.200	0.524	-0.787
eaves	10.00	0.500	-0.600	0.328	-0.393
ridge	10.00	0.400	-0.900	0.262	-0.590
gableboard	10.00	0.400	-1.100	0.262	-0.721
corner region (eave)	10.00	0.800	-1.200	0.524	-0.787
eaves	10.00	0.500	-0.600	0.328	-0.393

## Snow load

Snow load zone	2
Snow guard	No
Snow load on ground level	$s_k = 0.400 \text{ kN/m}^2$
Shape Coefficient for Snow	$\mu_i = 1.200$
Factor for roof pitch	$d_i = 0.866$
Snow load on roof, 50	$s_{i,50} = 0.416 \text{ kN/m}^2$
Adjustment factor for service life	$f_s = 1.000$
Snow load on roof, 25	$s_{i,25} = 0.386 \text{ kN/m}^2$

## Dead Load

Weight of module	$G_M = 23.4 \text{ kg}$
Weight of mounting system per module	$= 2.5 \text{ kg}$
Module area	$A_M = 2.08 \text{ m}^2$
Dead weight of module per m <sup>2</sup>	$= 11.24 \text{ kg/m}^2$
Dead weight of mounting system per m <sup>2</sup>	$= 1.20 \text{ kg/m}^2$
Total Dead Load (excl. ballast) per m <sup>2</sup>	$= 0.12 \text{ kN/m}^2$



# Structural analysis report | Roof 2

## Load Combinations

### Ultimate limit state

Partial safety factor unfavourable permanent load	$\gamma_{G,sup} = 1.35$
Partial safety factor favourable permanent load	$\gamma_{G,inf} = 1.00$
Partial safety factor destabilising permanent load	$\gamma_{G,dst} = 1.10$
Partial safety factor stabilising permanent load	$\gamma_{G,stab} = 0.90$
Partial safety factor variable loads	$\gamma_Q = 1.50$
Combination coefficient with regards to wind	$\psi_{0,W} = 0.60$
Combination coefficient with regards to wind (additional varying influences)	$\psi_{1,W} = 0.20$
Combination coefficient with regards to Snow	$\psi_{0,S} = 0.50$
Importance factor permanent	$K_{Fl,G} = 1.00$
Importance factor variable	$K_{Fl,Q} = 1.00$
Characteristic dead weight	$G_k$
Characteristic snow load on the roof	$S_{i,n}$
Characteristic wind load	$W_k$

Load case combination 01	$LCC\ 01_{uls} = \gamma_{G,sup} * K_{Fl,G} * G_k + \gamma_Q * K_{Fl,Q} * S_{i,n}$
Load case combination 02	$LCC\ 02_{uls} = \gamma_{G,sup} * K_{Fl,G} * G_k + \gamma_Q * K_{Fl,Q} * W_{k,Pressure}$
Load case combination 03	$LCC\ 03_{uls} = \gamma_{G,sup} * K_{Fl,G} * G_k + \gamma_Q * K_{Fl,Q} * (W_{k,Pressure} + \psi_{0,S} * S_{i,n})$
Load case combination 04	$LCC\ 04_{uls} = \gamma_{G,sup} * K_{Fl,G} * G_k + \gamma_Q * K_{Fl,Q} * (S_{i,n} + \psi_{0,W} * W_{k,Pressure})$
Load case combination 06	$LCC\ 06_{uls} = \gamma_{G,inf} * G_k + \gamma_Q * K_{Fl,Q} * W_{k,Suction}$

### Serviceability limit state

Combination coefficient with regards to wind	$\psi_{0,W} = 0.60$
Combination coefficient with regards to Snow	$\psi_{0,S} = 0.50$

Load case combination 01	$LCC\ 01_{sls} = G_k + S_{i,n}$
Load case combination 02	$LCC\ 02_{sls} = G_k + W_{k,Pressure}$
Load case combination 03	$LCC\ 03_{sls} = G_k + W_{k,Pressure} + \psi_{0,S} * S_{i,n}$
Load case combination 04	$LCC\ 04_{sls} = G_k + S_{i,n} + \psi_{0,W} * W_{k,Pressure}$
Load case combination 06	$LCC\ 06_{sls} = G_k + W_{k,Suction}$

# Structural analysis report | Roof 2

## Maximum load on modules (Mounting system dimensioning)

Array	A-TrA [m <sup>2</sup> ]	ultimate state [kN/m <sup>2</sup> ]				Serviceability [kN/m <sup>2</sup> ]			
		Pressure ⊥	Pressure	Uplift ⊥	Uplift	Pressure ⊥	Pressure	Uplift ⊥	Uplift
ridge	10.00	0.880	0.372	-0.779	0.061	0.597	0.254	-0.484	0.061
gableboard	10.00	0.880	0.372	-0.976	0.061	0.597	0.254	-0.615	0.061
corner region (eave)	10.00	1.180	0.372	-1.074	0.061	0.797	0.254	-0.681	0.061
eaves	10.00	0.939	0.372	-0.484	0.061	0.637	0.254	-0.288	0.061
ridge	10.00	0.880	0.372	-0.779	0.061	0.597	0.254	-0.484	0.061
gableboard	10.00	0.880	0.372	-0.976	0.061	0.597	0.254	-0.615	0.061
corner region (eave)	10.00	1.180	0.372	-1.074	0.061	0.797	0.254	-0.681	0.061
eaves	10.00	0.939	0.372	-0.484	0.061	0.637	0.254	-0.288	0.061

## Max. load on fastener

Array	A-TrA [m <sup>2</sup> ]	ultimate state [kN]				Serviceability [kN]			
		Pressure ⊥	Pressure	Uplift ⊥	Uplift	Pressure ⊥	Pressure	Uplift ⊥	Uplift
ridge	10.00	0.753	0.318	-0.666	0.052	0.511	0.217	-0.414	0.052
gableboard	10.00	0.753	0.318	-0.835	0.052	0.511	0.217	-0.526	0.052
corner region (eave)	10.00	0.505	0.159	-0.459	0.026	0.341	0.109	-0.291	0.026
eaves	10.00	0.803	0.318	-0.414	0.052	0.545	0.217	-0.246	0.052
ridge	10.00	0.753	0.318	-0.666	0.052	0.511	0.217	-0.414	0.052
gableboard	10.00	0.753	0.318	-0.835	0.052	0.511	0.217	-0.526	0.052
corner region (eave)	10.00	0.505	0.159	-0.459	0.026	0.341	0.109	-0.291	0.026
eaves	10.00	0.803	0.318	-0.414	0.052	0.545	0.217	-0.246	0.052

## Resistance Values of Components

### Base Rails

Base Rails	A [cm <sup>2</sup> ]	I <sub>y</sub> [cm <sup>4</sup> ]	I <sub>z</sub> [cm <sup>4</sup> ]	W <sub>y</sub> [cm <sup>3</sup> ]	W <sub>z</sub> [cm <sup>3</sup> ]
K2 SolidRail Light 37	3.150	4.36	6.98	2.25	3.54

### Fastener



# Structural analysis report | Roof 2

Fastener	$R_{D,Uplift,Perpendicular}$ [kN]	$R_{D,Pressure,Perpendicular}$ [kN]	$R_{D,Pressure,Parallel}$ [kN]
SolidHook Pan Tile Vario 1	2.21	1.05	1.93

## Utilisation result

No.	Module Array	roof areas	ultimate limit state			Usab.	Distances		maximum values	
			Pr $\sigma$ [%]	CL $\sigma$ [%]	Fst F[%]	Pr f[%]	Fst [m]	BR [m]	CL $L_{max}$ [m]	Fst Fst $D_{max}$ [m]
1		ridge	19.1	13.6	80.9	8.7	0.900	---	0.598	1.113
1		gableboard	19.1	9.8	80.9	8.7	0.900	---	0.598	1.113
1		corner region (eave)	6.1	12.4	50.9	1.4	0.450	---	0.450	0.885
1		eaves	20.1	8.7	85.7	9.2	0.900	---	0.588	1.051
2		ridge	19.1	13.4	80.9	8.7	0.900	---	0.598	1.113
2		gableboard	19.1	9.6	80.9	8.7	0.900	---	0.598	1.113
2		corner region (eave)	6.1	12.2	50.9	1.4	0.450	---	0.450	0.885
2		eaves	20.1	8.9	85.7	9.2	0.900	---	0.588	1.051

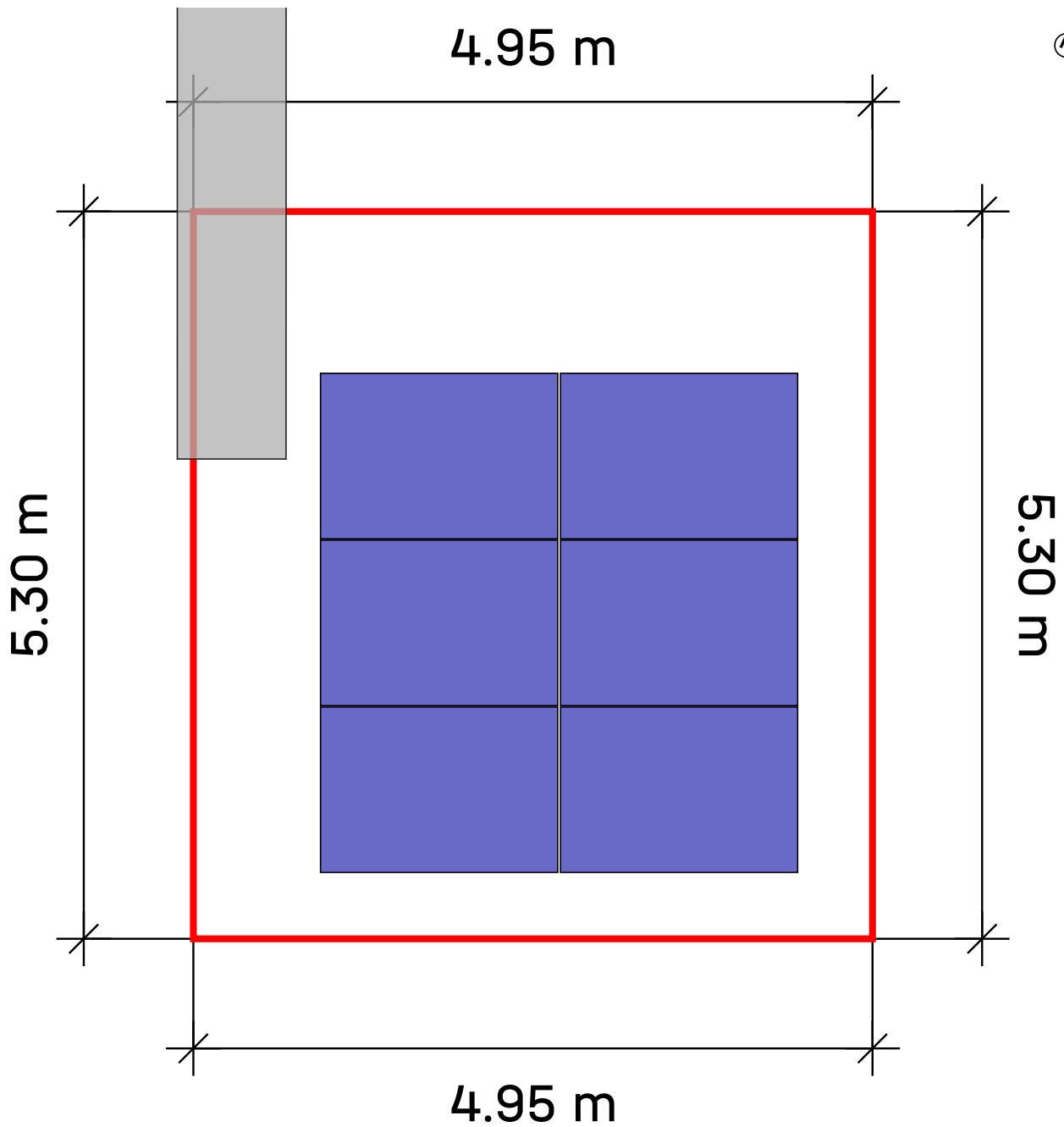
Pr	Profile	Fst $D_{max}$	maximum fastener spacing
Fst	Fastener	BR	Base Rail
$\sigma$	Stress	Usab.	serviceability limit state
f	Deflection	CL	Cantilever
F	Force		
$CL/L_{max}$	maximum cantilever length		



## Roofs | Roof 2 | Bill of material

Position	Item no.	Item description	Quantity	Weight
1	2004112	Wood screw 8×100	196	5.3 kg
2	2002589	OneEnd Black Set 30-42	24	2.1 kg
3	1000125	SolidHook Pan Tile Vario 1	98	69.8 kg
4	1000637	T-Bolt 28/15 M10×20	98	1.8 kg
5	1000042	Hexagon flange nut M10	98	1.1 kg
6	2003072	OneMid Black Set 30-42	40	3.2 kg
7	1004765	SolidRail Light End Cap	24	0.1 kg
8	2002870	K2 Solar Cable Manager	26	0.1 kg
9	2003232	SolidRail Light; 3.30 m	21	58.8 kg
10	1004107	SolidRail UltraLight+Light RailConnector Set	16	3.6 kg
<b>Total</b>				<b>145.8 kg</b>

## Roofs | Roof 3



Roof	System	Module	Height	Quantity	Total power
<a href="#">Roof 3</a>	<a href="#">SolidRail</a>	Alpha Pure-RX 1,728×1,205×30 mm 470 Wp	9.00 m	6	2.82 kWp
Tile					

# Roofs | Roof 3 | Assembly plan

## Base Rails

Type	Whole Rails		Rail cutting		
	Total Rail Length	Quantity 3.30 m	Part of Rail	Length	Rest
2*A	3.576	1*3.30 m	3.300	0.700 from 3.300	<u>2.590</u>
2*B	3.576	1*3.30 m	<u>2.590</u>	0.700 from 2.590	<u>1.880</u>
1*C	3.576	1*3.30 m	<u>1.880</u>	0.700 from 1.880	<u>1.170</u>
1*D	3.576	1*3.30 m	<u>1.170</u>	0.700 from 1.170	0.460

