



Surface Water Drainage Design

The proposed design of the car park development is for a permeable tarmac surface construction. The surface of the tarmac area shall be permeable with the underlying stone sub-base acting as an attenuation/storage area for surface water. The stone base will act as an attenuation system to increase attenuation capabilities of the area. The car park base will be permeable allowing the entire area to naturally infiltrate through to the underlying soils.

The granular substrate (typically consisting of Type 3 unbound (SHW 800 Series) to comply with BSEN 13285) is intended to provide onsite containment and attenuation within the granular sub-base, before surface water enters an outfall.

The designed surface water drainage solution should be based upon the following criteria, to maintain satisfactory system performance:

- Provide adequate functionality over a period of twenty years.
- Ensure that surface water is effectively removed from the facility construction to ensure that load bearing capacity of the substrate is not weakened by an increase in moisture content or becomes more susceptible to frost damage.
- Protect the installation from influences of groundwater or surface water from surrounding areas.
- Prevent the risk of uncontrolled flooding elsewhere (to land adjacent to the development).
- Comply with all applicable Sustainable Urban Drainage System (SUDS) requirements with attenuated flows (containment within the granular sub-base) incorporated wherever necessary, without affecting the performance of the car park.

Only natural surface water is being dealt with. The new development will not increase to the volume of water that the existing site area is currently subjected to.

The area is in a Category 1 flood zone and as such is at a low risk of flooding. Water discharging from the area currently infiltrates through the existing soils, without any control or restrictions. Therefore, the proposal for a permeable car park with voids within the sub-base will increase the attenuation capabilities of the site.

Infiltration testing has been undertaken near the proposed car park and has derived an infiltration rate of 8.16x10-6 m/s which has been used in the soakaway calculations to the right.

Surface water discharge rate will be attenuated weithin the car park before infiltrating through the car park sub-base of the site will provide a more careful, managed control of discharge than the current arrangement.

The foundations of the new synthetic turf area includes:

- 300mm deep layer of type 3 stone
- 100m macadam car park surface

The minimum 400mm deep aggregate base construction offers a wedge for surface water attenuation prior to filling and flooding the car park surface or surrounding areas. The available volume of the wedge is created through the following calculations:

- The car park layer constructed at a 1 in 65 gradient offers a volume and capacity of 75m3 -- Based on a voidage space of 40% this offers 30m3 of water attenuation.
- The surface water attenuation calculations as per the table right shows the following;

1 in 100 year storm event + 40% allowance for climate change would require **13m3** of attenuation

The attenuation provided by the car park design (30m3) will cater for a 1 in 100 storm event +40% (13m3) without flooding either the car park surface or surrounding areas.

66,0-+-	Lower East finished car park level: 65.475	Proposed car park levels	Extent and area of potential surface water attenuation storage		Higher west finished car park level: 65.925
65.0	Lower East formation car park level: 65.075	Fc	rmation levels	<u>* * * * * * * * * * * * * * * * * * * </u>	Higher west formation car park level: 65.525
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Section A	A-AA Showing	Surface Water	Attenuation	1 Storage -	Scale 1:100

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DO NOT SCALE FROM DRAWING UNLESS FOR PLANNING PURPOSES ONLY, ALL DIMENSIONS TO BE CHECKED ON SITE PRIOR T COMMENCEMENT OF WORKS.

ALL PERSONNEL SHOULD BE AWARE OF THE HEALTH AND SAFETY PLAN S RETAINED IN THE SITE MANAGERS OFFIC

consider Area to be drained = 424 m2			Discharge through entire car park sub-base and external soakaway				
100 vea	or + 40% climate	e change					
Flow	Timo	M5-D	72	M100-D	Inflow	Outflow	Storage
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	5	2011111 X 21	1 862	14.2	8.4	1.0	7 30
	5 10	10.8	1.002	14.2 20.8	0.4 12.3	1.0	10.33
	10	10.0	1.920	20.0	12.5	2.0	11.55
	30	12.0	1.950	24.7	14.0	5.0	12.02
	50	20.0	1.990	32.0	19.0		12.95
	120	20.0	2.030	40.0	24.1	12.1	12.00
	240	24.0	2.014	40.3 57 9	20.7	24.2 19.1	4.50
	240	29.2	1.9/0	57.0 62.5	34.3	40.4	
	360	32.0	1.954	02.0	37.1	72.0	
	600	30.0	1.914	70.1	41.6	121.0	
	1440	45.6	1.842	84.0	49.9	290.4	U
Soakawa Availablo Availablo Surface a Eff volun Addition	ay details: e storage un e storage wi e storage ar area: ne: al storage v	15.6 Ass nder car park (ithin drainage rea = 3(412 m2 30.00 m3 rolume require	m wide sume void r based on 1 system: 0.00 m3 > 12.9 ed: -1	26.4 ratio 0.4 :65 fall): 0.00 3 m3 OK	m lloennggth 30.00 m3	n 0.25 m dee .: 41) m3	əp m3
Half drai (soakawa Total Sto	n down time ay only): orage requir	e 3846.6 red = approx. <i>*</i>	5 secs 13m3. Stora	1.06851 age currently	hours / provided = :	30m3. No additic	onal storage
			1	equired.			
This vol In or	ume will acc der to meet	commodate flo building regu achieve	ows from all lation stana ed. This req	modelled 1 rds, a half d juirement is	in 100 year + rain-down tin met at the sit	• 40% climate cha ne of 24 hours sh te.	nge events. Iould be
ar P	ark /	Attenu	Jatio	n Vo	lume	Calcu	lations

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