1 cm is viewed as lost for each cutting

Red numbers are leftover rails which will not be used any longer

## Fastener Spacing

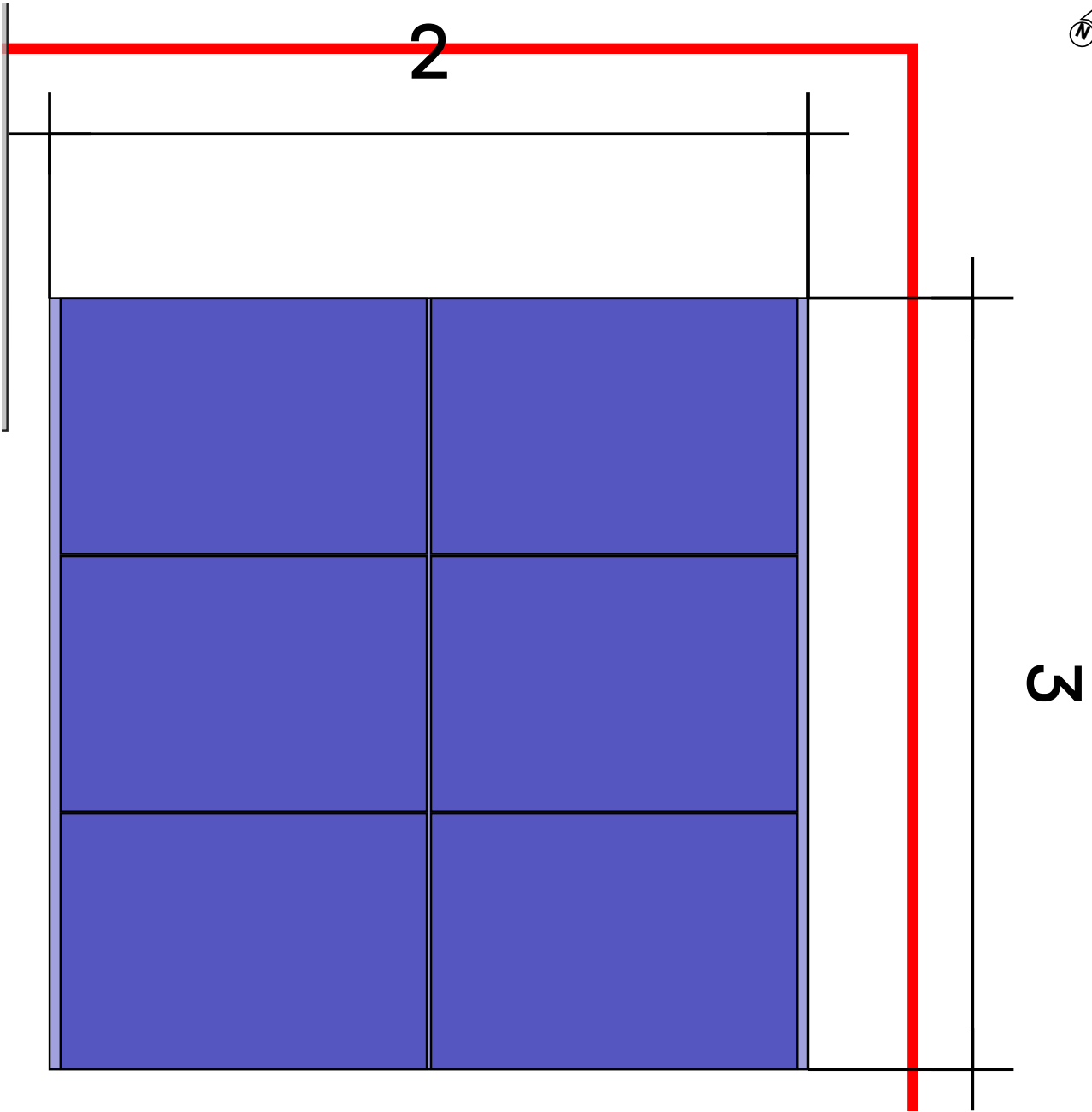
Module	Array	Distance	maximum cantilever length	maximum fastener spacing
1	field area	1.80 m	0.594	1.803
1	gableboard	1.80 m	0.542	1.803
1	corner region (eave)	1.35 m	0.563	1.598
1	eaves	1.35 m	0.626	1.747

## Module arrays

Module array	Width[m]	Length[m]	Width in modules	Length in modules
1	3.48	3.64	2	3



# Roofs | Roof 3 | Module array 1



Roof **③** Module array **①**

Mounting System

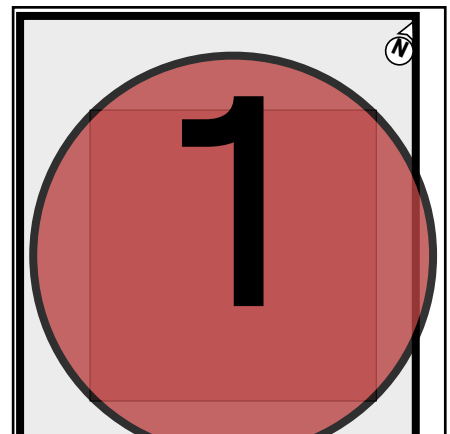
[SolidRail](#)

Module

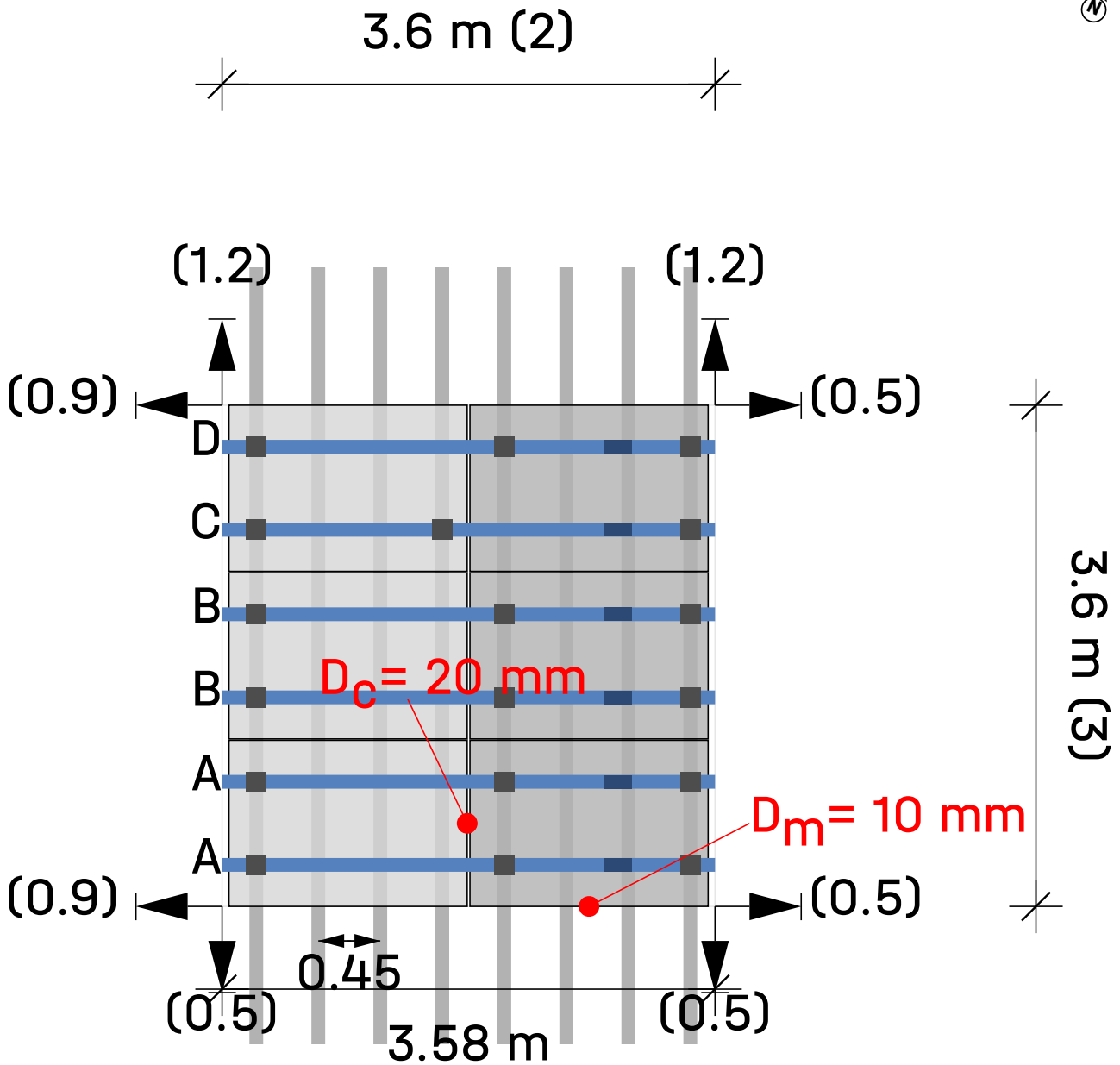
6(2.82 kWp) x Alpha Pure-RX

Row spacing

1.22 m



Roofs | Roof 3 | Module array 1 | Module blocks



Roof ③ Module array ① Module block ①


Modules  $2 \times 3 = 6$

Legend

- Fastener
- Mounting rail: K2 SolidRail Light 37
- Distance to Roof Edge [m]
- $D_c$  Distance for clamping between modules
- $D_m$  Distance between modules



# Results | Roof 3

Roof	System	Module	Height	Quantity	Total power
Roof 3	SolidRail	Alpha Pure-RX 1,728×1,205×30 mm 470 Wp	9.00 m	6	2.82 kWp
	Tile				

## Module

Name	Alpha Pure-RX
Manufacturer	REC
Output power	470 Wp
Dimensions	1,728×1,205×30 mm
Weight	23.4 kg

## Components

Fastener	SolidHook Pan Tile Vario 1
Base rails	K2 SolidRail Light 37

## Loads on modules (module dimensioning)

Array	A-TrA [m <sup>2</sup> ]	ultimate state [Pa]				Serviceability [Pa]			
		Pressure ⊥	Pressure	Uplift ⊥	Uplift	Pressure ⊥	Pressure	Uplift ⊥	Uplift
field area	2.08	802.3	252.7	-489.9	45.7	546.2	173.0	-288.9	45.7
gableboard	2.08	802.3	252.7	-1,206.7	45.7	546.2	173.0	-766.8	45.7
corner region (eave)	2.08	914.9	252.7	-1,307.2	45.7	621.2	173.0	-833.8	45.7
eaves	2.08	830.5	252.7	-550.2	45.7	565.0	173.0	-329.1	45.7

## Utilisation result

No.	Module Array	roof areas	ultimate limit state			Usab.	Distances		maximum values	
			Pr σ[%]	CL σ[%]	Fst F[%]	Pr f[%]	Fst [m]	BR [m]	CL L <sub>max</sub> [m]	Fst D <sub>max</sub> [m]
1		field area	46.0	0.0	99.8	52.3	1.800	---	0.594	1.803
1		gableboard	59.0	5.6	99.8	72.1	1.800	---	0.542	1.803
1		corner region (eave)	35.9	6.1	84.5	27.6	1.350	---	0.563	1.598
1		eaves	26.6	0.0	77.3	19.0	1.350	---	0.626	1.747

Pr	Profile	Fst D <sub>max</sub>	maximum fastener spacing
Fst	Fastener	BR	Base Rail
σ	Stress	Usab.	serviceability limit state



## Results | Roof 3

f	Deflection	CL	Cantilever
F	Force		
CL/L <sub>max</sub>	maximum cantilever length		



## Results | Roof 3

### Notes

- The dimensioning of the timber construction screws is not part of this structural analysis. The dimensioning and positioning of the timber construction screws to be used must be carried out in accordance with the respective applicable codes of practice.
- The structure was statically verified in accordance with Eurocode 9: Design of aluminum structures (DIN EN 1999-1-1:2021) and offers sufficient load-bearing capacity and stability for the loads specified in the chapter 'Maximum actions on the components'.
- Adjustment factor for wind load regarding service life period,  $f_W$ , is according to DIN EN 1991-1-4/ NA, NDP for 4.2 (2P) note 5, table 3
- Adjustment factor for snow load regarding service life period,  $f_S$ , is according to DIN EN 1991-1-3/ annex D, table 4
- The structural design complies with BS EN 1990 - Basis of Structural Design.
- Snow loads are determined in accordance with National Annex BS NA EN 1991-1-3 (2018) - UK National Annex to EC1 - Action on structures - general actions - snow loads.
- Wind loads are determined in accordance with National Annex BS NA EN 1991-1-4 - UK National Annex to EC1 - Action on structures, general actions - wind actions.
- Service life is recognised according to 'Eurocode EN 1991 - Action on structures, Snow loads' and 'Eurocode EN 1991 - Actions on structures, Wind actions'. Subject to the Building Regulations and for security-relevant reasons the installation has to be dismantled at the end of its service life.
- Failure consequence class is considered according to 'Eurocode EN 1990 - Basis of structural design'.
- Data and results must be verified with regard to local conditions and checked by a suitably qualified person. Please see our TCU under <https://k2-systems.com/en/base-tcu>, in particular § 2 ("technical and specialist requirements for the customer"), § 7 ("warranty provisions") and § 8 ("limitation of liability").
- The clamping range of the module may not comply with for this installation variant, as the rail spacing depends on the laying of the tiles and thus the batten length. Please ensure that the clamping range specified by the module manufacturer is observed.

# Structural analysis report | Roof 3

## General information

Name	Lincoln College - Museum Road - PV - Final
Mounting System	SolidRail
Author	Daniel Tempest

## Location information

Address	9 Museum Rd, Oxford OX1 3PX, UK
Ground elevation	65.14 m

## Roof information

Building height	9.00 m
Roof type	Gable roof
Roof pitch	22°
Roof covering	Tile
Min. roof edge distance	0.00 m
Rafter Spacing	0.450 m
Rafter width	50.0 mm
set edge rafter left	No
Rafter spacing left	225.0 mm
Rafter spacing right	No
Rafter Spacing	225.0 mm
Batten spacing	340.0 mm

## Loads

Design method	BS EN
Failure consequence class (CC)	CC2
Design working life	25 years
Terrain category	Town Terrain
Distance to shoreline	100.00 km
Distance inside town terrain	0.20 km

## Wind load

Velocity pressure, 50	$q_{p,50} = 0.727 \text{ kN/m}^2$
Adjustment factor for service life	$f_w = 1.000$
Velocity pressure, 25	$q_{p,25} = 0.670 \text{ kN/m}^2$

## Structural analysis report | Roof 3

### Roof areas

Array	load impact area [m <sup>2</sup> ]	maxCpe <sub>10</sub>	minCpe <sub>10</sub>	wind pressure [kN/m <sup>2</sup> ]	wind suction [kN/m <sup>2</sup> ]
field area	10.00	0.293	-0.600	0.197	-0.402
gableboard	10.00	0.293	-1.313	0.197	-0.880
corner region (eave)	10.00	0.480	-1.413	0.322	-0.947
eaves	10.00	0.340	-0.660	0.228	-0.442

### Snow load

Snow load zone	2
Snow guard	No
Snow load on ground level	$s_k = 0.400 \text{ kN/m}^2$
Shape Coefficient for Snow	$\mu_i = 0.800$
Factor for roof pitch	$d_i = 0.927$
Snow load on roof, 50	$s_{i,50} = 0.297 \text{ kN/m}^2$
Adjustment factor for service life	$f_s = 1.000$
Snow load on roof, 25	$s_{i,25} = 0.276 \text{ kN/m}^2$

### Dead Load

Weight of module	$G_M = 23.4 \text{ kg}$
Weight of mounting system per module	$= 2.5 \text{ kg}$
Module area	$A_M = 2.08 \text{ m}^2$
Dead weight of module per m <sup>2</sup>	$= 11.24 \text{ kg/m}^2$
Dead weight of mounting system per m <sup>2</sup>	$= 1.20 \text{ kg/m}^2$
Total Dead Load (excl. ballast) per m <sup>2</sup>	$= 0.12 \text{ kN/m}^2$



# Structural analysis report | Roof 3

## Load Combinations

### Ultimate limit state

Partial safety factor unfavourable permanent load	$\gamma_{G,sup} = 1.35$
Partial safety factor favourable permanent load	$\gamma_{G,inf} = 1.00$
Partial safety factor destabilising permanent load	$\gamma_{G,dst} = 1.10$
Partial safety factor stabilising permanent load	$\gamma_{G,stab} = 0.90$
Partial safety factor variable loads	$\gamma_Q = 1.50$
Combination coefficient with regards to wind	$\psi_{0,W} = 0.60$
Combination coefficient with regards to wind (additional varying influences)	$\psi_{1,W} = 0.20$
Combination coefficient with regards to Snow	$\psi_{0,S} = 0.50$
Importance factor permanent	$K_{Fl,G} = 1.00$
Importance factor variable	$K_{Fl,Q} = 1.00$
Characteristic dead weight	$G_k$
Characteristic snow load on the roof	$S_{i,n}$
Characteristic wind load	$W_k$

Load case combination 01	$LCC\ 01_{uls} = \gamma_{G,sup} * K_{Fl,G} * G_k + \gamma_Q * K_{Fl,Q} * S_{i,n}$
Load case combination 02	$LCC\ 02_{uls} = \gamma_{G,sup} * K_{Fl,G} * G_k + \gamma_Q * K_{Fl,Q} * W_{k,Pressure}$
Load case combination 03	$LCC\ 03_{uls} = \gamma_{G,sup} * K_{Fl,G} * G_k + \gamma_Q * K_{Fl,Q} * (W_{k,Pressure} + \psi_{0,S} * S_{i,n})$
Load case combination 04	$LCC\ 04_{uls} = \gamma_{G,sup} * K_{Fl,G} * G_k + \gamma_Q * K_{Fl,Q} * (S_{i,n} + \psi_{0,W} * W_{k,Pressure})$
Load case combination 06	$LCC\ 06_{uls} = \gamma_{G,inf} * G_k + \gamma_Q * K_{Fl,Q} * W_{k,Suction}$

### Serviceability limit state

Combination coefficient with regards to wind	$\psi_{0,W} = 0.60$
Combination coefficient with regards to Snow	$\psi_{0,S} = 0.50$

Load case combination 01	$LCC\ 01_{sls} = G_k + S_{i,n}$
Load case combination 02	$LCC\ 02_{sls} = G_k + W_{k,Pressure}$
Load case combination 03	$LCC\ 03_{sls} = G_k + W_{k,Pressure} + \psi_{0,S} * S_{i,n}$
Load case combination 04	$LCC\ 04_{sls} = G_k + S_{i,n} + \psi_{0,W} * W_{k,Pressure}$
Load case combination 06	$LCC\ 06_{sls} = G_k + W_{k,Suction}$



# Structural analysis report | Roof 3

## Maximum load on modules (Mounting system dimensioning)

Array	A-TrA [m <sup>2</sup> ]	ultimate state [kN/m <sup>2</sup> ]				Serviceability [kN/m <sup>2</sup> ]			
		Pressure ⊥	Pressure	Uplift ⊥	Uplift	Pressure ⊥	Pressure	Uplift ⊥	Uplift
field area	10.00	0.802	0.253	-0.490	0.046	0.546	0.173	-0.289	0.046
gableboard	10.00	0.802	0.253	-1.207	0.046	0.546	0.173	-0.767	0.046
corner region (eave)	10.00	0.915	0.253	-1.307	0.046	0.621	0.173	-0.834	0.046
eaves	10.00	0.830	0.253	-0.550	0.046	0.565	0.173	-0.329	0.046

## Max. load on fastener

Array	A-TrA [m <sup>2</sup> ]	ultimate state [kN]				Serviceability [kN]			
		Pressure ⊥	Pressure	Uplift ⊥	Uplift	Pressure ⊥	Pressure	Uplift ⊥	Uplift
field area	10.00	0.957	0.301	-0.584	0.055	0.652	0.206	-0.345	0.055
gableboard	10.00	0.957	0.301	-1.440	0.055	0.652	0.206	-0.915	0.055
corner region (eave)	10.00	0.819	0.226	-1.170	0.041	0.556	0.155	-0.746	0.041
eaves	10.00	0.743	0.226	-0.492	0.041	0.505	0.155	-0.294	0.041

## Resistance Values of Components

### Base Rails

Base Rails	A [cm <sup>2</sup> ]	I <sub>y</sub> [cm <sup>4</sup> ]	I <sub>z</sub> [cm <sup>4</sup> ]	W <sub>y</sub> [cm <sup>3</sup> ]	W <sub>z</sub> [cm <sup>3</sup> ]
K2 SolidRail Light 37	3.150	4.36	6.98	2.25	3.54

### Fastener

Fastener	R <sub>D,Uplift,Perpendicular</sub> [kN]	R <sub>D,Pressure,Perpendicular</sub> [kN]	R <sub>D,Pressure,Parallel</sub> [kN]
SolidHook Pan Tile Vario 1	2.21	1.05	1.93



# Structural analysis report | Roof 3

## Utilisation result

No.	Module Array	roof areas	ultimate limit state			Usab.	Distances		maximum values	
			Pr $\sigma$ [%]	CL $\sigma$ [%]	Fst F[%]	Pr f[%]	Fst [m]	BR [m]	CL $L_{max}$ [m]	Fst Fst $D_{max}$ [m]
1		field area	46.0	0.0	99.8	52.3	1.800	---	0.594	1.803
1		gableboard	59.0	5.6	99.8	72.1	1.800	---	0.542	1.803
1		corner region (eave)	35.9	6.1	84.5	27.6	1.350	---	0.563	1.598
1		eaves	26.6	0.0	77.3	19.0	1.350	---	0.626	1.747

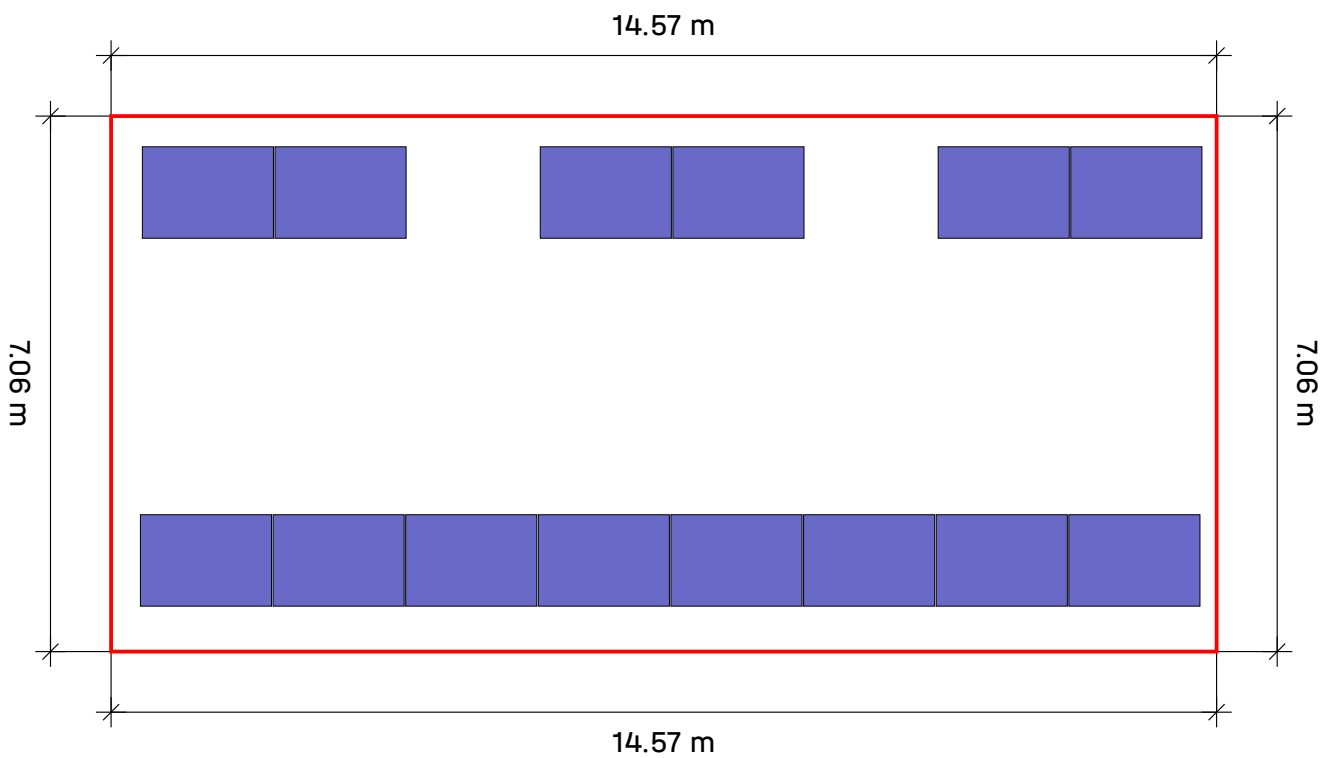
Pr **Profile** Fst  $D_{max}$  **maximum fastener spacing**  
 Fst **Fastener** BR **Base Rail**  
 $\sigma$  **Stress** Usab. **serviceability limit state**  
 f **Deflection** CL **Cantilever**  
 F **Force**  
 CL/ $L_{max}$  **maximum cantilever length**



## Roofs | Roof 3 | Bill of material

Position	Item no.	Item description	Quantity	Weight
1	1000125	SolidHook Pan Tile Vario 1	18	12.8 kg
2	1000637	T-Bolt 28/15 M10×20	18	0.3 kg
3	1000042	Hexagon flange nut M10	18	0.2 kg
4	2004112	Wood screw 8×100	36	1.0 kg
5	2002589	OneEnd Black Set 30-42	12	1.0 kg
6	2003072	OneMid Black Set 30-42	6	0.5 kg
7	1004765	SolidRail Light End Cap	12	0.1 kg
8	2002870	K2 Solar Cable Manager	6	0.0 kg
9	2003232	SolidRail Light; 3.30 m	8	22.4 kg
10	1004107	SolidRail UltraLight+Light RailConnector Set	6	1.4 kg
<b>Total</b>				<b>39.7 kg</b>

# Roofs | Roof 4



Roof	System	Module	Height	Quantity	Total power
<a href="#">Roof 4</a>	<a href="#">SolidRail</a>	Alpha Pure-RX 1,728×1,205×30 mm 470 Wp	8.50 m	14	6.58 kWp
Tile					

# Roofs | Roof 4 | Assembly plan

## Base Rails

Type	Whole Rails		Rail cutting		
	Total Rail Length	Quantity 3.30 m	Part of Rail	Length	Rest
1*A	14.064	4*3.30 m	3.300	0.864 from 3.300	<u>2.426</u>
1*B	14.064	4*3.30 m	<u>2.426</u>	0.864 from 2.426	<u>1.552</u>
1*C	3.576	1*3.30 m	<u>1.552</u>	0.700 from 1.552	<u>0.842</u>
1*D	3.576	1*3.30 m	<u>0.842</u>	0.700 from 0.842	0.132
1*E	3.576	1*3.30 m	3.300	0.700 from 3.300	<u>2.590</u>
1*F	3.576	1*3.30 m	<u>2.590</u>	0.700 from 2.590	<u>1.880</u>
1*G	3.576	1*3.30 m	<u>1.880</u>	0.700 from 1.880	<u>1.170</u>
1*H	3.576	1*3.30 m	<u>1.170</u>	0.700 from 1.170	0.460

1 cm is viewed as lost for each cutting

Red numbers are leftover rails which will not be used any longer

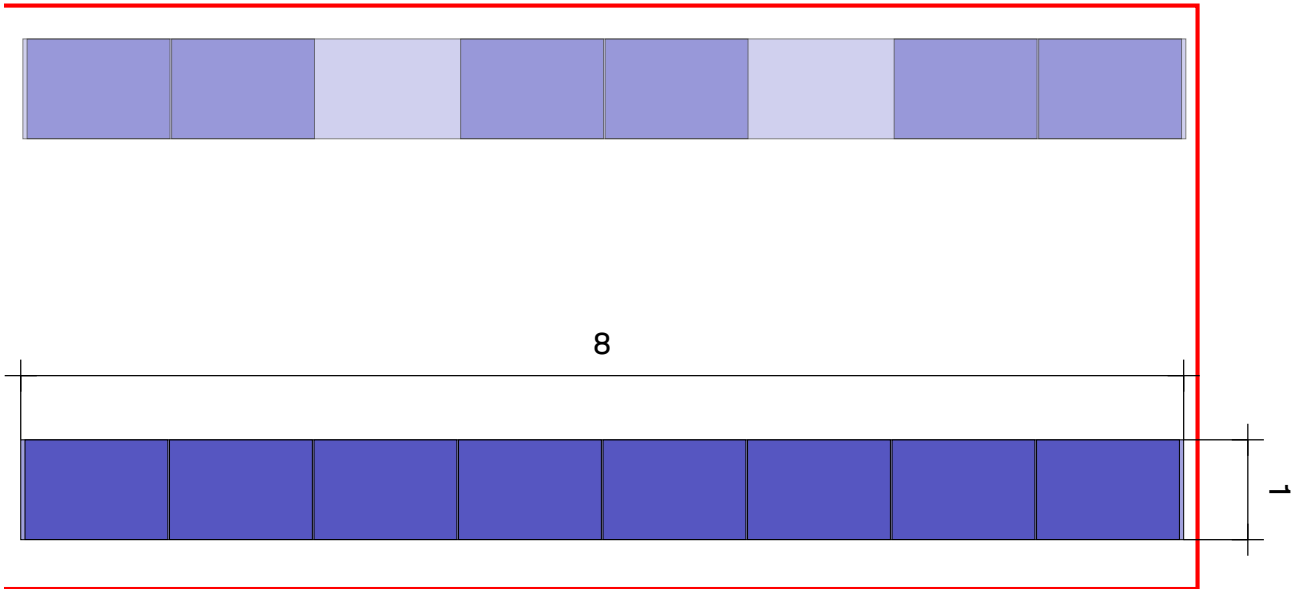
## Fastener Spacing

Module	Array	Distance	maximum cantilever length	maximum fastener spacing
1	field area	1.80 m	0.595	1.809
1	corner region (eave)	1.35 m	0.566	1.606
1	eaves	1.35 m	0.627	1.753
2	field area	1.80 m	0.595	1.809
2	ridge	1.80 m	0.576	1.809
2	gableboard	1.80 m	0.545	1.809

## Module arrays

Module array	Width[m]	Length[m]	Width in modules	Length in modules
1	13.96	1.21	8	1
2	13.96	1.21	8	1

# Roofs | Roof 4 | Module array 1



Roof **④** Module array **①**

Mounting System

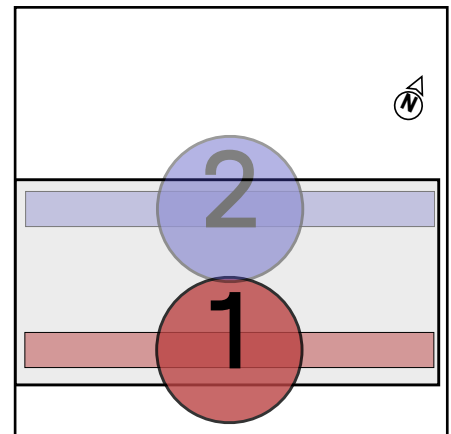
[SolidRail](#)

Module

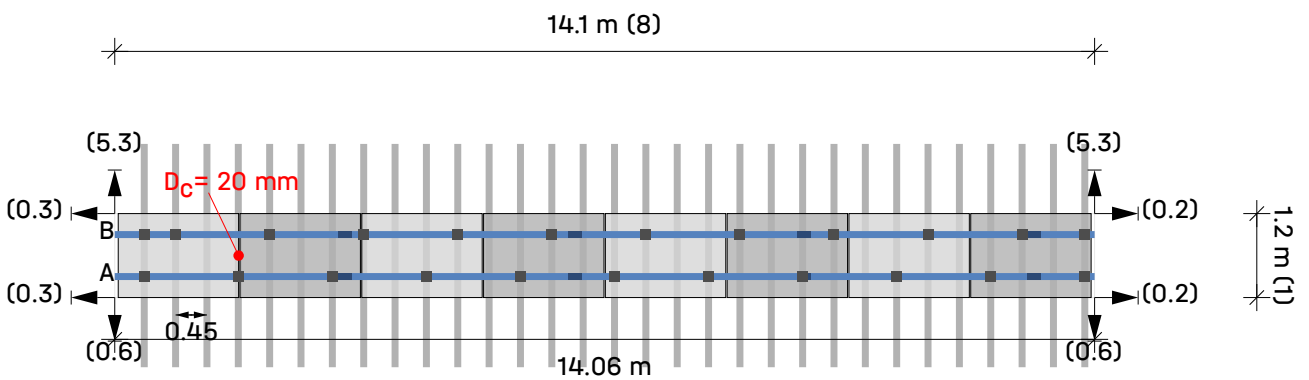
8(3.76 kWp) x Alpha Pure-RX

Row spacing

1.22 m



# Roofs | Roof 4 | Module array 1 | Module blocks

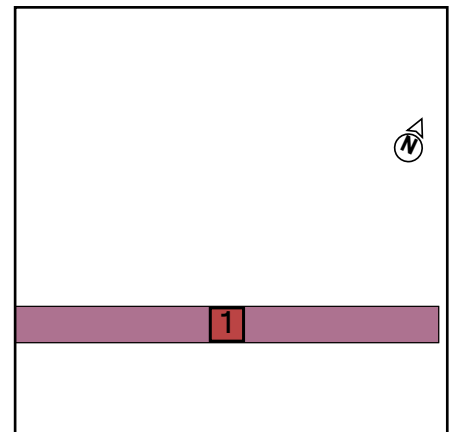


Roof **④** Module array **①** Module block **①**

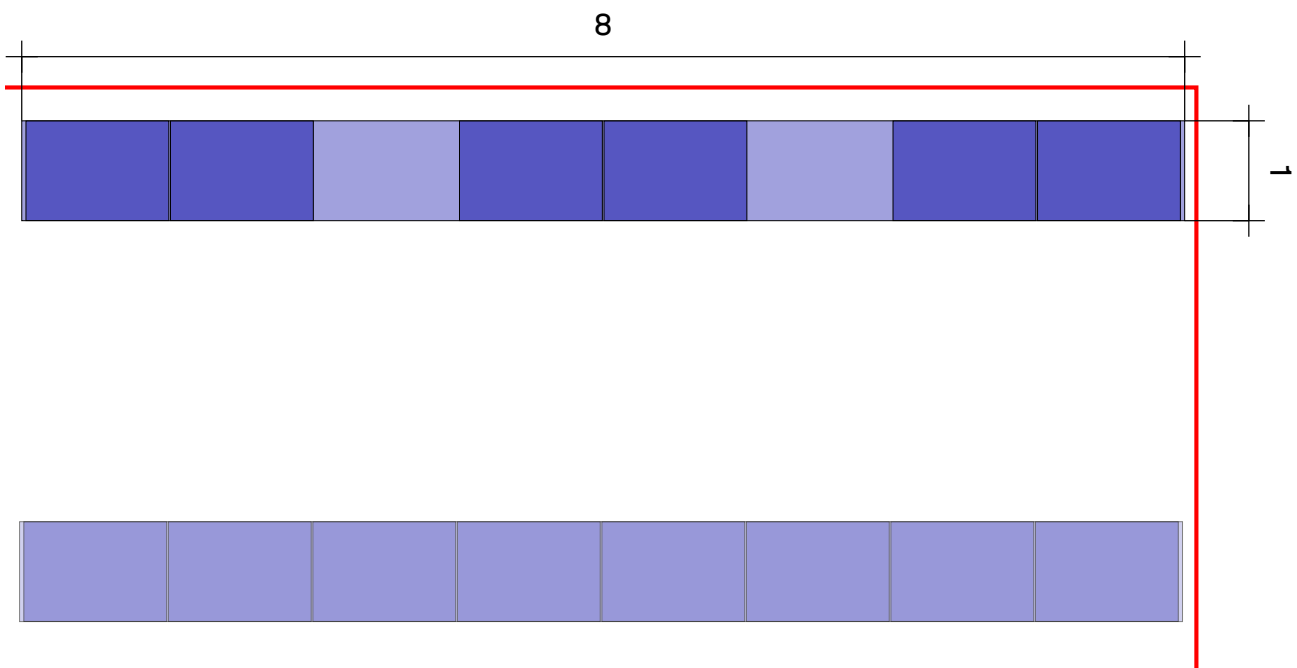
Modules  $8 \times 1 = 8$

Legend

- Fastener
- Mounting rail: K2 SolidRail Light 37
- Distance to Roof Edge [m]
- $D_c$  Distance for clamping between modules
- $D_m$  Distance between modules



# Roofs | Roof 4 | Module array 2



Roof **④** Module array **②**

Mounting System

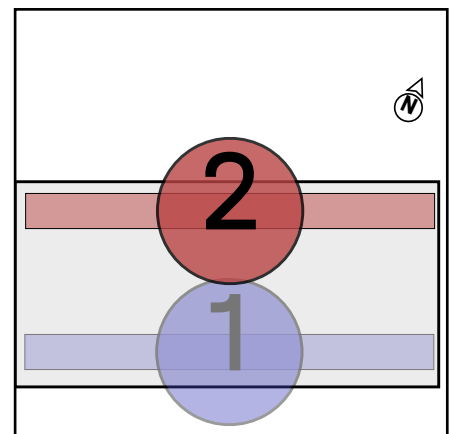
[SolidRail](#)

Module

6(2.82 kWp) x Alpha Pure-RX

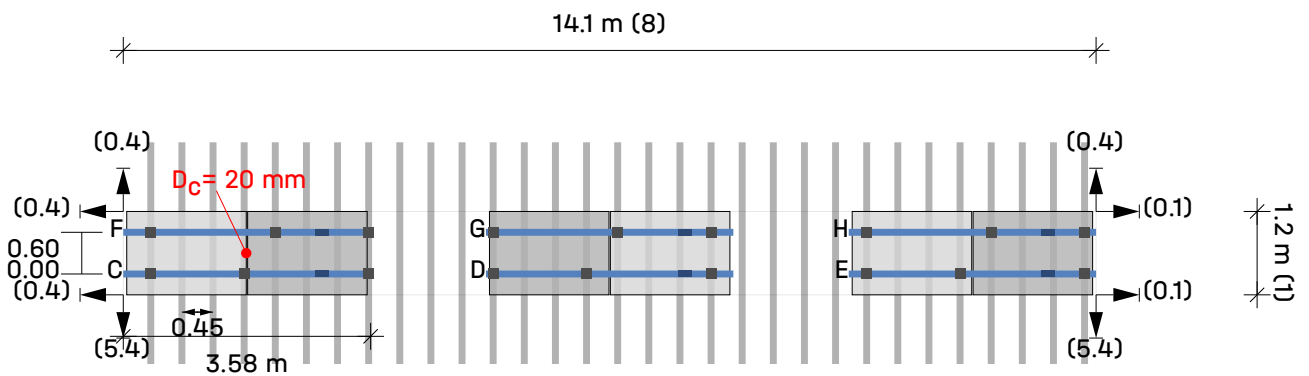
Row spacing

1.22 m





# Roofs | Roof 4 | Module array 2 | Module blocks

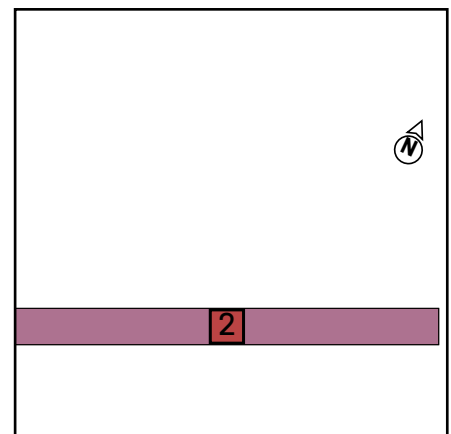


Roof **④**    Module array **②**    Module block **2**


Modules             $(8 \times 1) - 2 = 6$

Legend

- Fastener
- Mounting rail: K2 SolidRail Light 37
- Distance to Roof Edge [m]
- $D_c$  Distance for clamping between modules
- $D_m$  Distance between modules



# Results | Roof 4

Roof	System	Module	Height	Quantity	Total power
<a href="#">Roof 4</a>	<a href="#">SolidRail</a>	Alpha Pure-RX 1,728×1,205×30 mm 470 Wp	8.50 m	14	6.58 kWp
	Tile				

## Module

Name	Alpha Pure-RX
Manufacturer	REC
Output power	470 Wp
Dimensions	1,728×1,205×30 mm
Weight	23.4 kg

## Components

Fastener	SolidHook Pan Tile Vario 1
Base rails	K2 SolidRail Light 37

## Loads on modules (module dimensioning)

Array	A-TrA [m <sup>2</sup> ]	ultimate state [Pa]				Serviceability [Pa]			
		Pressure ⊥	Pressure 	Uplift ⊥	Uplift 	Pressure ⊥	Pressure 	Uplift ⊥	Uplift 
field area	2.08	799.6	252.7	-480.3	45.7	544.3	173.0	-282.5	45.7
corner region (eave)	2.08	910.3	252.7	-1,284.8	45.7	618.2	173.0	-818.8	45.7
eaves	2.08	827.2	252.7	-539.7	45.7	562.8	173.0	-322.1	45.7
field area	2.08	799.6	252.7	-480.3	45.7	544.3	173.0	-282.5	45.7
ridge	2.08	799.6	252.7	-988.1	45.7	544.3	173.0	-621.0	45.7
gableboard	2.08	799.6	252.7	-1,185.9	45.7	544.3	173.0	-752.9	45.7



# Results | Roof 4

## Utilisation result

No.	Module Array	roof areas	ultimate limit state			Usab.	Distances		maximum values	
			Pr $\sigma$ [%]	CL $\sigma$ [%]	Fst F[%]	Pr f[%]	Fst [m]	BR [m]	CL $L_{max}$ [m]	Fst Fst $D_{max}$ [m]
1		field area	45.8	0.0	99.5	52.1	1.800	---	0.595	1.809
1		corner region (eave)	35.3	17.3	84.1	27.1	1.350	---	0.566	1.606
1		eaves	26.5	0.0	77.0	18.9	1.350	---	0.627	1.753
2		field area	45.8	0.0	99.5	52.1	1.800	---	0.595	1.809
2		ridge	48.5	7.8	99.5	58.4	1.800	---	0.576	1.809
2		gableboard	58.0	14.2	99.5	70.8	1.800	---	0.545	1.809

- Pr Profile
- Fst Fastener
- $\sigma$  Stress
- f Deflection
- F Force
- CL/ $L_{max}$  maximum cantilever length
- Fst  $D_{max}$  maximum fastener spacing
- BR Base Rail
- Usab. serviceability limit state
- CL Cantilever



## Results | Roof 4

### Notes

- The dimensioning of the timber construction screws is not part of this structural analysis. The dimensioning and positioning of the timber construction screws to be used must be carried out in accordance with the respective applicable codes of practice.
- The structure was statically verified in accordance with Eurocode 9: Design of aluminum structures (DIN EN 1999-1-1:2021) and offers sufficient load-bearing capacity and stability for the loads specified in the chapter 'Maximum actions on the components'.
- Adjustment factor for wind load regarding service life period,  $f_W$ , is according to DIN EN 1991-1-4/ NA, NDP for 4.2 (2P) note 5, table 3
- Adjustment factor for snow load regarding service life period,  $f_S$ , is according to DIN EN 1991-1-3/ annex D, table 4
- The structural design complies with BS EN 1990 - Basis of Structural Design.
- Snow loads are determined in accordance with National Annex BS NA EN 1991-1-3 (2018) - UK National Annex to EC1 - Action on structures - general actions - snow loads.
- Wind loads are determined in accordance with National Annex BS NA EN 1991-1-4 - UK National Annex to EC1 - Action on structures, general actions - wind actions.
- Service life is recognised according to 'Eurocode EN 1991 - Action on structures, Snow loads' and 'Eurocode EN 1991 - Actions on structures, Wind actions'. Subject to the Building Regulations and for security-relevant reasons the installation has to be dismantled at the end of its service life.
- Failure consequence class is considered according to 'Eurocode EN 1990 - Basis of structural design'.
- Data and results must be verified with regard to local conditions and checked by a suitably qualified person. Please see our TCU under <https://k2-systems.com/en/base-tcu>, in particular § 2 ("technical and specialist requirements for the customer"), § 7 ("warranty provisions") and § 8 ("limitation of liability").
- The clamping range of the module may not comply with for this installation variant, as the rail spacing depends on the laying of the tiles and thus the batten length. Please ensure that the clamping range specified by the module manufacturer is observed.

# Structural analysis report | Roof 4

## General information

Name	Lincoln College - Museum Road - PV - Final
Mounting System	SolidRail
Author	Daniel Tempest

## Location information

Address	9 Museum Rd, Oxford OX1 3PX, UK
Ground elevation	65.14 m

## Roof information

Building height	8.50 m
Roof type	Gable roof
Roof pitch	22°
Roof covering	Tile
Min. roof edge distance	0.00 m
Rafter Spacing	0.450 m
Rafter width	50.0 mm
set edge rafter left	No
Rafter spacing left	310.0 mm
Rafter spacing right	No
Rafter Spacing	310.0 mm
Batten spacing	340.0 mm

## Loads

Design method	BS EN
Failure consequence class (CC)	CC2
Design working life	25 years
Terrain category	Town Terrain
Distance to shoreline	100.00 km
Distance inside town terrain	0.20 km

## Wind load

Velocity pressure, 50	$q_{p,50} = 0.716 \text{ kN/m}^2$
Adjustment factor for service life	$f_w = 1.000$
Velocity pressure, 25	$q_{p,25} = 0.659 \text{ kN/m}^2$

## Structural analysis report | Roof 4

### Roof areas

Array	load impact area [m <sup>2</sup> ]	maxCpe <sub>10</sub>	minCpe <sub>10</sub>	wind pressure [kN/m <sup>2</sup> ]	wind suction [kN/m <sup>2</sup> ]
field area	10.00	0.293	-0.600	0.193	-0.396
corner region (eave)	10.00	0.480	-1.413	0.317	-0.932
eaves	10.00	0.340	-0.660	0.224	-0.435
field area	10.00	0.293	-0.600	0.193	-0.396
ridge	10.00	0.293	-1.113	0.193	-0.734
gableboard	10.00	0.293	-1.313	0.193	-0.866

### Snow load

Snow load zone	2
Snow guard	No
Snow load on ground level	$s_k = 0.400 \text{ kN/m}^2$
Shape Coefficient for Snow	$\mu_i = 0.800$
Factor for roof pitch	$d_i = 0.927$
Snow load on roof, 50	$s_{i,50} = 0.297 \text{ kN/m}^2$
Adjustment factor for service life	$f_s = 1.000$
Snow load on roof, 25	$s_{i,25} = 0.276 \text{ kN/m}^2$

### Dead Load

Weight of module	$G_M = 23.4 \text{ kg}$
Weight of mounting system per module	$= 2.5 \text{ kg}$
Module area	$A_M = 2.08 \text{ m}^2$
Dead weight of module per m <sup>2</sup>	$= 11.24 \text{ kg/m}^2$
Dead weight of mounting system per m <sup>2</sup>	$= 1.20 \text{ kg/m}^2$
Total Dead Load (excl. ballast) per m <sup>2</sup>	$= 0.12 \text{ kN/m}^2$



# Structural analysis report | Roof 4

## Load Combinations

### Ultimate limit state

Partial safety factor unfavourable permanent load	$\gamma_{G,sup} = 1.35$
Partial safety factor favourable permanent load	$\gamma_{G,inf} = 1.00$
Partial safety factor destabilising permanent load	$\gamma_{G,dst} = 1.10$
Partial safety factor stabilising permanent load	$\gamma_{G,stab} = 0.90$
Partial safety factor variable loads	$\gamma_Q = 1.50$
Combination coefficient with regards to wind	$\psi_{0,W} = 0.60$
Combination coefficient with regards to wind (additional varying influences)	$\psi_{1,W} = 0.20$
Combination coefficient with regards to Snow	$\psi_{0,S} = 0.50$
Importance factor permanent	$K_{Fl,G} = 1.00$
Importance factor variable	$K_{Fl,Q} = 1.00$
Characteristic dead weight	$G_k$
Characteristic snow load on the roof	$S_{i,n}$
Characteristic wind load	$W_k$

Load case combination 01	$LCC\ 01_{uls} = \gamma_{G,sup} * K_{Fl,G} * G_k + \gamma_Q * K_{Fl,Q} * S_{i,n}$
Load case combination 02	$LCC\ 02_{uls} = \gamma_{G,sup} * K_{Fl,G} * G_k + \gamma_Q * K_{Fl,Q} * W_{k,Pressure}$
Load case combination 03	$LCC\ 03_{uls} = \gamma_{G,sup} * K_{Fl,G} * G_k + \gamma_Q * K_{Fl,Q} * (W_{k,Pressure} + \psi_{0,S} * S_{i,n})$
Load case combination 04	$LCC\ 04_{uls} = \gamma_{G,sup} * K_{Fl,G} * G_k + \gamma_Q * K_{Fl,Q} * (S_{i,n} + \psi_{0,W} * W_{k,Pressure})$
Load case combination 06	$LCC\ 06_{uls} = \gamma_{G,inf} * G_k + \gamma_Q * K_{Fl,Q} * W_{k,Suction}$

### Serviceability limit state

Combination coefficient with regards to wind	$\psi_{0,W} = 0.60$
Combination coefficient with regards to Snow	$\psi_{0,S} = 0.50$

Load case combination 01	$LCC\ 01_{sls} = G_k + S_{i,n}$
Load case combination 02	$LCC\ 02_{sls} = G_k + W_{k,Pressure}$
Load case combination 03	$LCC\ 03_{sls} = G_k + W_{k,Pressure} + \psi_{0,S} * S_{i,n}$
Load case combination 04	$LCC\ 04_{sls} = G_k + S_{i,n} + \psi_{0,W} * W_{k,Pressure}$
Load case combination 06	$LCC\ 06_{sls} = G_k + W_{k,Suction}$

## Structural analysis report | Roof 4

### Maximum load on modules (Mounting system dimensioning)

Array	A-TrA [m <sup>2</sup> ]	ultimate state [kN/m <sup>2</sup> ]				Serviceability [kN/m <sup>2</sup> ]			
		Pressure ⊥	Pressure 	Uplift ⊥	Uplift	Pressure ⊥	Pressure 	Uplift ⊥	Uplift
field area	10.00	0.800	0.253	-0.480	0.046	0.544	0.173	-0.283	0.046
corner region (eave)	10.00	0.910	0.253	-1.285	0.046	0.618	0.173	-0.819	0.046
eaves	10.00	0.827	0.253	-0.540	0.046	0.563	0.173	-0.322	0.046
field area	10.00	0.800	0.253	-0.480	0.046	0.544	0.173	-0.283	0.046
ridge	10.00	0.800	0.253	-0.988	0.046	0.544	0.173	-0.621	0.046
gableboard	10.00	0.800	0.253	-1.186	0.046	0.544	0.173	-0.753	0.046

### Max. load on fastener

Array	A-TrA [m <sup>2</sup> ]	ultimate state [kN]				Serviceability [kN]			
		Pressure ⊥	Pressure 	Uplift ⊥	Uplift	Pressure ⊥	Pressure 	Uplift ⊥	Uplift
field area	10.00	0.954	0.301	-0.573	0.055	0.649	0.206	-0.337	0.055
corner region (eave)	10.00	0.814	0.226	-1.150	0.041	0.553	0.155	-0.733	0.041
eaves	10.00	0.740	0.226	-0.483	0.041	0.504	0.155	-0.288	0.041
field area	10.00	0.954	0.301	-0.573	0.055	0.649	0.206	-0.337	0.055
ridge	10.00	0.954	0.301	-1.179	0.055	0.649	0.206	-0.741	0.055
gableboard	10.00	0.954	0.301	-1.415	0.055	0.649	0.206	-0.898	0.055

### Resistance Values of Components

#### Base Rails

Base Rails	A [cm <sup>2</sup> ]	I <sub>y</sub> [cm <sup>4</sup> ]	I <sub>z</sub> [cm <sup>4</sup> ]	W <sub>y</sub> [cm <sup>3</sup> ]	W <sub>z</sub> [cm <sup>3</sup> ]
K2 SolidRail Light 37	3.150	4.36	6.98	2.25	3.54

#### Fastener

Fastener	R <sub>D,Uplift,Perpendicular</sub> [kN]	R <sub>D,Pressure,Perpendicular</sub> [kN]	R <sub>D,Pressure,Parallel</sub> [kN]
SolidHook Pan Tile Vario 1	2.21	1.05	1.93





# Structural analysis report | Roof 4

## Utilisation result

No.	Module Array	roof areas	ultimate limit state			Usab.	Distances		maximum values	
			Pr $\sigma$ [%]	CL $\sigma$ [%]	Fst F[%]	Pr f[%]	Fst [m]	BR [m]	CL $L_{max}$ [m]	Fst Fst $D_{max}$ [m]
1		field area	45.8	0.0	99.5	52.1	1.800	---	0.595	1.809
1		corner region (eave)	35.3	17.3	84.1	27.1	1.350	---	0.566	1.606
1		eaves	26.5	0.0	77.0	18.9	1.350	---	0.627	1.753
2		field area	45.8	0.0	99.5	52.1	1.800	---	0.595	1.809
2		ridge	48.5	7.8	99.5	58.4	1.800	---	0.576	1.809
2		gableboard	58.0	14.2	99.5	70.8	1.800	---	0.545	1.809

Pr **Profile**  
 Fst **Fastener**  
 $\sigma$  **Stress**  
 f **Deflection**  
 F **Force**  
 CL/ $L_{max}$  **maximum cantilever length**

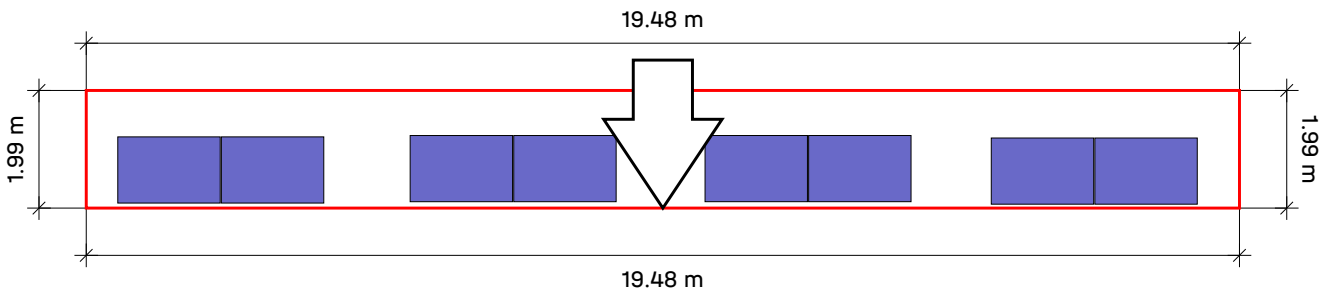
Fst  $D_{max}$  **maximum fastener spacing**  
 BR **Base Rail**  
 Usab. **serviceability limit state**  
 CL **Cantilever**



## Roofs | Roof 4 | Bill of material

Position	Item no.	Item description	Quantity	Weight
1	1000125	SolidHook Pan Tile Vario 1	41	29.2 kg
2	1000637	T-Bolt 28/15 M10×20	41	0.8 kg
3	1000042	Hexagon flange nut M10	41	0.5 kg
4	2004112	Wood screw 8×100	82	2.2 kg
5	2002589	OneEnd Black Set 30-42	16	1.4 kg
6	2003072	OneMid Black Set 30-42	20	1.6 kg
7	1004765	SolidRail Light End Cap	16	0.1 kg
8	2002870	K2 Solar Cable Manager	14	0.0 kg
9	2003232	SolidRail Light; 3.30 m	16	44.8 kg
10	1004107	SolidRail UltraLight+Light RailConnector Set	14	3.2 kg
<b>Total</b>				<b>83.7 kg</b>

# Roofs | Roof 5



Roof	System	Module	Height	Quantity	Total power
Roof 5	<a href="#">SolidRail</a>	REC 420 AA Pure-R (Alpha) 1,730×1,118×30 mm 420 Wp	9.32 m	8	3.36 kWp
Tile					

# Roofs | Roof 5 | Assembly plan

## Base Rails

Type	Whole Rails		Rail cutting		
	Total Rail Length	Quantity 3.30 m	Part of Rail	Length	Rest
2*A	3.580	1*3.30 m	3.300	0.700 from 3.300	<u>2.590</u>
2*B	3.580	1*3.30 m	<u>2.590</u>	0.700 from 2.590	<u>1.880</u>
2*C	3.580	1*3.30 m	<u>1.880</u>	0.700 from 1.880	<u>1.170</u>
2*D	3.580	1*3.30 m	<u>1.170</u>	0.700 from 1.170	0.460

1 cm is viewed as lost for each cutting

Red numbers are leftover rails which will not be used any longer

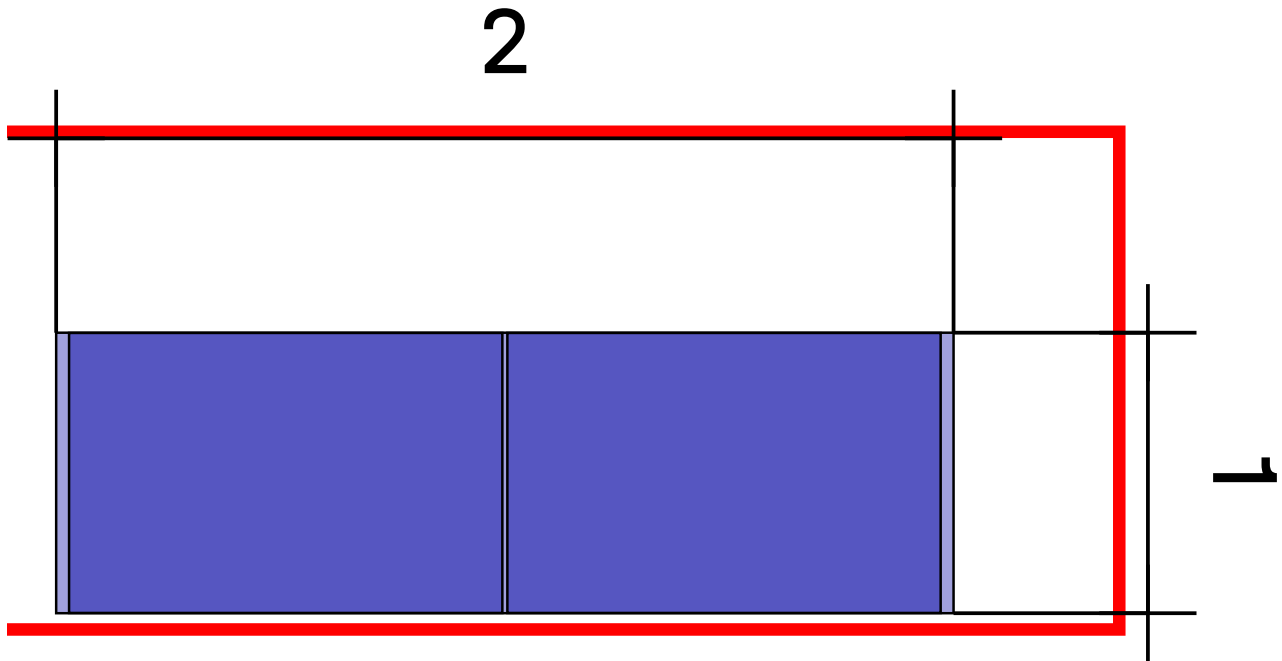
## Fastener Spacing

Module	Array	Distance	maximum cantilever length	maximum fastener spacing
1	field area	1.35 m	0.625	1.708
2	field area	1.35 m	0.625	1.708
3	field area	1.35 m	0.625	1.708
4	field area	1.35 m	0.625	1.708

## Module arrays

Module array	Width[m]	Length[m]	Width in modules	Length in modules
1	3.48	1.12	2	1
2	3.48	1.12	2	1
3	3.48	1.12	2	1
4	3.48	1.12	2	1

# Roofs | Roof 5 | Module array 1



Roof ⑤ Module array ①

Mounting System

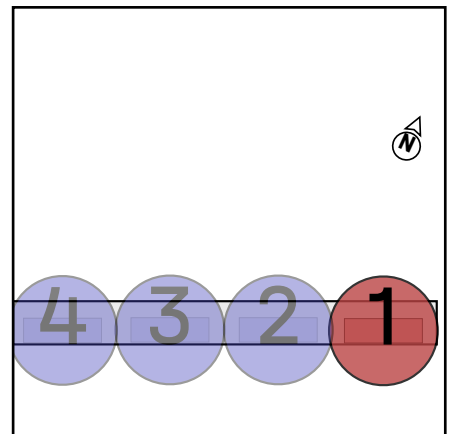
[SolidRail](#)

Module

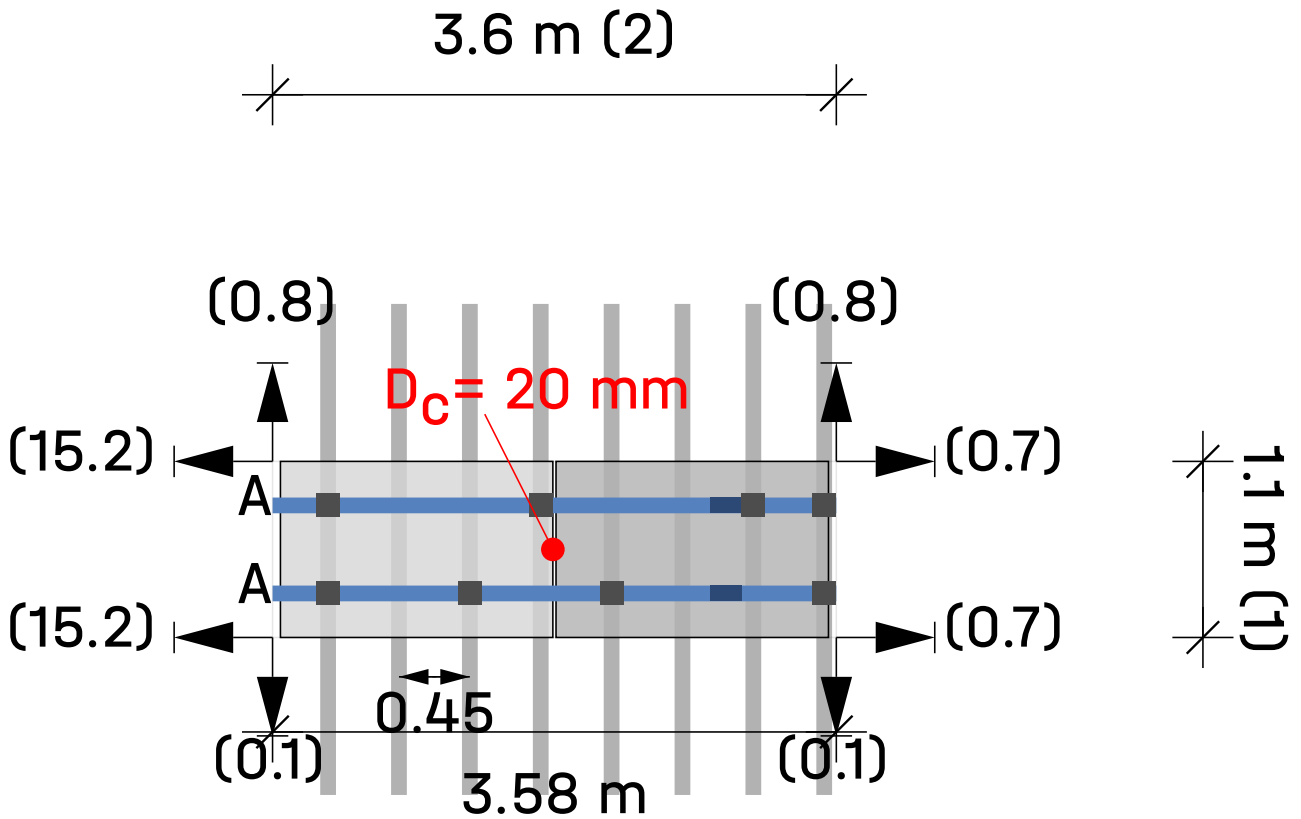
2(0.84 kWp) x REC 420 AA  
Pure-R (Alpha)

Row spacing

1.13 m



# Roofs | Roof 5 | Module array 1 | Module blocks

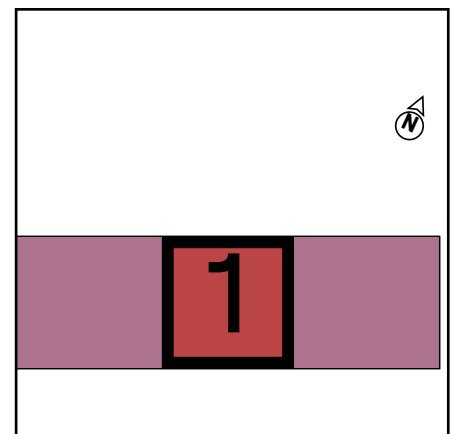


Roof **5**    Module array **1**    Module block **1**

Modules                     $2 \times 1 = 2$

Legend

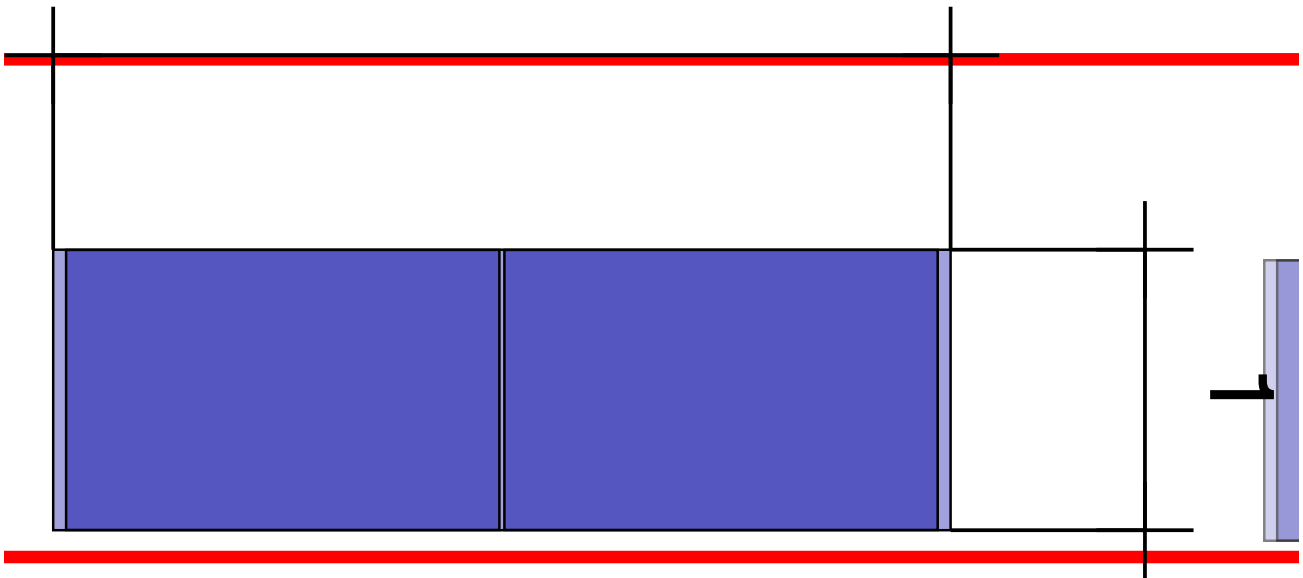
- Fastener
- Mounting rail: K2 SolidRail Light 37
- Distance to Roof Edge [m]
- $D_c$  Distance for clamping between modules
- $D_m$  Distance between modules



# Roofs | Roof 5 | Module array 2



# 2



Roof ⑤ Module array ②

Mounting System

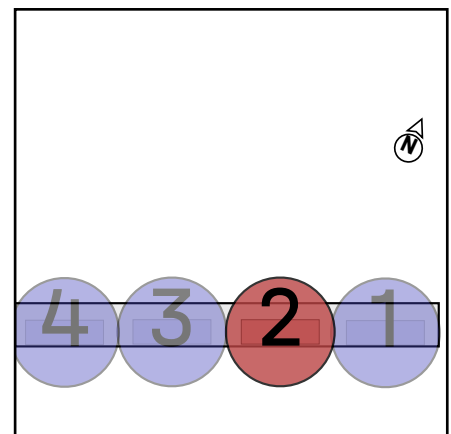
[SolidRail](#)

Module

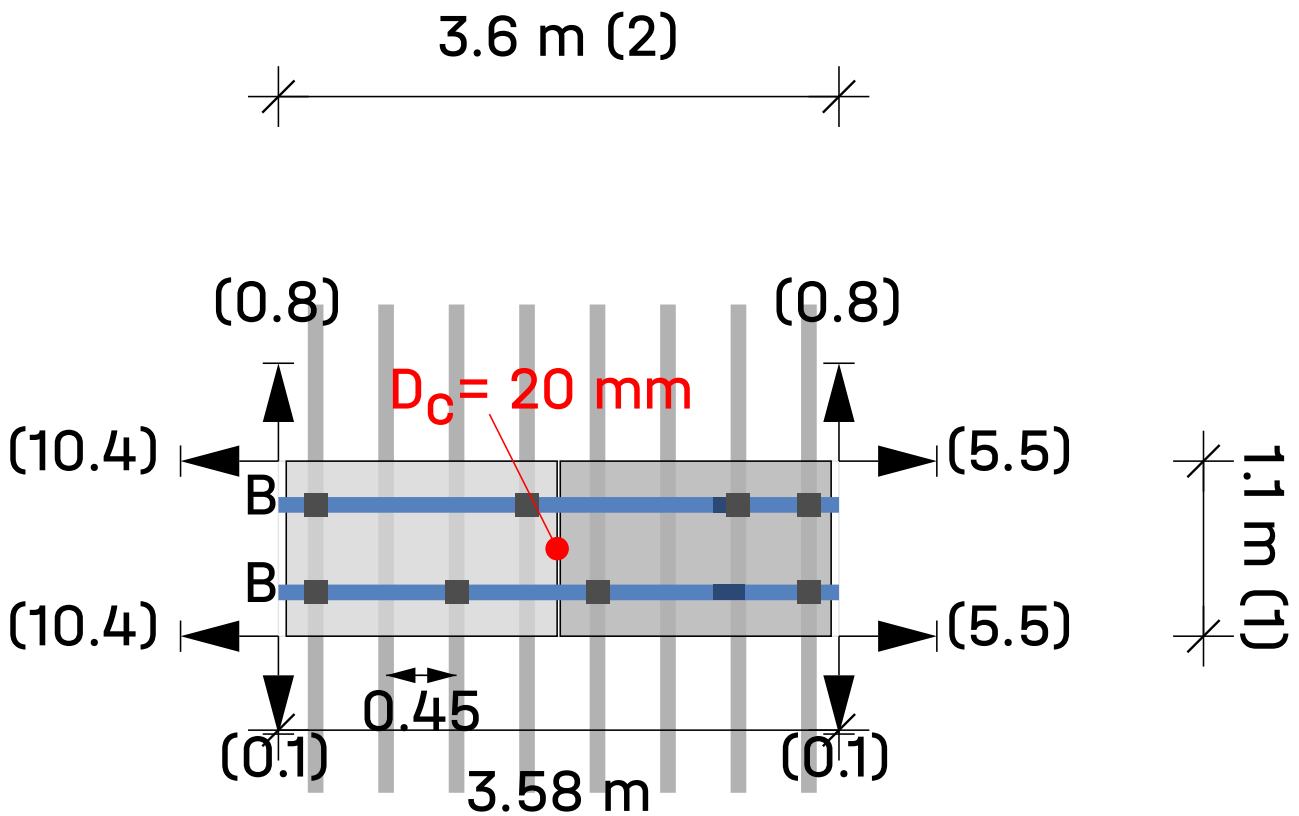
2(0.84 kWp) x REC 420 AA  
Pure-R (Alpha)

Row spacing

1.13 m



# Roofs | Roof 5 | Module array 2 | Module blocks

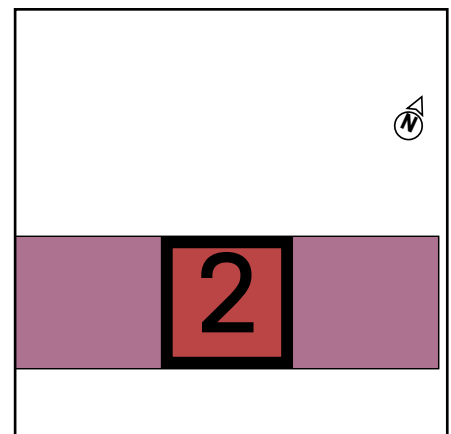


Roof **⑤** Module array **②** Module block **2**

Modules  $2 \times 1 = 2$

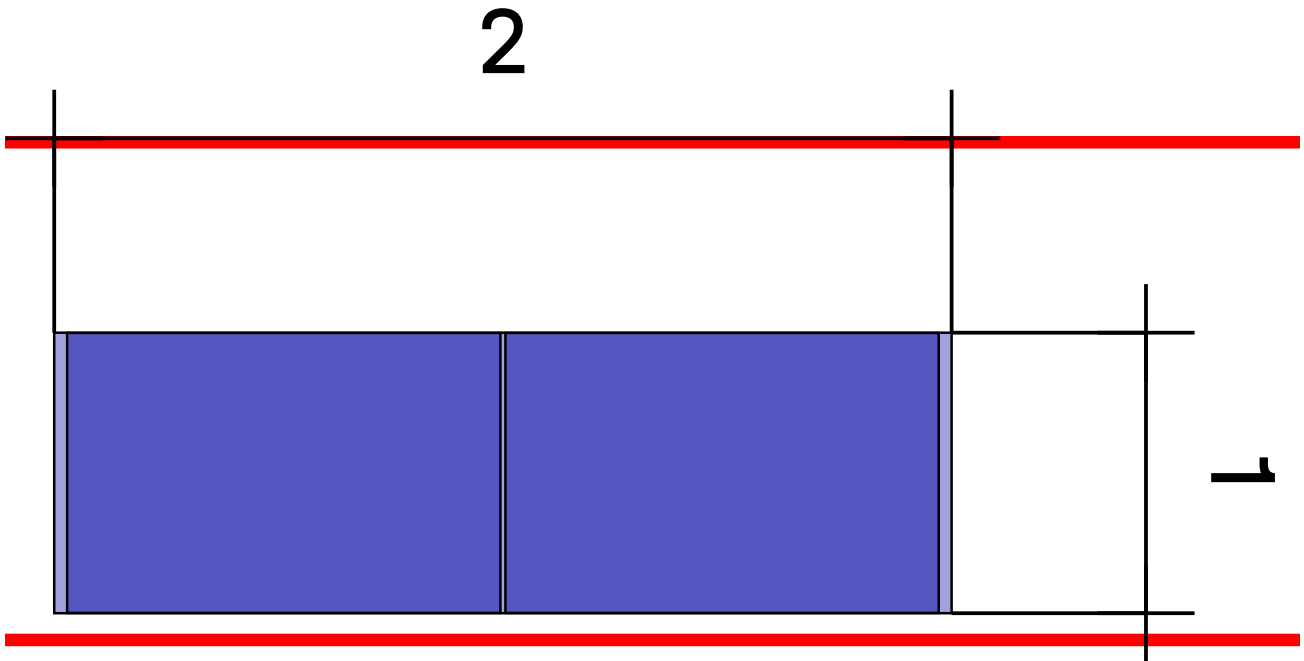
Legend

- Fastener
- Mounting rail: K2 SolidRail Light 37
- Distance to Roof Edge [m]
- $D_c$  Distance for clamping between modules
- $D_m$  Distance between modules





# Roofs | Roof 5 | Module array 3



Roof ⑤ Module array ③

Mounting System

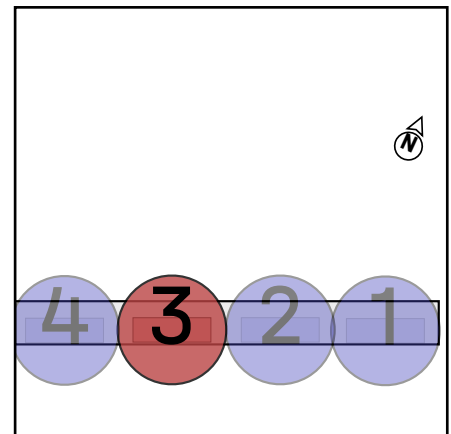
[SolidRail](#)

Module

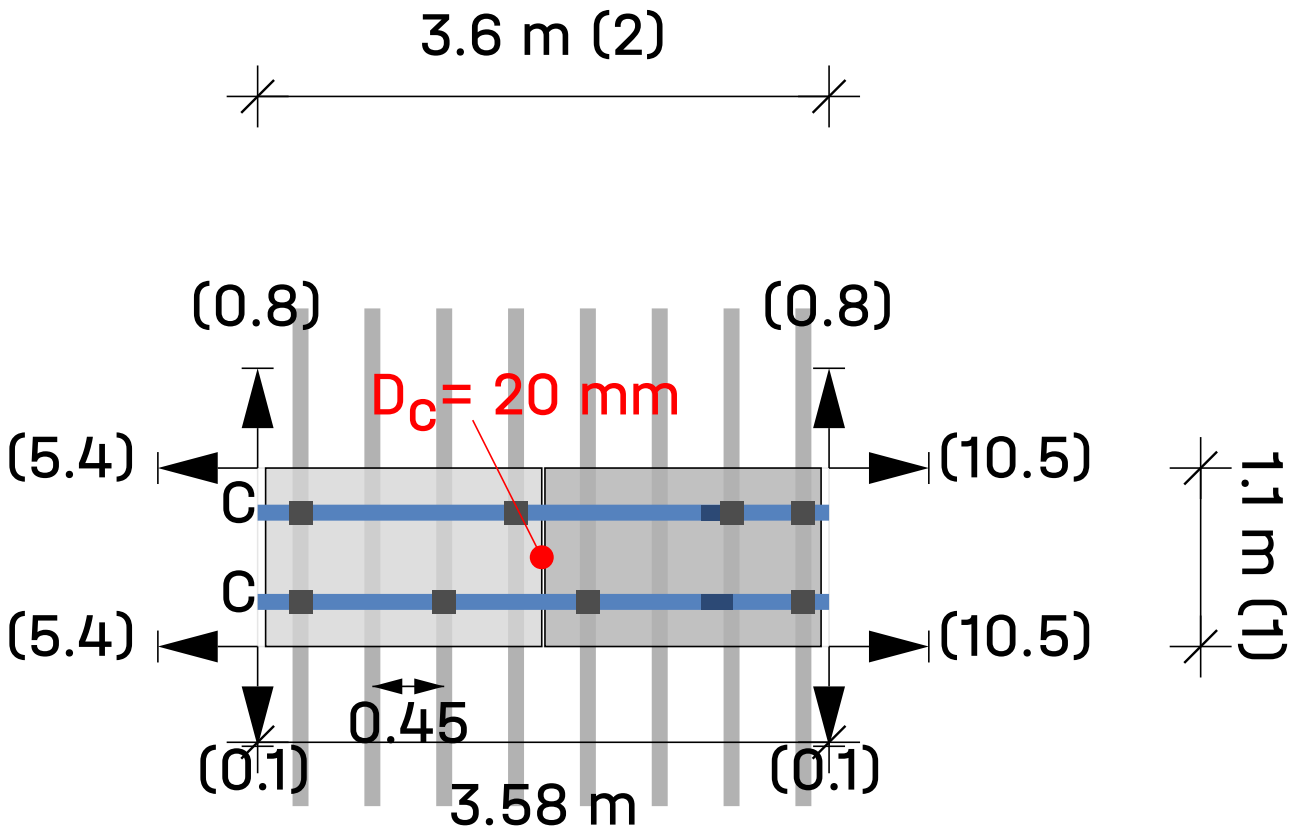
2(0.84 kWp) x REC 420 AA  
Pure-R (Alpha)

Row spacing

1.13 m



# Roofs | Roof 5 | Module array 3 | Module blocks

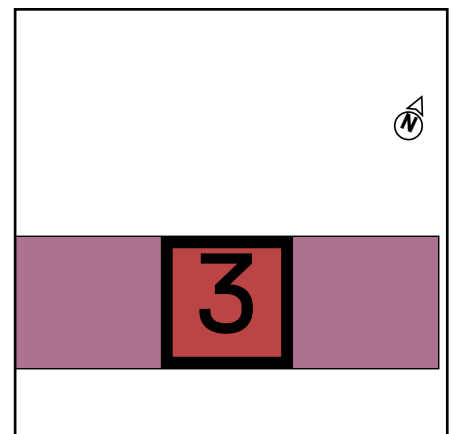


Roof **⑤** Module array **③** Module block **3**

Modules  $2 \times 1 = 2$

Legend

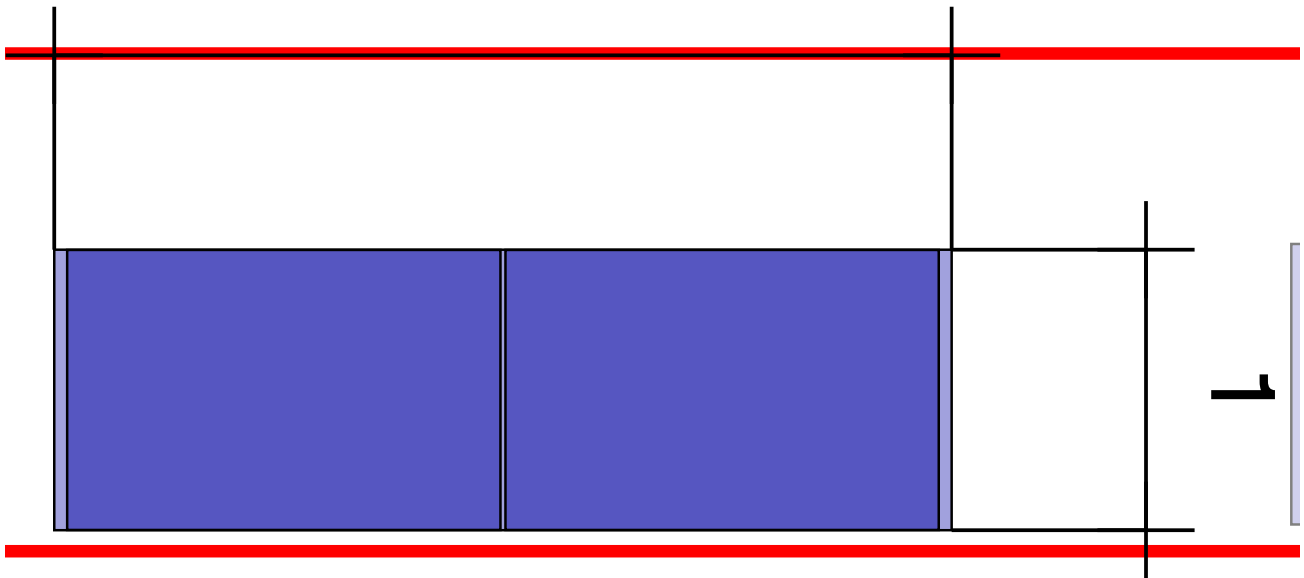
- Fastener
- Mounting rail: K2 SolidRail Light 37
- Distance to Roof Edge [m]
- $D_c$  Distance for clamping between modules
- $D_m$  Distance between modules



# Roofs | Roof 5 | Module array 4



2



Roof ⑤ Module array ④

Mounting System

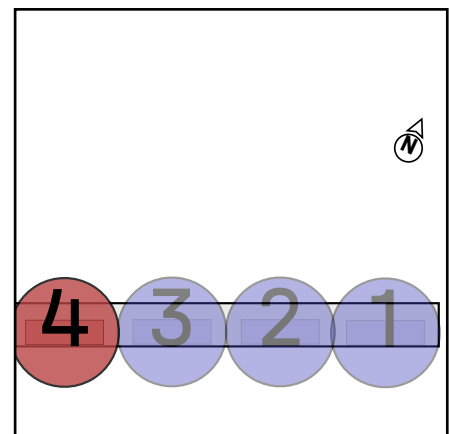
[SolidRail](#)

Module

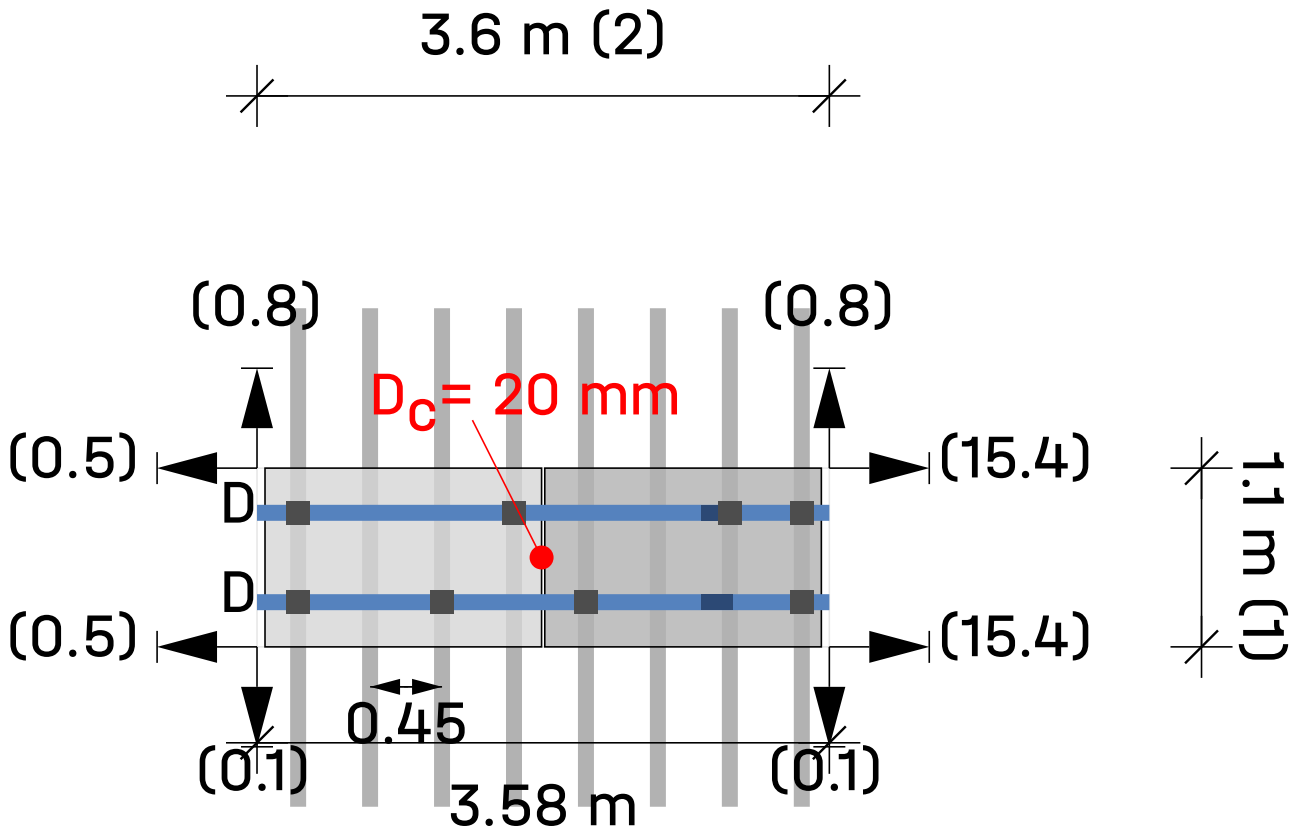
2(0.84 kWp) x REC 420 AA  
Pure-R (Alpha)

Row spacing

1.13 m



# Roofs | Roof 5 | Module array 4 | Module blocks

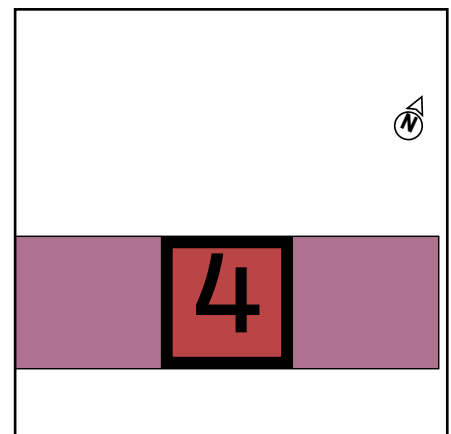


Roof **5**    Module array **4**    Module block **4**

Modules  $2 \times 1 = 2$


Legend

- Fastener
- Mounting rail: K2 SolidRail Light 37
- Distance to Roof Edge [m]
- $D_c$  Distance for clamping between modules
- $D_m$  Distance between modules





# Results | Roof 5

Roof	System	Module	Height	Quantity	Total power
Roof 5	SolidRail	REC 420 AA Pure-R (Alpha) 1,730×1,118×30 mm 420 Wp	9.32 m	8	3.36 kWp
 Tile					

## Module

Name	REC 420 AA Pure-R (Alpha)
Manufacturer	REC Solar AS
Output power	420 Wp
Dimensions	1,730×1,118×30 mm
Weight	21.5 kg

## Components

Fastener	SolidHook Pan Tile Vario 1
Base rails	K2 SolidRail Light 37

## Loads on modules (module dimensioning)

Array	A-TrA [m <sup>2</sup> ]	ultimate state [Pa]				Serviceability [Pa]			
		Pressure ⊥	Pressure	Uplift ⊥	Uplift	Pressure ⊥	Pressure	Uplift ⊥	Uplift
field area	1.93	887.4	371.8	-503.4	60.8	602.2	253.9	-300.5	60.8
field area	1.93	887.4	371.8	-503.4	60.8	602.2	253.9	-300.5	60.8
field area	1.93	887.4	371.8	-503.4	60.8	602.2	253.9	-300.5	60.8
field area	1.93	887.4	371.8	-503.4	60.8	602.2	253.9	-300.5	60.8

## Utilisation result

No.	roof areas	ultimate limit state			Usab.	Distances		maximum values	
		Pr	CL	Fst		Pr	Fst	CL	Fst
Module Array		σ[%]	σ[%]	F[%]	f[%]	[m]	[m]	L <sub>max</sub> [m]	D <sub>max</sub> [m]
1	field area	28.0	9.6	79.0	19.1	1.350	---	0.625	1.708
2	field area	28.0	4.3	79.0	19.1	1.350	---	0.625	1.708
3	field area	28.0	5.5	79.0	19.1	1.350	---	0.625	1.708
4	field area	28.0	5.1	79.0	19.1	1.350	---	0.625	1.708

Pr	Profile	Fst D <sub>max</sub>	maximum fastener spacing
Fst	Fastener	BR	Base Rail
σ	Stress	Usab.	serviceability limit state
f	Deflection	CL	Cantilever



## Results | Roof 5

F Force

CL/L<sub>max</sub> maximum cantilever length



## Results | Roof 5

### Notes

- The dimensioning of the timber construction screws is not part of this structural analysis. The dimensioning and positioning of the timber construction screws to be used must be carried out in accordance with the respective applicable codes of practice.
- The structure was statically verified in accordance with Eurocode 9: Design of aluminum structures (DIN EN 1999-1-1:2021) and offers sufficient load-bearing capacity and stability for the loads specified in the chapter 'Maximum actions on the components'.
- Adjustment factor for wind load regarding service life period,  $f_W$ , is according to DIN EN 1991-1-4/ NA, NDP for 4.2 (2P) note 5, table 3
- Adjustment factor for snow load regarding service life period,  $f_S$ , is according to DIN EN 1991-1-3/ annex D, table 4
- The structural design complies with BS EN 1990 - Basis of Structural Design.
- Snow loads are determined in accordance with National Annex BS NA EN 1991-1-3 (2018) - UK National Annex to EC1 - Action on structures - general actions - snow loads.
- Wind loads are determined in accordance with National Annex BS NA EN 1991-1-4 - UK National Annex to EC1 - Action on structures, general actions - wind actions.
- Service life is recognised according to 'Eurocode EN 1991 - Action on structures, Snow loads' and 'Eurocode EN 1991 - Actions on structures, Wind actions'. Subject to the Building Regulations and for security-relevant reasons the installation has to be dismantled at the end of its service life.
- Failure consequence class is considered according to 'Eurocode EN 1990 - Basis of structural design'.
- Data and results must be verified with regard to local conditions and checked by a suitably qualified person. Please see our TCU under <https://k2-systems.com/en/base-tcu>, in particular § 2 ("technical and specialist requirements for the customer"), § 7 ("warranty provisions") and § 8 ("limitation of liability").
- The clamping range of the module may not comply with for this installation variant, as the rail spacing depends on the laying of the tiles and thus the batten length. Please ensure that the clamping range specified by the module manufacturer is observed.

# Structural analysis report | Roof 5

## General information

Name	Lincoln College - Museum Road - PV - Final
Mounting System	SolidRail
Author	Daniel Tempest

## Location information

Address	9 Museum Rd, Oxford OX1 3PX, UK
Ground elevation	65.14 m

## Roof information

Building height	9.32 m
Roof type	Gable roof
Roof pitch	30°
Roof covering	Tile
Min. roof edge distance	0.00 m
Rafter Spacing	0.450 m
Rafter width	50.0 mm
set edge rafter left	No
Rafter spacing left	290.0 mm
Rafter spacing right	No
Rafter Spacing	290.0 mm
Batten spacing	340.0 mm

## Loads

Design method	BS EN
Failure consequence class (CC)	CC2
Design working life	25 years
Terrain category	Town Terrain
Distance to shoreline	100.00 km
Distance inside town terrain	0.20 km

## Wind load

Velocity pressure, 50	$q_{p,50} = 0.734 \text{ kN/m}^2$
Adjustment factor for service life	$f_w = 0.921$
Velocity pressure, 25	$q_{p,25} = 0.676 \text{ kN/m}^2$



# Structural analysis report | Roof 5

## Roof areas

Array	load impact area [m <sup>2</sup> ]	maxCpe <sub>10</sub>	minCpe <sub>10</sub>	wind pressure [kN/m <sup>2</sup> ]	wind suction [kN/m <sup>2</sup> ]
field area	10.00	0.400	-0.600	0.271	-0.406
field area	10.00	0.400	-0.600	0.271	-0.406
field area	10.00	0.400	-0.600	0.271	-0.406
field area	10.00	0.400	-0.600	0.271	-0.406

## Snow load

Snow load zone	2
Snow guard	No
Snow load on ground level	$s_k = 0.400 \text{ kN/m}^2$
Shape Coefficient for Snow	$\mu_i = 1.200$
Factor for roof pitch	$d_i = 0.866$
Snow load on roof, 50	$s_{i,50} = 0.416 \text{ kN/m}^2$
Adjustment factor for service life	$f_s = 0.929$
Snow load on roof, 25	$s_{i,25} = 0.386 \text{ kN/m}^2$

## Dead Load

Weight of module	$G_M = 21.5 \text{ kg}$
Weight of mounting system per module	$= 2.5 \text{ kg}$
Module area	$A_M = 1.93 \text{ m}^2$
Dead weight of module per m <sup>2</sup>	$= 11.12 \text{ kg/m}^2$
Dead weight of mounting system per m <sup>2</sup>	$= 1.29 \text{ kg/m}^2$
Total Dead Load (excl. ballast) per m <sup>2</sup>	$= 0.12 \text{ kN/m}^2$



# Structural analysis report | Roof 5

## Load Combinations

### Ultimate limit state

Partial safety factor unfavourable permanent load	$\gamma_{G,sup} = 1.35$
Partial safety factor favourable permanent load	$\gamma_{G,inf} = 1.00$
Partial safety factor destabilising permanent load	$\gamma_{G,dst} = 1.10$
Partial safety factor stabilising permanent load	$\gamma_{G,stab} = 0.90$
Partial safety factor variable loads	$\gamma_Q = 1.50$
Combination coefficient with regards to wind	$\psi_{0,W} = 0.60$
Combination coefficient with regards to wind (additional varying influences)	$\psi_{1,W} = 0.20$
Combination coefficient with regards to Snow	$\psi_{0,S} = 0.50$
Importance factor permanent	$K_{Fl,G} = 1.00$
Importance factor variable	$K_{Fl,Q} = 1.00$
Characteristic dead weight	$G_k$
Characteristic snow load on the roof	$S_{i,n}$
Characteristic wind load	$W_k$

Load case combination 01	$LCC\ 01_{uls} = \gamma_{G,sup} * K_{Fl,G} * G_k + \gamma_Q * K_{Fl,Q} * S_{i,n}$
Load case combination 02	$LCC\ 02_{uls} = \gamma_{G,sup} * K_{Fl,G} * G_k + \gamma_Q * K_{Fl,Q} * W_{k,Pressure}$
Load case combination 03	$LCC\ 03_{uls} = \gamma_{G,sup} * K_{Fl,G} * G_k + \gamma_Q * K_{Fl,Q} * (W_{k,Pressure} + \psi_{0,S} * S_{i,n})$
Load case combination 04	$LCC\ 04_{uls} = \gamma_{G,sup} * K_{Fl,G} * G_k + \gamma_Q * K_{Fl,Q} * (S_{i,n} + \psi_{0,W} * W_{k,Pressure})$
Load case combination 06	$LCC\ 06_{uls} = \gamma_{G,inf} * G_k + \gamma_Q * K_{Fl,Q} * W_{k,Suction}$

### Serviceability limit state

Combination coefficient with regards to wind	$\psi_{0,W} = 0.60$
Combination coefficient with regards to Snow	$\psi_{0,S} = 0.50$

Load case combination 01	$LCC\ 01_{sls} = G_k + S_{i,n}$
Load case combination 02	$LCC\ 02_{sls} = G_k + W_{k,Pressure}$
Load case combination 03	$LCC\ 03_{sls} = G_k + W_{k,Pressure} + \psi_{0,S} * S_{i,n}$
Load case combination 04	$LCC\ 04_{sls} = G_k + S_{i,n} + \psi_{0,W} * W_{k,Pressure}$
Load case combination 06	$LCC\ 06_{sls} = G_k + W_{k,Suction}$

## Maximum load on modules (Mounting system dimensioning)

Array	A-TrA [m <sup>2</sup> ]	ultimate state [kN/m <sup>2</sup> ]				Serviceability [kN/m <sup>2</sup> ]			
		Pressure ⊥	Pressure	Uplift ⊥	Uplift	Pressure ⊥	Pressure	Uplift ⊥	Uplift
field area	10.00	0.887	0.372	-0.503	0.061	0.602	0.254	-0.300	0.061
field area	10.00	0.887	0.372	-0.503	0.061	0.602	0.254	-0.300	0.061
field area	10.00	0.887	0.372	-0.503	0.061	0.602	0.254	-0.300	0.061
field area	10.00	0.887	0.372	-0.503	0.061	0.602	0.254	-0.300	0.061

# Structural analysis report | Roof 5

## Max. load on fastener

Array	A-TrA [m <sup>2</sup> ]	ultimate state [kN]				Serviceability [kN]			
		Pressure ⊥	Pressure	Uplift ⊥	Uplift	Pressure ⊥	Pressure	Uplift ⊥	Uplift
field area	10.00	0.737	0.309	-0.418	0.051	0.500	0.211	-0.249	0.051
field area	10.00	0.737	0.309	-0.418	0.051	0.500	0.211	-0.249	0.051
field area	10.00	0.737	0.309	-0.418	0.051	0.500	0.211	-0.249	0.051
field area	10.00	0.737	0.309	-0.418	0.051	0.500	0.211	-0.249	0.051

## Resistance Values of Components

### Base Rails

Base Rails	A [cm <sup>2</sup> ]	I <sub>y</sub> [cm <sup>4</sup> ]	I <sub>z</sub> [cm <sup>4</sup> ]	W <sub>y</sub> [cm <sup>3</sup> ]	W <sub>z</sub> [cm <sup>3</sup> ]
K2 SolidRail Light 37	3.150	4.36	6.98	2.25	3.54

### Fastener

Fastener	R <sub>D,Uplift,Perpendicular</sub> [kN]	R <sub>D,Pressure,Perpendicular</sub> [kN]	R <sub>D,Pressure,Parallel</sub> [kN]
SolidHook Pan Tile Vario 1	2.21	1.05	1.93

### Utilisation result

No.	roof areas	ultimate limit state			Usab.	Distances		maximum values	
		Pr σ[%]	CL σ[%]	Fst F[%]	Pr f[%]	Fst [m]	BR [m]	CL L <sub>max</sub> [m]	Fst Fst D <sub>max</sub> [m]
1	field area	28.0	9.6	79.0	19.1	1.350	---	0.625	1.708
2	field area	28.0	4.3	79.0	19.1	1.350	---	0.625	1.708
3	field area	28.0	5.5	79.0	19.1	1.350	---	0.625	1.708
4	field area	28.0	5.1	79.0	19.1	1.350	---	0.625	1.708

- Pr **Profile**
- Fst **Fastener**
- σ **Stress**
- f **Deflection**
- F **Force**
- CL/L<sub>max</sub> **maximum cantilever length**
- Fst D<sub>max</sub> **maximum fastener spacing**
- BR **Base Rail**
- Usab. **serviceability limit state**
- CL **Cantilever**



## Roofs | Roof 5 | Bill of material

Position	Item no.	Item description	Quantity	Weight
1	2004112	Wood screw 8×100	64	1.7 kg
2	2002589	OneEnd Black Set 30-42	16	1.4 kg
3	1000125	SolidHook Pan Tile Vario 1	32	22.8 kg
4	1000637	T-Bolt 28/15 M10×20	32	0.6 kg
5	1000042	Hexagon flange nut M10	32	0.4 kg
6	2003072	OneMid Black Set 30-42	8	0.6 kg
7	1004765	SolidRail Light End Cap	16	0.1 kg
8	2002870	K2 Solar Cable Manager	8	0.0 kg
9	2003232	SolidRail Light; 3.30 m	10	28.0 kg
10	1004107	SolidRail UltraLight+Light RailConnector Set	8	1.8 kg
<b>Total</b>				<b>57.4 kg</b>



## Bill of material

Position	Item no.	Item description	Quantity	Weight
1	2004112	Wood screw 8×100	562	15.2 kg
2	2002589	OneEnd Black Set 30-42	124	10.8 kg
3	1000125	SolidHook Pan Tile Vario 1	281	200.1 kg
4	1000637	T-Bolt 28/15 M10×20	281	5.1 kg
5	1000042	Hexagon flange nut M10	281	3.1 kg
6	2003072	OneMid Black Set 30-42	98	7.7 kg
7	1004765	SolidRail Light End Cap	124	0.7 kg
8	2002870	K2 Solar Cable Manager	80	0.2 kg
9	2003232	SolidRail Light; 3.30 m	87	243.6 kg
10	1004107	SolidRail UltraLight+Light RailConnector Set	68	15.3 kg
<b>Total</b>				<b>501.9 kg</b>

## Thank you for choosing a K2 mounting system.

Systems from K2 Systems are quick and easy to install.  
We hope these instructions have helped.  
Please contact us with any questions or suggestions for improvement.

Our contact data:

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